# Updated Speed and Crash Analysis of Speed Limit Changes on Eastern Oregon Interstates and Highways 

# FINAL REPORT 

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## EXECUTIVE SUMMARY

Maximum speed limits for passenger vehicles and trucks was increased in March 2016 on approximately 1,400 miles of highways and interstates in Eastern Oregon. Specifically, speed limits were increased to $70 \mathrm{mi} / \mathrm{hr}$ (cars) and $65 \mathrm{mi} / \mathrm{hr}$ (trucks) on interstates and $65 \mathrm{mi} / \mathrm{hr}$ (cars) and $55 \mathrm{mi} / \mathrm{hr}$ (trucks) on a network of rural highways. This report presents the analysis of the changes in average vehicles speed, percentage of vehicles traveling over 65,75 , and $85 \mathrm{mi} / \mathrm{hr}$, traffic volume, total crashes, and fatal and injury crashes for both vehicles and trucks after the speed limit increases. The before and after data on the segments where the speed limits were increased are compared to a data for segments in a control group. In addition, an EmpiricalBayes before and after safety analysis, recommended by AASHTO Highway Safety Manual as state-of-the-practice, was conducted for total crashes on interstate and non-interstate roadways.

The change in vehicle speeds was evaluated by comparing three years of before and after data available from 18 automated traffic recorders (ATRs) in control sections to 16 ATRs in the segments where the speeds were increased. By all measures, vehicle speeds have increased on the highways where the posted speed limits were increased. The average speeds have increased approximately $3 \mathrm{mi} / \mathrm{hr}$, and these changes are statistically significant. More importantly, the percentage of fast vehicles has also increased. On the interstates where the speed limit was increased to $70 \mathrm{mi} / \mathrm{hr}$, about $13 \%$ percent more vehicles were observed traveling faster than 75 $\mathrm{mi} / \mathrm{hr}$ than before the speed limit change. On the control sections, the change over the same time period was only a $2.5 \%$ increase. For the network of rural highways where the speed limits were increased to $65 \mathrm{mi} / \mathrm{hr}$, about $2 \%$ percent more vehicles were traveling faster than $75 \mathrm{mi} / \mathrm{hr}$ compared to only a $0.25 \%$ increase on the control sections in the same time period.

The safety performance of the $1,426.5$ miles of highway where the speed limits were increased was compared to a selected control group of 691.2 miles of highway using three years of before and after crash data. On the interstates, where speed limits were increased to $70 \mathrm{mi} / \mathrm{hr}$, the descriptive crash analysis suggests that the total number of crashes, fatal/injury A crashes, and truck-involved crashes have increased. Using the more robust Empirical Bayes before-after method, a crash modification factor (CMF) of 1.10 (1.0 to 1.2) was calculated, which suggests total crashes increased by $10 \%$ following the increase in speed limits. On the network of rural two-lane highways where speeds were increased from $55 \mathrm{mi} / \mathrm{hr}$ to $65 \mathrm{mi} / \mathrm{hr}$, both the descriptive analysis of crash frequency and the Empirical Bayes before-after method suggest a much more significant increase of total crashes, fatal/injury A crashes, and truck-involved crashes in crashes. A CMF of 1.49 , with a $95 \%$ confidence band of 1.40 to 1.58 , suggests total crashes increased due to the speed limit increase by nearly $50 \%$.

### 1.0 INTRODUCTION

This report is an updated analysis of speed and crash performance changes on Eastern Oregon highways and interstates. A preliminary analysis, using only one year of crash data after the changes were implemented, was conducted in 2018. A report was prepared and presented to the Oregon Transportation Commission (OTC). The analysis found that vehicle speeds increased on the highways where the posted speed limits increased, and the preliminary trend in crash performance warranted continued monitoring. This report is a follow-up, updating the analysis with a longer time series of data.

The speed limit changes became law and were effective March 1, 2016. The legislation raised maximum speed limits for passenger vehicles and trucks on approximately 1,400 miles of highways and interstates in Eastern Oregon. Specifically, speed limits were increased as follows: $70 \mathrm{mi} / \mathrm{hr}$ (cars) and $65 \mathrm{mi} / \mathrm{hr}$ (trucks), and $65 \mathrm{mi} / \mathrm{hr}$ (cars) and $55 \mathrm{mi} / \mathrm{hr}$ (trucks). The speed limit increases are detailed in Section 1 in ORS 811.111, where the corridors with speed limit increases are as follows (corridors visually shown in Figure 1.1): ${ }^{1}$

- (2) Interstate 84 beginning at the eastern city limit of The Dalles and ending at the Idaho border.
o $65 \mathrm{mi} / \mathrm{hr}$ for trucks; $70 \mathrm{mi} / \mathrm{hr}$ for all other vehicles.
- (3) State Highway 95 beginning at the Idaho border and ending at the Nevada border.
o $65 \mathrm{mi} / \mathrm{hr}$ for trucks; $70 \mathrm{mi} / \mathrm{hr}$ for all other vehicles.
- (4) State Highway 20 beginning in Bend and ending in Ontario.
o $60 \mathrm{mi} / \mathrm{hr}$ for trucks; $65 \mathrm{mi} / \mathrm{hr}$ for all other vehicles.
- (5) State Highway 197 beginning in The Dalles and ending at the junction with State Highway 97, and State Highway 97 beginning at the junction with State Highway 197 and the California border.
o $60 \mathrm{mi} / \mathrm{hr}$ for trucks; $65 \mathrm{mi} / \mathrm{hr}$ for all other vehicles.
- (6) State Highway 31 beginning in Valley Falls and ending in La Pine. o $60 \mathrm{mi} / \mathrm{hr}$ for trucks; $65 \mathrm{mi} / \mathrm{hr}$ for all other vehicles.
- (7) State Highway 78 beginning in Burns Junction and ending in Burns.

0 $60 \mathrm{mi} / \mathrm{hr}$ for trucks; $65 \mathrm{mi} / \mathrm{hr}$ for all other vehicles.

- (8) State Highway 395 beginning in Burns and ending in John Day o $60 \mathrm{mi} / \mathrm{hr}$ for trucks; $65 \mathrm{mi} / \mathrm{hr}$ for all other vehicles.
- (9) State Highway 395 beginning in Riley and ending at the California border. o $60 \mathrm{mi} / \mathrm{hr}$ for trucks; $65 \mathrm{mi} / \mathrm{hr}$ for all other vehicles.
- (10) Oregon Route 205 beginning in Burns and ending in Frenchglen.
o $60 \mathrm{mi} / \mathrm{hr}$ for trucks; $65 \mathrm{mi} / \mathrm{hr}$ for all other vehicles.
- (11) State Highway 26 beginning in John Day and ending in Vale. o $60 \mathrm{mi} / \mathrm{hr}$ for trucks; $65 \mathrm{mi} / \mathrm{hr}$ for all other vehicles.

[^0]- (12) Interstate 82 beginning at the Washington border and ending at the junction with Interstate 84.
o $65 \mathrm{mi} / \mathrm{hr}$ for trucks; $70 \mathrm{mi} / \mathrm{hr}$ for all other vehicles.


Figure 1.1: Corridors with a Speed Limit Change

Further, the Oregon Department of Transportation lowered speeds to $55 \mathrm{mi} / \mathrm{hr}$ (cars) and 55 $\mathrm{mi} / \mathrm{hr}$ (trucks) on sections of the increased speed limit corridors following an engineering analysis. The speed reduction segments are as follows:

- US-97 - Dover Lane to Lower Bridge (MP 98.6 to MP 115, Madras to Terrebonne)
- US-20 - Dalton Street to Dodds Road (MP 3.17 to MP 9.19, east of Bend)
- US-97 - 6th Street to Masten Road (MP 168.8 to MP 169.87, south of La Pine)
- US-20 - Hawley Loop to Olds Ferry-Ontario (MP 246.84 to MP 258.20, Vale to Cairo)


### 2.0 DATA AND METHODS

The updated analysis compares speed, volume, and safety changes on segments where speed limits were increased. In addition to before-after comparisons, an Empirical Bayes approach to assess the impact of increased speed limits on safety through the use of a crash modification factor (CMF) was determined. For comparison purposes, a set of highways in which speed limits did not change were chosen as control segments. To compare before-after safety performance on segments where there was a speed limit increase and the control segments (no increase), three years of before data and three years of after data were used.

### 2.1 SPEED AND VOLUME DATA FROM ATR STATIONS

Throughout Oregon, the Department of Transportation operates and maintains a network of permanent traffic counting stations. These permanent stations continuously record the presence of vehicles and their corresponding speed, and dependent on the type of station (e.g., weigh-inmotion stations), record weight, axle counts, spacing, and vehicle classification. For this updated analysis, the same ATR stations assessed in the preliminary analysis are investigated with additional years of data after the speed limit increase. This includes both segments with increased speed limits and control segments. Portland State University obtained traffic and speed data at select ATRs from March 2013 to April 2019 through the Oregon Traffic Monitoring System website. This new platform was implemented after the previous study, resulting in changes in the traffic data file formatting (see Figure 2.1 for an example of the monthly report format). As a result, all data used in this updated analysis was obtained from the new platform.

As shown in Figure 2.1, the monthly report includes vehicle counts by day of the week for various speed bins, direction, and total vehicle counts for the month by speed bin. The monthly reports average all lanes and vehicle types (passenger vehicles and trucks). Due to trucks traveling at lower speeds and some stations having higher truck volumes, the overall (all vehicle) speed metrics may be lower due to the heavy volumes of lower speed trucks.


Figure 2.1: Sample ATR Station Report

All monthly reports have the same number/type of speed bins with the updated formatting. Following the preliminary analysis, computing estimated monthly average speed for each ATR was done by considering the number of vehicles in a given speed bin ( $n_{\text {bin }}$ ) multiplying by the speed bins' midpoint assumed speed ( $s_{b i n}$ ). This computation was summed for each bin then divided by the total number of vehicles ( $n_{\text {total }}$ ) to obtain an estimated monthly average speed:

$$
\begin{equation*}
\text { Estimated Average Speed }=\frac{\sum_{\forall \text { bins }}\left(n_{b i n}\right)\left(s_{b i n}\right)}{\left(n_{\text {total }}\right)} \tag{2.1}
\end{equation*}
$$

The speed bins used for computing the average speed, as shown in Eq. (2.1), are detailed in Table 2.1.

Table 2.1: Bin Speeds Used for Computing Average Speed

| Bin (mi/hr) | Midpoint Speed $\left(\boldsymbol{s}_{\boldsymbol{b} \boldsymbol{i n}}\right)$ |
| :---: | :---: |
| $0-20$ | 18 |
| $20-25$ | 22.5 |
| $25-30$ | 27.5 |
| $30-35$ | 32.5 |
| $35-40$ | 37.5 |
| $40-45$ | 42.5 |
| $45-50$ | 47.5 |
| $50-55$ | 52.5 |
| $55-60$ | 57.5 |
| $60-65$ | 62.5 |
| $65-70$ | 67.5 |
| $70-75$ | 72.5 |
| $75-80$ | 77.5 |
| $80-85$ | 82.5 |
| $>85$ | 87.5 |

### 2.1.1 Control Locations

The same ATR stations in the preliminary analysis were considered for this updated analysis. Table 2.2 shows the selected ATRs, current speed limit, location (Oregon highway number), most current available AADT (2019), and the percentage of trucks. The control location ATRs are visually shown in Figure 2.2. Control locations were chosen on I-84 west of The Dalles at Rowena (33-001) and Cascade Locks (14-004), as well as I-5 at Wilsonville (03-011), North Albany (22-016), and Lake Creek (22-016). These stations were chosen to compare with the speed increase segments on I-84 east of The Dalles and on I-82.

The next set of locations was selected to match highways that most closely resemble the highways where speed limits were increased, including OR-58 near Oakridge (20-107), US-26 near Rhododendron (03-006), and US-20 near Black Butte (09-014). The final set of ATRs was located on US-97, where the speed limit is $45 \mathrm{mi} / \mathrm{hr}$, to assess the effect of any carryover speeds in Madras (16-002) and North Redmond (09-023) from the increased speed limits on US-97. The remaining control locations include ATRs in Eastern Oregon on highways with no speed limit increase.

Table 2.2: Control Segment ATRs

| ATR | Name | Speed Limit <br> $(\mathbf{m i} / \mathbf{h r})$ | Location (Oregon Hwy. No.) | MP | AADT ${ }^{\text {a }}$ | Percent <br> Trucks |
| :---: | :--- | :---: | :--- | :---: | :---: | :---: |
| $01-001$ | North Powder | 55 | LA GRANDE-BAKER (066) | 37.7 | 820 | 39.9 |
| $03-006$ | Rhododendron | 55 | MT. HOOD (026) | 46.38 | 10,200 | 16.8 |
| $03-011$ | Wilsonville | 65 | PACIFIC (001) | 281.2 | 100,100 | 16.2 |
| $09-003$ | Lava Butte | 55 | THE DALLES-CALIFORNIA (004) | 142.41 | 26,700 | 10.4 |
| $09-014$ | Sisters | 55 | SANTIAM (016) | 93.12 | 9,200 | 10.4 |
| $09-023$ | North Redmond | 45 | THE DALLES-CALIFORNIA (004) | 119.09 | 23,500 | 9.8 |
| $11-007$ | Shutler | 55 | JOHN DAY (005) | 6.81 | 970 | 49.1 |
| $12-003$ | Dayville | 55 | JOHN DAY (005) | 131.4 | 940 | 28.9 |
| $14-004$ | Cascade Locks | 65 | COLUMBIA RIVER (002) | 45.53 | 25,400 | 25.8 |
| $15-013$ | Shady Cove | 55 | CRATER LAKE (022) | 15.46 | 8,600 | 11.3 |
| $16-002$ | Madras | 45 | THE DALLES-CALIFORNIA (004) | 97.11 | 15,100 | 15.4 |
| $20-005$ | Noti | 55 | FLORENCE-EUGENE (062) | 43.86 | 6,800 | 21.6 |
| $20-017$ | Oakridge | 55 | WILLAMETTE (018) | 37.36 | 3,200 | 42.4 |
| $22-005$ | North Albany | 65 | PACIFIC (001) | 234.8 | 70,500 | 18.5 |
| $22-016$ | Lake Creek | 65 | PACIFIC (001) | 214.56 | 42,500 | 23.4 |
| $25-007$ | Lexington | 55 | HEPPNER (052) | 37.83 | 1,400 | 23.7 |
| $30-021$ | Milton | 55 | OREGON-WASHINGTON (008) | 34.46 | 15,300 | 4.8 |
| 33-001 | Rowena | 65 | COLUMBIA RIVER (002) | 75.93 | 24,700 | 25.9 |

${ }^{\text {a }}$ All AADT values are for 2019

### 2.1.2 Increased Speed Limit Segments

A total of 16 ATR locations were selected to measure speeds at locations where there was a speed limit increase (these stations follow those in the preliminary analysis). Table 2.3 shows the selected ATRs, current speed limit, location (Oregon highway number), most current available AADT (2019), and the percentage of trucks.

Where speed limits were increased to $70 \mathrm{mi} / \mathrm{hr}$, six ATRs on I-84 were selected (01-013 at South Baker, 23-016 at Huntington, 25-008 at Boardman Jct, 28-002 at Rufus, 30-004 at Pendleton, and 31-007 at North La Grande). Following ATR selections from the preliminary analysis, no ATRs were selected on US-95, a two-lane highway with a speed limit increase to $70 \mathrm{mi} / \mathrm{hr}$.

Four ATRs on US-97 were selected where speed limits were increased to $65 \mathrm{mi} / \mathrm{hr}$ (09-020 at Redmond, 18-019 at Midland, 18-022 at Modoc Point, and 33-005 at Dufur). The remaining selected ATRs are on two-lane highway segments where speed limits were increased to $65 \mathrm{mi} / \mathrm{hr}$.

Table 2.3: Increased Speed Limit Segment ATRs

| ATR | Name | Speed Limit <br> $(\mathbf{m i} / \mathbf{h r})$ | Location (Oregon Hwy. No.) | MP | AADT ${ }^{\mathbf{a}}$ | Percent <br> Trucks |
| :---: | :--- | :---: | :--- | :---: | :---: | :---: |
| $01-013$ | South Baker | 70 | OLD OREGON TRAIL (006) | 309.02 | 10,600 | 44.2 |
| $09-005$ | Pilot Butte | 65 | CENTRAL OREGON (007) | 6.28 | 3,200 | 29.7 |
| $09-020$ | Redmond | 65 | THE DALLES-CALIFORNIA (004) | 124.39 | 36,800 | 9.7 |
| $12-009$ | Prairie City | 65 | JOHN DAY (005) | 175.79 | 920 | 31.4 |
| $13-001$ | Burns | 65 | JOHN DAY-BURNS (048) | 66.3 | 510 | 32.6 |
| $13-007$ | Steens | 65 | STEENS (442) | 1.7 | 1,800 | 33.4 |
| $18-019$ | Midland | 65 | THE DALLES-CALIFORNIA (004) | 289.44 | 4,200 | 43.7 |
| $18-022$ | Modoc Point | 65 | THE DALLES-CALIFORNIA (004) | 254.3 | 6,600 | 39.5 |
| $19-008$ | New Pine Creek | 65 | FREMONT (019) | 157.43 | 870 | 26.1 |
| $23-006$ | Cairo Junction | 65 | CENTRAL OREGON (007) | 257.86 | 5,400 | 23.8 |
| $23-016$ | Huntington | 70 | OLD OREGON TRAIL (006) | 353.47 | 11,900 | 44.0 |
| $25-008$ | Boardman Jct | 70 | OLD OREGON TRAIL (006) | 168.55 | 17,900 | 26.1 |
| $28-002$ | Rufus | 70 | COLUMBIA RIVER (002) | 109.51 | 13,000 | 40.3 |
| $30-004$ | Pendleton | 70 | OLD OREGON TRAIL (006) | 203.34 | 17,500 | 30.3 |
| $31-007$ | North La Grande | 70 | OLD OREGON TRAIL(006) | 260.12 | 10,800 | 42.9 |
| 33-005 | Dufur | 65 | THE DALLES-CALIFORNIA (004) | 10.3 | 2,800 | 19.0 |

${ }^{\text {a }}$ All AADT values are for 2019

## Legend

- ATRs in Control Sections
- ATRs in Speed Change
- state_boundary Posted Speed
SPEED
SPEED
-55
-60
二 $_{70}^{65}$


Figure 2.2: ATR Locations Used in Speed Analysis

### 2.2 SAFETY PERFORMANCE

Crash data used for this analysis was obtained from historical DECODE databases, provided by ODOT. Only crashes on the mainline were considered, and crashes were specifically queried based on milepost markers and highway name/number. This was visually and manually confirmed for each highway using geospatial locations of crashes and ArcGIS (i.e., any crash that was not on the mainline was removed). Routes, speed limits, highway name (Oregon highway number), and milepost information used for the queries are shown in Table 2.4 (control locations) and Table 2.5(speed limit increase locations). The crash data, total crashes, fatal and injury A crashes, and proportions of crash types were analyzed. Additionally, truck-involved crashes were analyzed, where truck-involved crashes were defined as any crash in which a truck was involved (i.e., at least one of the vehicles in the crash was a truck). From the DECODE databases, trucks were defined as the following: (1) Vehicle Type 2 - truck tractor with no trailer (bobtail), (2) Vehicle Type 4 - truck tractor with trailer/mobile home in tow, and (3) Vehicle Type 5 - truck with non-detachable bed, panel, etc.

It should be noted that the DECODE databases that are released annually may have small differences when compared to the official 10-year database maintained by ODOT. This is due to errors that are corrected after the publication of the annual data. It is expected that these differences are minor.

### 2.2.1 Control Segments

A set of control highway segments was selected to compare crash trends to locations in which there was no speed limit increase. Sections of highways that pass through cities, towns, or congested areas were not included. A total of 691.17 miles of highway were selected for comparison (see Table 2.4).

For interstates, a segment on I-84 from east of Portland to The Dalles (C-1) and a segment on I-5 from Wilsonville to just north of Eugene (C-11) were selected. On interstates, speed limits for trucks were raised to $60 \mathrm{mi} / \mathrm{hr}$ in late 2017, which is not explicitly accounted for in this analysis.

Highway segments, to the extent possible, were selected based on their representation as higher volume, higher speed, and two-lane highways, such as US-26 (C-3), US-20 (C-6), OR-58 (C-5), and OR-126 (C-7 and C-10). The remaining highway segments were selected based on them being the best comparison to Eastern Oregon, where speeds were not increased, including OR-19 (C-8), OR-3 (C-9), and OR-140 (C-12). As these Eastern Oregon highways were not being selected by the Oregon legislature as part of the speed limit increases, they are not necessarily an ideal comparison to the higher volume principal roads where the speeds were changed; however, they do best represent the lower volume speed limit change highways.

Table 2.4: Control Segments for Crash Analysis

| Control Segment | Route | Trucks ${ }^{\text {a,b }}$ | Passenger Vehicles ${ }^{\text {a }}$ | Highway (Oregon Highway No.) | BMP | EMP | Length |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| C-1 | I-84 | 55 | 65 | COLUMBIA RIVER (002) | 18.25 | 87.00 | 68.8 |
| C-2 | OR-11 | 55 | 55 | OREGON-WASHINGTON (008) | 17.80 | 26.59 | 8.8 |
|  | OR-11 | 55 | 55 | OREGON-WASHINGTON (008) | 0.52 | 16.15 | 15.6 |
| C-3 | US-26 | 55 | 55 | OCHOCO (041) | 20.75 | 65.68 | 44.9 |
|  | US-26 | 55 | 55 | OCHOCO (041) | 66.88 | 98.36 | 31.5 |
|  | US-26 | 55 | 55 | JOHN DAY (005) | 123.98 | 160.28 | 36.3 |
| C-4 | US-26 | 55 | 55 | WARM SPRINGS (053) | 72.00 | 102.83 | 30.8 |
|  | US-26 | 55 | 55 | WARM SPRINGS (053) | 105.64 | 115.81 | 10.2 |
| C-5 | OR-58 | 55 | 55 | WILLAMETTE (018) | 5.80 | 33.94 | 28.1 |
|  | OR-58 | 55 | 55 | WILLAMETTE (018) | 36.49 | 86.45 | 50.0 |
| C-6 | US-20 | 55 | 55 | SANTIAM (016) | 88.20 | 99.53 | 11.3 |
|  | US-20 | 55 | 55 | MCKENZIE-BEND (017) | 0.37 | 14.47 | 14.1 |
| C-7 | OR-126 | 55 | 55 | MCKENZIE (015) | 93.38 | 110.14 | 16.8 |
| C-8 | OR-19 | 55 | 55 | JOHN DAY (005) | 1.30 | 37.50 | 36.2 |
|  | OR-19 | 55 | 55 | JOHN DAY (005) | 36.68 | 57.58 | 20.9 |
|  | OR-19 | 55 | 55 | JOHN DAY (005) | 59.64 | 91.68 | 32.0 |
| C-9 | OR-3 | 55 | 55 | ENTERPRISE-LEWISTON (011) | 0.00 | 42.42 | 42.4 |
| C-10 | OR-126 | 55 | 55 | BELTLINE (069) | 0.00 | 2.28 | 2.3 |
|  | OR-126 | 55 | 55 | FLORENCE-EUGENE (062) | 47.04 | 52.69 | 5.7 |
|  | OR-126 | 55 | 55 | FLORENCE-EUGENE (062) | 32.80 | 46.76 | 14.0 |
| C-11 | I-5 | 55 | 65 | PACIFIC (001) | 260.85 | 288.60 | 27.8 |
|  | I-5 | 55 | 65 | PACIFIC (001) | 196.00 | 251.00 | 55.0 |
| C-12 | OR-140 | 55 | 55 | KLAMATH FALLS-LAKEVIEW (020) | 5.64 | 40.53 | 34.9 |
|  | OR-140 | 55 | 55 | KLAMATH FALLS-LAKEVIEW (020) | 41.33 | 53.17 | 11.8 |
|  | OR-140 | 55 | 55 | KLAMATH FALLS-LAKEVIEW (020) | 54.18 | 95.25 | 41.1 |

${ }^{\text {a }}$ All speeds in $\mathrm{mi} / \mathrm{hr}$
${ }^{\mathrm{b}}$ Interstate speed limits were raised to $60 \mathrm{mi} / \mathrm{hr}$ on C-1 and C-11 in late 2017

### 2.2.2 Increased Speed Limit Segments

From the preliminary analysis, segments with an increase in speed limit were obtained. Sections of highways that pass through cities, towns, or congested areas were not included. The increased speed limit segments were based on the 12 segments identified in ORS 811.111. Table 2.5 shows details of these segments. A total of $1,426.5$ miles of highway were considered for analysis.

Table 2.5: Increased Speed Limit Segments for Crash Analysis

| ORS <br> Segment | Route | Trucks ${ }^{\text {a }}$ | Passenger Vehicles ${ }^{\text {a }}$ | Highway (Oregon Highway No.) | BMP | EMP | Length |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| ORS-2 | I-84 | 65 | 70 | COLUMBIA RIVER (002) | 87.00 | 167.58 | 80.6 |
|  | I-84 | 65 | 70 | OLD OREGON TRAIL (006) | 167.73 | 378.01 | 210.3 |
| ORS-3 | US-95 | 65 | 70 | I.O.N. (456) | 0.00 | 19.44 | 19.4 |
|  | US-95 | 65 | 70 | I.O.N. (456) | 21.64 | 117.65 | 96.0 |
| ORS-4 | US-20 | 60 | 65 | CENTRAL OREGON (007) | 3.17 | 42.50 | 39.3 |
|  | US-20 | 60 | 65 | CENTRAL OREGON (007) | 43.00 | 127.70 | 84.7 |
|  | US-20 | 60 | 65 | CENTRAL OREGON (007) | 132.72 | 245.48 | 112.8 |
|  | US-26/20 | 60 | 65 | CENTRAL OREGON (007) | 246.79 | 258.20 | 11.4 |
| ORS-5 | US-197 | 60 | 65 | THE DALLES-CALIFORNIA (004) | 2.32 | 42.90 | 40.6 |
|  | US-197/97 | 60 | 65 | THE DALLES-CALIFORNIA (004) | 46.35 | 89.40 | 43.1 |
|  | US-97 | 60 | 65 | THE DALLES-CALIFORNIA (004) | 98.60 | 114.90 | 16.3 |
|  | US-97 | 60 | 65 | THE DALLES-CALIFORNIA (004) | 124.40 | 133.32 | 8.9 |
|  | US-97 | 60 | 65 | THE DALLES-CALIFORNIA (004) | 142.52 | 164.19 | 21.7 |
|  | US-97 | 60 | 65 | THE DALLES-CALIFORNIA (004) | 168.80 | 183.80 | 15.0 |
|  | US-97 | 60 | 65 | THE DALLES-CALIFORNIA (004) | 187.14 | 202.45 | 15.3 |
|  | US-97 | 60 | 65 | THE DALLES-CALIFORNIA (004) | 204.23 | 209.05 | 4.8 |
|  | US-97 | 60 | 65 | THE DALLES-CALIFORNIA (004) | 209.41 | 270.50 | 61.1 |
|  | US-97 | 60 | 65 | THE DALLES-CALIFORNIA (004) | 279.04 | 291.73 | 12.7 |
| ORS-6 | OR-31 | 60 | 65 | FREMONT (019) | 0.00 | 46.58 | 46.6 |
|  | OR-31 | 60 | 65 | FREMONT (019) | 48.11 | 68.74 | 20.6 |
|  | OR-31 | 60 | 65 | FREMONT (019) | 70.50 | 97.98 | 27.5 |
|  | OR-31 | 60 | 65 | FREMONT (019) | 99.09 | 120.43 | 21.3 |
| ORS-7 | OR-78 | 60 | 65 | STEENS (442) | 0.90 | 91.60 | 90.7 |
| ORS-8 | US-395 | 60 | 65 | JOHN DAY-BURNS (048) | 2.72 | 24.55 | 21.8 |
|  | US-395 | 60 | 65 | JOHN DAY-BURNS (048) | 29.58 | 35.42 | 5.8 |
| ORS-9 | US-395 | 60 | 65 | FREMONT (019) | 120.57 | 139.68 | 19.1 |
|  | US-395 | 60 | 65 | FREMONT (019) | 145.70 | 157.24 | 11.5 |
|  | US-395 | 60 | 65 | LAKEVIEW BURNS (049) | 0.01 | 90.02 | 90.0 |
| ORS-10 | OR-205 | 60 | 65 | FRENCHGLEN (440) | 0.00 | 57.83 | 57.8 |
| ORS-11 | US-26 | 60 | 65 | JOHN DAY (005) | 163.69 | 174.51 | 10.8 |
|  | US-26 | 60 | 65 | JOHN DAY (005) | 176.04 | 211.47 | 35.4 |
|  | US-26 | 60 | 65 | JOHN DAY (005) | 213.15 | 230.55 | 17.4 |
|  | US-26 | 60 | 65 | JOHN DAY (005) | 231.33 | 253.73 | 22.4 |
|  | US-26 | 60 | 65 | JOHN DAY (005) | 255.09 | 266.26 | 11.2 |
|  | US-26 | 60 | 65 | JOHN DAY (005) | 267.34 | 277.44 | 10.1 |
| ORS-12 | I-82 | 65 | 70 | MCNARY (070) | 0.00 | 10.78 | 10.8 |

${ }^{\text {a }}$ All speeds in $\mathrm{mi} / \mathrm{hr}$

### 2.2.3 Speed Reduction Segments

In addition to assessing control locations and speed limit increase locations, a set of speed reduction segments were analyzed. Following an engineering analysis by ODOT, speed limits were lowered to $55 \mathrm{mi} / \mathrm{hr}$ for trucks and $55 \mathrm{mi} / \mathrm{hr}$ for all other vehicles in specific sections of the increased speed limit corridors.

Table 2.6: Speed Limit Reduction Segments

| Segment | Route | Trucks $^{\text {a }}$ | Passenger $^{\text {Vehicles }^{\mathbf{a}}}$ | Highway (Oregon Highway No.) | BMP | EMP | Length |
| :---: | :---: | :---: | :---: | :---: | :--- | :--- | :--- |
| 13 | US-97 | 55 | 55 | THE DALLES-CALIFORNIA (004) | 168.80 | 169.87 | 1.1 |
| 14 | US-97 | 55 | 55 | THE DALLES-CALIFORNIA (004) | 98.60 | 115.00 | 16.4 |
| 15 | US-20 | 55 | 55 | CENTRAL OREGON (007) | 3.17 | 9.19 | 6.0 |
| 16 | US-20 | 55 | 55 | CENTRAL OREGON (007) | 248.84 | 258.20 | 9.4 |

[^1]
### 3.0 UPDATED ANALYSIS

The updated analysis builds on the preliminary analysis by using additional years of after data (traffic speed, volume, and crash data after the speed limit increase became effective). Therefore, this analysis compares the changes in speed, volume, and safety performance following the previous work using these additional years of after data. Comparisons are made between the control segments (no increase) and change segments (increased speed limit). A new empiricalbayes (EB) before after safety analysis is added. EB analysis is the recommended before-after safety approach in the AASHTO Highway Safety Manual.

### 3.1 ESTIMATED SPEED CHANGE

The estimated average speed and the percentage of vehicles exceeding $65 \mathrm{mi} / \mathrm{hr}, 75 \mathrm{mi} / \mathrm{hr}$, and 85 $\mathrm{mi} / \mathrm{hr}$ were determined and tabulated for each ATRs considered for analysis. The metrics were grouped by the posted speed limit for passenger vehicles at the ATR location. Speed data were compared for before and after the speed limit increase. All months in which speed data was available from January 2015 to March 2020 were compared. For the before period, this included January 2015 to February 2016. For the after period, this included April 2016 to March 2020, where the month in which the speed limit increase was enacted was excluded (March 2016).

To assess if there was a statistical difference in mean speeds, two tests were conducted: (1) a $t$ test assuming unequal variance and (2) a paired $t$-test. The results of these tests are presented as $p$-values in the tables, where significant levels of at least $95 \%$ are indicated by highlighted cells. For the $t$-test, the null hypothesis is that the means are equal ( $\mu_{\text {Before }}=\mu_{\text {After }}$ ). If the reported $p$-value is less than the desired significance level (in this analysis, $95 \%$, or a $p$-value of 0.05 or less), the null hypothesis is rejected in favor of the alternative hypothesis that the mean speeds are not equal. A paired $t$-test was also conducted. Due to this test required that the number of observations be the same in both periods, a random sample of before and after measurements was obtained to run this test.

### 3.1.1 Estimated Change in Average Speed

### 3.1.1.1 Control Segments

A summary of analysis results for the control segments is shown in Table 3.1. Visually, Figure 3.1 shows the average monthly speed plots for all ATRs considered for analysis. As shown, about half of the control segments experienced a statistically significant change in mean speed for both $t$-tests (eight of 18), with just two ATRs seeing a change of greater than $1 \mathrm{mi} / \mathrm{hr}$ : Lava Butte and Wilsonville. Lave Butte has the highest change in speed but may be explained by its location (it is located near a segment where the speed limit was increased). The results shown in Table 3.1 may capture some carryover effect of increased speed limits in adjacent roadway sections. Overall, the average speed change for the sample is less than $1 \mathrm{mi} / \mathrm{hr}$.

Table 3.1: Estimated Average Monthly Speed (mi/hr) at Control Locations

| Speed Group | ATR | Jan. 2015 - Mar. 2020 (Excluding Mar. 2016) |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Pre | Post | Delta | Means ( $p$-value) | Paired ( $p$-value) |
| $45 \mathrm{mi} / \mathrm{hr}$ | Madras (16-002) | 46.31 | 45.98 | -0.33 | 0.027 | 0.060 |
|  | North Redmond (09-023) | 56.91 | 57.46 | 0.55 | 0.078 | 0.061 |
|  | Average |  |  | 0.11 |  |  |
| $55 \mathrm{mi} / \mathrm{hr}$ | Dayville (12-003) | 49.00 | 48.81 | -0.19 | 0.375 | 0.920 |
|  | Lava Butte (09-003) | 57.91 | 59.47 | 1.56 | 0.004 | 0.015 |
|  | Lexington (25-007) | 59.66 | 59.71 | 0.05 | 0.826 | 0.873 |
|  | Milton (30-021) | 54.61 | 54.92 | 0.30 | 0.078 | 0.900 |
|  | North Powder (01-001) | 59.56 | 59.57 | 0.01 | 0.982 | 0.578 |
|  | Noti (20-005) | 64.52 | 65.01 | 0.50 | 0.027 | 0.000 |
|  | Oakridge (20-017) | 56.34 | 56.65 | 0.31 | 0.046 | 0.013 |
|  | Rhododendron (03-006) | 53.34 | 53.20 | -0.14 | 0.813 | 0.871 |
|  | Shady Cove (15-013) | 59.94 | 60.51 | 0.57 | 0.000 | 0.001 |
|  | Shutler (11-007) | 56.78 | 57.11 | 0.33 | 0.197 | 0.426 |
|  | Sisters (09-014) | 57.75 | 58.39 | 0.65 | 0.328 | 0.944 |
|  | Average |  |  | 0.36 |  |  |
| $65 \mathrm{mi} / \mathrm{hr}$ | Cascade Locks (14-004) | 66.62 | 66.05 | -0.57 | 0.170 | 0.062 |
|  | Lake Creek (22-016) | 68.70 | 69.67 | 0.96 | 0.000 | 0.000 |
|  | North Albany (22-005) | 64.52 | 65.01 | 0.50 | 0.027 | 0.012 |
|  | Rowena (33-001) | 67.38 | 67.71 | 0.32 | 0.265 | 0.008 |
|  | Wilsonville (03-011) | 65.96 | 67.03 | 1.07 | 0.000 | 0.000 |
|  | Average |  |  | 0.46 |  |  |






Figure 3.1: Average Monthly Speed Plots for Control Segments

### 3.1.1.2 Increased Speed Limit Segments

A summary of analysis results for the control segments is shown in Table 3.2. Figure 3.2 shows the average monthly speed plots for all ATRs considered for analysis, and Figure 3.3 and Figure 3.4 show ATRs that experienced an increase in observed speed. As shown in Table 3.2, all but two locations have statistically different mean speeds, with an average mean speed increase of greater than $3 \mathrm{mi} / \mathrm{hr}$ for the sample. Just one location has a decrease; the decrease is small at 0.10 $\mathrm{mi} / \mathrm{hr}$.

Some potential outliers to discuss are Cairo Junction, New Pine Creek, and Prairie City. In the previous analysis, it was found that Cairo Junction experienced a large increase due to problems with the station in the before period; it is unclear if that remains true with the newly formatted data obtained. Secondly, New Pine Creek is in a location where speeds are lower. Lastly, when obtaining the newly formatted data, there were no monthly files for Prairie City in the before period.

Table 3.2: Estimated Average Monthly Speed (mi/hr) at Speed Limit Change Locations

| Speed Group | ATR | Jan. 2015 - Mar. 2020 (Excluding Mar. 2016) |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Pre | Post | Delta | Means ( $p$-value) | Paired ( $p$-value) |
| $65 \mathrm{mi} / \mathrm{hr}$ | Burns (13-001) | 61.29 | 63.31 | 2.01 | 0.000 | 0.002 |
|  | Cairo Junction (23-006) | 43.76 | 54.31 | 10.54 | 0.000 | 0.000 |
|  | Dufur (33-005) | 60.89 | 65.04 | 4.15 | 0.000 | 0.000 |
|  | Midland (18-019) | 59.53 | 64.68 | 5.16 | 0.000 | 0.000 |
|  | Modoc Point (18-022) | 61.88 | 65.45 | 3.57 | 0.000 | 0.000 |
|  | New Pine Creek (19-008) | 49.44 | 49.34 | -0.10 | 0.676 | 0.396 |
|  | Pilot Butte (09-005) | 60.34 | 61.07 | 0.73 | 0.064 | 0.171 |
|  | Prairie City (12-009) | - | - | - |  |  |
|  | Redmond (09-020) | 60.08 | 62.93 | 2.85 | 0.000 | 0.000 |
|  | Steens (13-007) | 52.63 | 55.71 | 3.08 | 0.000 | 0.051 |
|  | Average |  |  | 3.56 |  |  |
| $70 \mathrm{mi} / \mathrm{hr}$ | Boardman Jct (25-008) | 67.91 | 71.13 | 3.22 | 0.000 | 0.000 |
|  | Huntington (23-016) | 63.16 | 67.14 | 3.98 | 0.000 | 0.000 |
|  | North La Grande (31-007) | 65.87 | 69.40 | 3.53 | 0.000 | 0.000 |
|  | Pendleton (30-004) | 67.81 | 71.93 | 4.12 | 0.000 | 0.000 |
|  | Rufus (28-002) | 68.86 | 70.29 | 1.43 | 0.005 | - |
|  | South Baker (01-013) | 67.92 | 70.88 | 2.96 | 0.000 | 0.000 |
|  | Average |  |  | 3.21 |  |  |




Speed Increase of $55 \mathrm{mi} / \mathrm{hr}$ to $65 \mathrm{mi} / \mathrm{hr}$



Figure 3.2: Average Monthly Speed Plots for Increased Speed Limit Segments


Figure 3.3: ATRs With Speed Increase from $55 \mathrm{mi} / \mathrm{hr}$ to $65 \mathrm{mi} / \mathrm{hr}$


Figure 3.4: ATRs With Speed Increase from $65 \mathrm{mi} / \mathrm{hr}$ to $70 \mathrm{mi} / \mathrm{hr}$

### 3.1.2 Average Percentage of Vehicles per Month Exceeding Speeds

### 3.1.2.1 Control Segments

Table 3.4 shows the analysis results for the percentage of vehicles exceeding $65 \mathrm{mi} / \mathrm{hr}$. There is some variation in the results, but the majority of locations experienced a significant difference in the mean percentage of vehicles exceeding $65 \mathrm{mi} / \mathrm{hr}$ for both $t$-tests. In the $45 \mathrm{mi} / \mathrm{hr}$ group, there was no difference at Madras, while North Redmond experienced an increase of about 2.1\%. In the $55 \mathrm{mi} / \mathrm{hr}$ group, the largest difference was at Noti (difference of 6.15\%), and two locations experienced a decrease in mean percentage. Overall, however, there was an average increase of about $2.1 \%$. In the $65 \mathrm{mi} / \mathrm{hr}$ group, large proportions of vehicles exceed $65 \mathrm{mi} / \mathrm{hr}$ in both the before and after periods.

Table 3.3 shows the percentage of vehicles exceeding $75 \mathrm{mi} / \mathrm{hr}$ at the control locations. As observed in both the $45 \mathrm{mi} / \mathrm{hr}$ and $55 \mathrm{mi} / \mathrm{hr}$ speed groups, small proportions of vehicles exceeded $75 \mathrm{mi} / \mathrm{hr}$. The largest change was observed at Lave Butte (+0.66\%) and Noti (+0.55\%). However, more variation is observed in the $65 \mathrm{mi} / \mathrm{hr}$ group, with Lake Creek and Rowena experiencing a moderately high proportion of vehicles exceeding $75 \mathrm{mi} / \mathrm{hr}$. Rowena and Wilsonville experienced the highest increase from before to after. On average, a $2.44 \%$ increase was observed in the $65 \mathrm{mi} / \mathrm{hr}$ group.

Table 3.5 shows the percentage of vehicles exceeding $85 \mathrm{mi} / \mathrm{hr}$ at the control locations. As observed, a very small proportion of vehicles exceeded this speed threshold in the $45 \mathrm{mi} / \mathrm{hr}$ and $55 \mathrm{mi} / \mathrm{hr}$ groups. In the $65 \mathrm{mi} / \mathrm{hr}$ group, the highest change was again observed at Lake Creek ( $+0.33 \%$ ), but the increase, on average, of vehicles exceeding $85 \mathrm{mi} / \mathrm{hr}$ was slight at $0.12 \%$.

Table 3.3: Average Percentage of Vehicles Exceeding $75 \mathrm{mi} / \mathrm{hr}$ at Control Sections

| Speed Group | ATR |  |  |  |  |  |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: |
|  |  | Pre | Post | Delta | Means <br> $(\boldsymbol{p}$-value) $)$ | Paired <br> $(\boldsymbol{p}$-value $)$ |
| $45 \mathrm{mi} / \mathrm{hr}$ | Madras (16-002) | 0.01 | 0.01 | 0.00 | 0.366 | 0.551 |
|  | North Redmond (09-023) | 0.20 | 0.30 | 0.10 | 0.000 | 0.002 |
|  | Average |  |  | $\mathbf{0 . 0 5}$ |  |  |
| $55 \mathrm{mi} / \mathrm{hr}$ | Dayville (12-003) | 0.09 | 0.10 | 0.01 | 0.053 | 0.115 |
|  | Lava Butte (09-003) | 0.53 | 1.20 | 0.66 | 0.000 | 0.000 |
|  | Lexington (25-007) | 1.18 | 1.19 | 0.01 | 0.855 | 0.889 |
|  | Milton (30-021) | 0.07 | 0.09 | 0.02 | 0.000 | 0.000 |
|  | North Powder (01-001) | 1.08 | 1.25 | 0.17 | 0.018 | 0.057 |
|  | Noti (20-005) | 0.89 | 1.44 | 0.55 | 0.000 | 0.000 |
|  | Oakridge (20-017) | 0.13 | 0.16 | 0.04 | 0.000 | 0.008 |
|  | Rhododendron (03-006) | 0.15 | 0.17 | 0.02 | 0.271 | 0.551 |
|  | Shady Cove (15-013) | 1.13 | 1.59 | 0.46 | 0.000 | 0.007 |
|  | Shutler (11-007) | 0.76 | 1.01 | 0.25 | 0.001 | 0.051 |
|  | Sisters (09-014) | 0.25 | 0.37 | 0.12 | 0.000 | 0.004 |
|  | Average |  |  | $\mathbf{0 . 2 1}$ |  |  |
| $65 \mathrm{mi} / \mathrm{hr}$ | Cascade Locks (14-004) | 8.94 | 9.04 | 0.10 | 0.846 | 0.689 |
|  | Lake Creek (22-016) | 14.69 | 19.72 | 5.04 | 0.000 | 0.000 |
|  | North Albany (22-005) | 4.97 | 6.72 | 1.75 | 0.000 | 0.005 |
|  | Rowena (33-001) | 8.29 | 10.04 | 1.75 | 0.001 | 0.624 |
|  | Wilsonville (03-011) | 4.91 | 8.47 | 3.56 | 0.000 | 0.000 |
|  | Average |  |  | 2.44 |  |  |

Table 3.4: Average Percentage of Vehicles Exceeding $65 \mathrm{mi} / \mathrm{hr}$ at Control Sections

| Speed <br> Group | ATR | Jan. 2015 - Mar. 2020 (Excluding Mar. 2016) |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Pre | Post | Delta | Means ( $p$-value) | Paired ( $p$-value) |
| $45 \mathrm{mi} / \mathrm{hr}$ | Madras (16-002) | 0.13 | 0.13 | 0.00 | 0.830 | 0.654 |
|  | North Redmond (09-023) | 7.03 | 9.12 | 2.09 | 0.000 | 0.017 |
|  | Average |  |  | 1.04 |  |  |
| $55 \mathrm{mi} / \mathrm{hr}$ | Dayville (12-003) | 1.70 | 1.68 | -0.01 | 0.892 | 0.089 |
|  | Lava Butte (09-003) | 13.28 | 21.03 | 7.75 | 0.000 | 0.001 |
|  | Lexington (25-007) | 15.90 | 15.64 | -0.26 | 0.671 | 0.153 |
|  | Milton (30-021) | 1.32 | 1.90 | 0.57 | 0.000 | 0.000 |
|  | North Powder (01-001) | 14.29 | 15.11 | 0.82 | 0.122 | 0.940 |
|  | Noti (20-005) | 13.27 | 19.42 | 6.15 | 0.000 | 0.000 |
|  | Oakridge (20-017) | 5.08 | 5.97 | 0.88 | 0.000 | 0.001 |
|  | Rhododendron (03-006) | 3.12 | 3.46 | 0.34 | 0.330 | 0.473 |
|  | Shady Cove (15-013) | 15.27 | 18.55 | 3.29 | 0.000 | 0.000 |
|  | Shutler (11-007) | 10.00 | 11.11 | 1.11 | 0.030 | 0.019 |
|  | Sisters (09-014) | 8.63 | 10.98 | 2.35 | 0.005 | 0.005 |
|  | Average |  |  | 2.09 |  |  |
| $65 \mathrm{mi} / \mathrm{hr}$ | Cascade Locks (14-004) | 65.72 | 62.96 | -2.75 | 0.138 | 0.267 |
|  | Lake Creek (22-016) | 72.55 | 75.53 | 2.97 | 0.000 | 0.001 |
|  | North Albany (22-005) | 51.84 | 54.48 | 2.64 | 0.016 | 0.028 |
|  | Rowena (33-001) | 67.62 | 69.58 | 1.96 | 0.183 | 0.917 |
|  | Wilsonville (03-011) | 60.79 | 68.05 | 7.25 | 0.000 | 0.000 |
|  | Average |  |  | 2.41 |  |  |

Table 3.5: Average Percentage of Vehicles Exceeding $85 \mathrm{mi} / \mathrm{hr}$ at Control Sections

| Speed Group | ATR | Jan. 2015-Mar. 2020 (Excluding Mar. 2016) |  |  |  |  |
| :---: | :--- | :--- | :---: | :---: | :---: | :---: |
|  |  | Pre | Post | Delta | Means <br> (p-value) | Paired <br> $(\boldsymbol{p}$-value) |
| $45 \mathrm{mi} / \mathrm{hr}$ | Madras (16-002) | 0.00 | 0.00 | 0.00 | 0.258 | 0.464 |
|  | North Redmond (09-023) | 0.02 | 0.03 | 0.01 | 0.000 | 0.016 |
|  | Average |  |  | $\mathbf{0 . 0 0}$ |  |  |
| $55 \mathrm{mi} / \mathrm{hr}$ | Dayville (12-003) | 0.09 | 0.10 | 0.01 | 0.053 | 0.153 |
|  | Lava Butte (09-003) | 0.05 | 0.08 | 0.03 | 0.000 | 0.000 |
|  | Lexington (25-007) | 0.14 | 0.15 | 0.01 | 0.451 | 0.355 |
|  | Milton (30-021) | 0.01 | 0.02 | 0.00 | 0.062 | 0.410 |
|  | North Powder (01-001) | 0.13 | 0.16 | 0.02 | 0.145 | 0.282 |
|  | Noti (20-005) | 0.11 | 0.16 | 0.05 | 0.000 | 0.002 |
|  | Oakridge (20-017) | 0.01 | 0.01 | 0.00 | 0.368 | 0.413 |
|  | Rhododendron (03-006) | 0.02 | 0.02 | 0.00 | 0.563 | 0.774 |
|  | Shady Cove (15-013) | 0.13 | 0.17 | 0.05 | 0.000 | 0.008 |
|  | Shutler (11-007) | 0.10 | 0.14 | 0.04 | 0.018 | 0.014 |
|  | Sisters (09-014) | 0.02 | 0.02 | 0.01 | 0.019 | 0.411 |
|  | Average |  |  | $\mathbf{0 . 0 2}$ |  |  |
| $65 \mathrm{mi} / \mathrm{hr}$ | Cascade Locks (14-004) | 0.30 | 0.29 | -0.01 | 0.677 | 0.099 |
|  | Lake Creek (22-016) | 0.50 | 0.82 | 0.33 | 0.000 | 0.000 |
|  | North Albany (22-005) | 0.16 | 0.22 | 0.07 | 0.000 | 0.000 |
|  | Rowena (33-001) | 0.27 | 0.32 | 0.05 | 0.141 | 0.121 |
|  | Wilsonville (03-011) | 0.27 | 0.41 | 0.15 | 0.000 | 0.003 |
|  | Average |  |  | $\mathbf{0 . 1 2}$ |  |  |

### 3.1.2.2 Increased Speed Limit Segments

The percentage of vehicles exceeding $65 \mathrm{mi} / \mathrm{hr}, 75 \mathrm{mi} / \mathrm{hr}$, and $85 \mathrm{mi} / \mathrm{hr}$ increased at each location in the increased speed limit sections. These changes in percentages are important because small changes in average speeds (measured in $\mathrm{mi} / \mathrm{hr}$ ) do not necessarily provide insight into changes in speed distributions. Results for the percentage of vehicles exceeding these speed thresholds are given in Table 3.6 ( $65 \mathrm{mi} / \mathrm{hr}$ ), Table 3.7 ( $75 \mathrm{mi} / \mathrm{hr}$ ), and Table 3.8 ( $85 \mathrm{mi} / \mathrm{hr}$ ).

All differences were statistically significant in this group, with well over 99\% confidence. In locations where the speed limit increased from $55 \mathrm{mi} / \mathrm{hr}$ to $65 \mathrm{mi} / \mathrm{hr}$ (for passenger vehicles), the percentage of vehicle exceeding $65 \mathrm{mi} / \mathrm{hr}$ increased, on average, about $16.1 \%$. In locations where the speed limit increased from $65 \mathrm{mi} / \mathrm{hr}$ to $70 \mathrm{mi} / \mathrm{hr}$ (for passenger vehicles), the percentage of vehicles exceeding $65 \mathrm{mi} / \mathrm{hr}$ increased, on average, nearly $17 \%$. In this group, all differences were statistically different, with well over $99 \%$ confidence.

Vehicles exceeding $75 \mathrm{mi} / \mathrm{hr}$ at locations where the speed limit increased from $55 \mathrm{mi} / \mathrm{hr}$ to 65 $\mathrm{mi} / \mathrm{hr}$ the percentage increased, on average, approximately $1.9 \%$. At the $65 \mathrm{mi} / \mathrm{hr}$ to $70 \mathrm{mi} / \mathrm{hr}$ increase segments, the percentage of vehicles exceeding $75 \mathrm{mi} / \mathrm{hr}$ increased, on average, nearly $13 \%$. All differences were statistically significant with over $99 \%$ confidence, except for New Pine Creek ATR.

In locations where the speed limit increased from $55 \mathrm{mi} / \mathrm{hr}$ to $65 \mathrm{mi} / \mathrm{hr}$, the percentage of vehicles exceeding $85 \mathrm{mi} / \mathrm{hr}$ increased, on average, $0.50 \%$. In this group, all but three differences
were statistically significant with a high level of confidence (Cairo Junction, New Pine Creek, and Pilot Butte). In locations where the speed limit increased from $65 \mathrm{mi} / \mathrm{hr}$ to $70 \mathrm{mi} / \mathrm{hr}$, the percentage of vehicles exceeding $85 \mathrm{mi} / \mathrm{hr}$ increased, on average, $1 \%$. With the exception of the paired test using random samples at South Baker, all differences were statistically significant with well over 99\% confidence.

Table 3.6: Average Percentage of Vehicles Exceeding $65 \mathrm{mi} / \mathrm{hr}$ at Change Locations

| Speed Group | ATR | Jan. 2015 - Mar. 2020 (Excluding Mar. 2016) |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Pre | Post | Delta | Means ( $p$-value) | Paired ( $p$-value) |
| $65 \mathrm{mi} / \mathrm{hr}$ | Burns (13-001) | 29.87 | 44.48 | 14.61 | 0.000 | 0.000 |
|  | Cairo Junction (23-006) | 0.49 | 2.20 | 1.71 | 0.000 | 0.001 |
|  | Dufur (33-005) | 19.97 | 54.33 | 34.35 | 0.000 | 0.000 |
|  | Midland (18-019) | 15.96 | 47.66 | 31.69 | 0.000 | 0.000 |
|  | Modoc Point (18-022) | 23.60 | 53.81 | 30.21 | 0.000 | 0.000 |
|  | New Pine Creek (19-008) | 2.02 | 2.44 | 0.43 | 0.001 | 0.008 |
|  | Pilot Butte (09-005) | 20.77 | 26.59 | 5.82 | 0.000 | 0.001 |
|  | Prairie City (12-009) | - | - | - |  |  |
|  | Redmond (09-020) | 24.01 | 39.82 | 15.81 | 0.000 | 0.000 |
|  | Steens (13-007) | 25.34 | 35.52 | 10.18 | 0.000 | 0.000 |
|  | Average |  |  | 16.09 |  |  |
| $70 \mathrm{mi} / \mathrm{hr}$ | Boardman Jct (25-008) | 69.86 | 83.15 | 13.29 | 0.000 | 0.000 |
|  | Huntington (23-016) | 47.32 | 64.99 | 17.67 | 0.000 | 0.001 |
|  | North La Grande (31-007) | 52.43 | 73.65 | 21.22 | 0.000 | 0.000 |
|  | Pendleton (30-004) | 69.29 | 88.78 | 19.49 | 0.000 | 0.000 |
|  | Rufus (28-002) | 69.32 | 79.64 | 10.33 | 0.001 | - |
|  | South Baker (01-013) | 62.44 | 81.54 | 19.10 | 0.000 | 0.000 |
|  | Average |  |  | 16.85 |  |  |

Table 3.7: Average Percentage of Vehicles Exceeding $75 \mathrm{mi} / \mathrm{hr}$ at Change Locations

| Speed Group | ATR | Jan. 2015 - Mar. 2020 (Excluding Mar. 2016) |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Pre | Post | Delta | Means ( $p$-value) | Paired ( $p$-value) |
| $65 \mathrm{mi} / \mathrm{hr}$ | Burns (13-001) | 3.23 | 4.99 | 1.76 | 0.000 | 0.011 |
|  | Cairo Junction (23-006) | 0.01 | 0.06 | 0.05 | 0.000 | 0.000 |
|  | Dufur (33-005) | 1.77 | 4.06 | 2.29 | 0.000 | 0.000 |
|  | Midland (18-019) | 1.26 | 5.53 | 4.27 | 0.000 | 0.012 |
|  | Modoc Point (18-022) | 1.43 | 5.20 | 3.77 | 0.000 | 0.000 |
|  | New Pine Creek (19-008) | 0.16 | 0.18 | 0.01 | 0.321 | 0.184 |
|  | Pilot Butte (09-005) | 1.27 | 1.72 | 0.45 | 0.000 | 0.020 |
|  | Prairie City (12-009) | - | - | - | - | - |
|  | Redmond (09-020) | 0.51 | 1.95 | 1.44 | 0.000 | 0.000 |
|  | Steens (13-007) | 3.72 | 6.86 | 3.14 | 0.000 | 0.000 |
|  | Average |  |  | 1.91 |  |  |
| $70 \mathrm{mi} / \mathrm{hr}$ | Boardman Jct (25-008) | 10.94 | 28.32 | 17.38 | 0.000 | 0.000 |
|  | Huntington (23-016) | 4.21 | 13.73 | 9.51 | 0.000 | 0.000 |
|  | North La Grande (31-007) | 10.93 | 22.67 | 11.74 | 0.000 | 0.003 |
|  | Pendleton (30-004) | 8.78 | 30.76 | 21.98 | 0.000 | 0.000 |
|  | Rufus (28-002) | 16.92 | 24.40 | 7.48 | 0.001 | - |
|  | South Baker (01-013) | 17.46 | 27.14 | 9.68 | 0.000 | 0.000 |
|  | Average |  |  | 12.96 |  |  |

Table 3.8: Average Percentage of Vehicles Exceeding $85 \mathrm{mi} / \mathrm{hr}$ at Change Locations

| Speed Group | ATR |  |  |  |  | Jan. 2015-Mar. 2020 (Excluding Mar. 2016) |  |  |  |
| :---: | :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Pre | Post | Delta | Means <br> (p-value) | Paired <br> ( $\boldsymbol{p}$-value) |  |  |  |
| $65 \mathrm{mi} / \mathrm{hr}$ | Burns (13-001) | 0.28 | 0.42 | 0.14 | 0.000 | 0.001 |  |  |  |
|  | Cairo Junction (23-006) | 0.01 | 0.01 | 0.00 | 0.996 | 0.559 |  |  |  |
|  | Dufur (33-005) | 0.22 | 0.37 | 0.14 | 0.000 | 0.001 |  |  |  |
|  | Midland (18-019) | 0.11 | 1.00 | 0.90 | 0.013 | 0.103 |  |  |  |
|  | Modoc Point (18-022) | 0.11 | 0.28 | 0.17 | 0.000 | 0.000 |  |  |  |
|  | New Pine Creek (19-008) | 0.02 | 0.02 | 0.00 | 0.747 | 0.742 |  |  |  |
|  | Pilot Butte (09-005) | 0.15 | 0.15 | 0.00 | 0.807 | 0.930 |  |  |  |
|  | Prairie City (12-009) | - | - | - | - | - |  |  |  |
|  | Redmond (09-020) | 0.03 | 0.08 | 0.04 | 0.000 | 0.000 |  |  |  |
|  | Steens (13-007) | 3.72 | 6.86 | 3.14 | 0.000 | 0.000 |  |  |  |
|  | Average |  |  | $\mathbf{0 . 5 0}$ |  |  |  |  |  |
| $70 \mathrm{mi} / \mathrm{hr}$ | Boardman Jct (25-008) | 0.52 | 1.17 | 0.64 | 0.000 | 0.000 |  |  |  |
|  | Huntington (23-016) | 0.22 | 0.61 | 0.40 | 0.000 | 0.000 |  |  |  |
|  | North La Grande (31-007) | 0.72 | 2.96 | 2.24 | 0.000 | 0.001 |  |  |  |
|  | Pendleton (30-004) | 0.41 | 1.07 | 0.66 | 0.000 | 0.000 |  |  |  |
|  | Rufus (28-002) | 0.80 | 1.10 | 0.30 | 0.006 | - |  |  |  |
|  | South Baker (01-013) | 2.16 | 3.98 | 1.82 | 0.001 | 0.178 |  |  |  |
|  | Average |  |  | $\mathbf{1 . 0 1}$ |  |  |  |  |  |

### 3.2 ESTIMATED VOLUME CHANGE

An analysis of total monthly traffic volume was conducted to establish traffic volume changes. The volume data are presented as 100,000 vehicles. The index column represents the postaverage volume divided by the pre-average volume (an index value of 1.1 would indicate a $10 \%$ increase in traffic volume).

### 3.2.1 Control Segments

Table 3.9 shows the before and after traffic volumes at the control locations according to the speed data. Regardless of speed group, the trend is increased volume in the after period. In both the $45 \mathrm{mi} / \mathrm{hr}$ and $55 \mathrm{mi} / \mathrm{hr}$ speed groups, the average index is approximately 1.1 (an estimated $10 \%$ increase in traffic volume). In these two groups, the differences were not statistically significant for the majority of locations. In the $65 \mathrm{mi} / \mathrm{hr}$ group, volume increases were marginal, with an average index of 1.06 (an approximate $6 \%$ increase in traffic volume). In this group, no differences were statistically significant.

Table 3.9: Estimated Average Monthly Volume (in 100,000) at Control Locations

| Speed Group | ATR | Jan. 2015-Mar. 2020 (Excluding Mar. 2016) |  |  |  |  |
| :--- | :--- | :--- | :---: | :---: | :---: | :---: |
|  |  | Pre | Post | Delta | Means <br> (p-value) | Paired <br> (p-value) |
| $45 \mathrm{mi} / \mathrm{hr}$ | Madras (16-002) | 4.09 | 4.46 | 1.09 | 0.078 | 0.064 |
|  | North Redmond (09-023) | 6.23 | 6.99 | 1.12 | 0.006 | 0.067 |
|  | Average |  |  | $\mathbf{1 . 1 1}$ |  |  |
| $55 \mathrm{mi} / \mathrm{hr}$ | Dayville (12-003) | 0.27 | 0.29 | 1.07 | 0.461 | 0.131 |
|  | Lava Butte (09-003) | 7.18 | 7.71 | 1.07 | 0.300 | 0.212 |
|  | Lexington (25-007) | 0.41 | 0.43 | 1.03 | 0.376 | 0.792 |
|  | Milton (30-021) | 4.42 | 4.62 | 1.05 | 0.085 | 0.255 |
|  | North Powder (01-001) | 0.25 | 0.25 | 1.03 | 0.510 | 0.279 |
|  | Noti (20-005) | 1.88 | 1.96 | 1.05 | 0.423 | 0.978 |
|  | Oakridge (20-017) | 0.83 | 0.96 | 1.16 | 0.162 | 0.436 |
|  | Rhododendron (03-006) | 2.60 | 3.07 | 1.18 | 0.042 | 0.141 |
|  | Shady Cove (15-013) | 2.34 | 2.59 | 1.10 | 0.032 | 0.173 |
|  | Shutler (11-007) | 0.19 | 0.24 | 1.24 | 0.000 | 0.001 |
|  | Sisters (09-014) | 2.55 | 2.65 | 1.04 | 0.717 | 0.279 |
|  | Average |  |  | $\mathbf{1 . 0 9}$ |  |  |
| $65 \mathrm{mi} / \mathrm{hr}$ | Cascade Locks (14-004) | 6.57 | 6.98 | 1.06 | 0.482 | 0.668 |
|  | Lake Creek (22-016) | 11.88 | 12.15 | 1.02 | 0.635 | 0.098 |
|  | North Albany (22-005) | 19.26 | 20.41 | 1.06 | 0.112 | 0.999 |
|  | Rowena (33-001) | 6.59 | 7.17 | 1.09 | 0.121 | 0.200 |
|  | Wilsonville (03-011) | 27.47 | 29.05 | 1.06 | 0.069 | 0.217 |
|  | Average |  |  | $\mathbf{1 . 0 6}$ |  |  |

### 3.2.2 Increased Speed Limit Segments

Table 3.10 shows the before and after traffic volumes at the speed limit increase locations according to the speed data. Once more, the general trend is an increase in traffic volume in the after period. In both the $65 \mathrm{mi} / \mathrm{hr}$ speed group, the average index is approximately 1.1 (an estimated $10 \%$ increase in traffic volume). The difference is statistically significant for about half of the locations in this group. In the $70 \mathrm{mi} / \mathrm{hr}$ group, volume increases were similar to that of the $65 \mathrm{mi} / \mathrm{hr}$ group, with an average index of 1.1 (an approximate $10 \%$ increase in traffic volume). In this group, the majority of locations did not experience a statistically significant difference in traffic volume.

Table 3.10: Estimated Average Monthly Volume (in 100,000) at Change Locations

| Speed Group | ATR | Jan. 2015-Mar. 2020 (Excluding Mar. 2016) |  |  |  |  |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: |
|  |  | Pre | Post | Delta | Means ( $\boldsymbol{p}$-value) | Paired $(\boldsymbol{p}$-value) |
| $65 \mathrm{mi} / \mathrm{hr}$ | Burns (13-001) | 0.14 | 0.16 | 1.11 | 0.261 | 0.419 |
|  | Cairo Junction (23-006) | 1.43 | 1.62 | 1.13 | 0.025 | 0.279 |
|  | Dufur (33-005) | 0.72 | 0.86 | 1.18 | 0.001 | 0.031 |
|  | Midland (18-019) | 1.12 | 1.19 | 1.06 | 0.454 | 0.260 |
|  | Modoc Point (18-022) | 1.77 | 1.86 | 1.05 | 0.577 | 0.828 |
|  | New Pine Creek (19-008) | 0.25 | 0.26 | 1.05 | 0.515 | 0.366 |
|  | Pilot Butte (09-005) | 0.80 | 0.90 | 1.13 | 0.041 | 0.185 |
|  | Prairie City (12-009) | - | - | - | - |  |
|  | Redmond (09-020) | 9.28 | 10.46 | 1.13 | 0.001 | 0.002 |
|  | Steens (13-007) | 0.45 | 0.50 | 1.13 | 0.017 | 0.173 |
|  | Average |  |  | $\mathbf{1 . 1 1}$ |  |  |
| $70 \mathrm{mi} / \mathrm{hr}$ | Boardman Jct (25-008) | 4.63 | 5.15 | 1.11 | 0.074 | 0.503 |
|  | Huntington (23-016) | 2.82 | 3.04 | 1.08 | 0.386 | 0.028 |
|  | North La Grande (31-007) | 2.83 | 3.16 | 1.11 | 0.098 | 0.073 |
|  | Pendleton (30-004) | 4.68 | 5.17 | 1.10 | 0.055 | 0.003 |
|  | Rufus (28-002) | 3.33 | 3.58 | 1.08 | 0.480 | - |
|  | South Baker (01-013) | 2.73 | 3.10 | 1.13 | 0.064 | 0.210 |
|  | Average |  |  | $\mathbf{1 . 1 0}$ |  |  |

### 3.3 DESCRIPTIVE CRASH ANALYSIS

Two crash analyses were conducted: (1) descriptive analysis based on crash frequencies and (2) Empirical Bayes analysis to assess the impact of the increased speed limits on the expected number of crashes. For this analysis, three years of before data and three years of after data were considered, excluding the month the increased speed limits were implemented (March 2016).

The crash data was aggregated to monthly crash counts in the control and speed increase segments. Counts considered include the total number of crashes, the number of fatal and injury A crashes, the number of truck-involved crashes, and the number of crashes by crash type. To ensure the before and after periods are equal in months, the following aggregations were used:

- Before
o March 2013 to February 2014 (2013-2014)
o March 2014 to February 2015 (2014-2015)
o March 2015 to February 2016 (2015-2016)
- After
o April 2016 to March 2017 (2016-2017)
o April 2017 to March 2018 (2017-2018)
o April 2018 to March 2019 (2018-2019)

The three-year before average and three-year after averages are presented in the succeeding tables. The "Delta" column indicates the difference in the number of crashes, where a negative
value represents a decrease in crashes from before to after the speed limit increase. The index value presented divides the three-year after average by the three-year before average. For each group of segments (by speed group), a subtotal row is presented with a corresponding index value for the subtotals. A total row is also provided, representing the total number of crashes in a given year. An index value is computed for the total values as well. An index less than 1.0 indicates a decrease in the number of crashes, while an index greater than 1.0 indicates an increase in crashes. No statistical tests were conducted for this descriptive analysis as this simple analysis of crash frequencies is not robust.

### 3.3.1 Total Crashes

The total number of crashes are presented for the control segments, increased speed limit segments, and the speed limit reduction segments. These crashes include only mainline crashes within the milepost boundaries presented in Table 2.4, Table 2.5, and Table 2.6. To ensure this, a visual inspection of all crashes was conducted.

### 3.3.1.1 Control Segments

Table 3.11 shows the total number of crashes by segment for the control locations. In the 65 $\mathrm{mi} / \mathrm{hr}$ group, an index value of 1.27 was computed, and the three-year average of crash counts in the after period increased by 216 . In the $55 \mathrm{mi} / \mathrm{hr}$ group, an index value of 1.11 was computed and the three-year average of crash counts in the after period increased by 45 . Figure 3.5 shows the total number of crashes for all control locations by year and posted speed limit.

Table 3.11: Total Crashes (Before-to-After) on Control Segments

| Speed Group | Control Segment | Route | $\begin{aligned} & \hline 2013 \\ & 2014 \end{aligned}$ | $\begin{gathered} \hline 2014- \\ 2015 \end{gathered}$ | $\begin{array}{r} 2015 \\ 2016 \end{array}$ | 3-yr Average | $\begin{aligned} & \hline 2016 \\ & 2017 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline 2017 \\ & 2018 \end{aligned}$ | $\begin{aligned} & \hline 2018- \\ & 2019 \end{aligned}$ | 3-yr Average | $\begin{gathered} \hline \text { 3-yr } \\ \text { Index } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $65 \mathrm{mi} / \mathrm{hr}$ | C-1 | I-84 | 198 | 231 | 229 | 219 | 269 | 242 | 191 | 234 | 1.07 |
|  | C-11 | I-5 | 503 | 534 | 697 | 578 | 800 | 831 | 706 | 779 | 1.35 |
|  | Subtotal |  | 701 | 765 | 926 | 797 | 1,069 | 1,073 | 897 | 1,013 | 1.27 |
| $55 \mathrm{mi} / \mathrm{hr}$ | C-2 | OR-11 | 20 | 29 | 17 | 22 | 35 | 23 | 25 | 28 | 1.26 |
|  | C-3 | US-26 | 31 | 35 | 63 | 43 | 54 | 46 | 37 | 46 | 1.06 |
|  | C-4 | US-26 | 18 | 32 | 47 | 32 | 31 | 58 | 33 | 41 | 1.26 |
|  | C-5 | OR-58 | 79 | 95 | 90 | 88 | 117 | 92 | 109 | 106 | 1.20 |
|  | C-6 | US-20 | 53 | 48 | 66 | 56 | 53 | 48 | 62 | 54 | 0.98 |
|  | C-7 | OR-126 | 27 | 36 | 31 | 31 | 33 | 27 | 34 | 31 | 1.00 |
|  | C-8 | OR-19 | 11 | 16 | 11 | 13 | 10 | 14 | 8 | 11 | 0.84 |
|  | C-9 | OR-3 | 5 | 10 | 4 | 6 | 5 | 3 | 10 | 6 | 0.95 |
|  | C-10 | OR-126 | 70 | 53 | 76 | 66 | 77 | 78 | 66 | 74 | 1.11 |
|  | C-12 | OR-140 | 58 | 46 | 54 | 53 | 47 | 72 | 59 | 59 | 1.13 |
|  | Sub | tal | 372 | 400 | 459 | 410 | 462 | 461 | 443 | 455 | 1.11 |
|  | Total |  | 1,073 | 1,165 | 1,385 | 1,208 | 1,531 | 1,534 | 1,340 | 1,468 | 1.22 |



Figure 3.5: Total Crashes by Year and Speed Limit Group at Control Locations

### 3.3.1.2 Increased Speed Limit Segments

Table 3.12 shows the total number of crashes by segment for the increased speed limit locations. In the group where the speed limit was increased to $70 \mathrm{mi} / \mathrm{hr}$, an index value of 1.34 was computed and the three-year average of crash counts in the after period increased by 172. The index computed for crash counts is much higher than that of the index computed for volume changes (1.1). I-84 experienced the highest increases in crash counts in the after years. In the group where the speed limit was increased to $65 \mathrm{mi} / \mathrm{hr}$, an index value of 1.27 was computed and the three-year average of crash counts in the after period increased by 142. The computed crash index was greater than the index computed for volume changes (1.1). Figure 3.6 shows the total number of crashes for all control locations by year and posted speed limit.

Table 3.12: Total Crashes (Before-to-After) on Change Segments

| Speed <br> Group | Change Segment | Route | $\begin{array}{r} 2013- \\ 2014 \\ \hline \end{array}$ | $\begin{array}{r} 2014- \\ 2015 \\ \hline \end{array}$ | $\begin{array}{r} \hline 2015- \\ 2016 \\ \hline \end{array}$ | 3-yr <br> Average | $\begin{gathered} \hline \text { 2016- } \\ \hline \end{gathered}$ | $\begin{aligned} & \hline 2017 \\ & 2018 \\ & \hline \end{aligned}$ | $\begin{gathered} 2018- \\ 2019 \\ \hline \end{gathered}$ | 3-yr <br> Average | $\begin{gathered} \text { 3-yr } \\ \text { Index } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{gathered} 70 \\ \mathrm{mi} / \mathrm{hr} \end{gathered}$ | ORS-2 | I-84 | 424 | 471 | 466 | 454 | 789 | 503 | 482 | 591 | 1.30 |
|  | ORS-3 | US-95 | 23 | 41 | 46 | 37 | 68 | 61 | 47 | 59 | 1.60 |
|  | ORS-12 | I-82 | 12 | 22 | 20 | 18 | 48 | 22 | 19 | 30 | 1.65 |
|  | Subtotal |  | 459 | 534 | 532 | 508 | 905 | 586 | 548 | 680 | 1.34 |
| $\begin{gathered} 65 \\ \mathrm{mi} / \mathrm{hr} \end{gathered}$ | ORS-4 | US-26/US-20 | 106 | 119 | 139 | 121 | 166 | 145 | 141 | 151 | 1.24 |
|  | ORS-5 | US-97 | 260 | 287 | 368 | 305 | 426 | 370 | 358 | 385 | 1.26 |
|  | ORS-6 | OR-31 | 14 | 28 | 29 | 24 | 40 | 27 | 30 | 32 | 1.37 |
|  | ORS-7 | OR-78 | 6 | 17 | 12 | 12 | 22 | 31 | 21 | 25 | 2.11 |
|  | ORS-8 | US-395 | 10 | 5 | 5 | 7 | 9 | 12 | 6 | 9 | 1.35 |
|  | ORS-9 | US-395 | 20 | 12 | 18 | 17 | 36 | 24 | 19 | 26 | 1.58 |
|  | ORS-10 | OR-205 | 9 | 8 | 5 | 7 | 5 | 9 | 5 | 6 | 0.86 |
|  | ORS-11 | US-26 | 24 | 32 | 30 | 29 | 23 | 37 | 26 | 29 | 1.00 |
|  | Subtotal |  | 449 | 508 | 606 | 521 | 727 | 655 | 606 | 663 | 1.27 |
| Total |  |  | 908 | 1,042 | 1,138 | 1,029 | 1,632 | 1,241 | 1,154 | 1,342 | 1.30 |



Figure 3.6: Total Crashes by Year and Speed Limit Group at Increased Speed Limit Locations

### 3.3.1.3 Temporary Speed Limit Reduction Segments

Table 3.13 shows the total number of crashes by segment for the speed reduction locations. Compared to the control and speed limit increase segments, these segments are short and experience fewer crashes. The computed index is 1.22 , but it should be noted that one of the US97 segments and one of the US-20 segments have indices of 0.55 and 1.02 , respectively.

Table 3.13: Total Crashes (Before-to-After) on Reduction Segments

| Reduction <br> Segment | Route | $\mathbf{2 0 1 3 -}$ <br> $\mathbf{2 0 1 4}$ | $\mathbf{2 0 1 4 -}$ <br> $\mathbf{2 0 1 5}$ | $\mathbf{2 0 1 5}$ <br> $\mathbf{2 0 1 6}$ | 3-yr <br> Average | $\mathbf{2 0 1 6}$ <br> $\mathbf{2 0 1 7}$ | $\mathbf{2 0 1 7}$ <br> $\mathbf{2 0 1 8}$ | $\mathbf{2 0 1 8}-$ <br> $\mathbf{2 0 1 9}$ | 3-yr <br> Average | 3-yr <br> Index |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 13 | US-97 | 7 | 2 | 2 | 4 | 1 | 4 | 1 | 2 | 0.55 |
| 14 | US-97 | 27 | 41 | 24 | 31 | 35 | 46 | 41 | 41 | 1.33 |
| 15 | US-20 | 11 | 17 | 8 | 12 | 13 | 18 | 20 | 17 | 1.42 |
| 16 | US-20 | 14 | 13 | 22 | 16 | 22 | 13 | 15 | 17 | 1.02 |
| Total |  | $\mathbf{5 9}$ | $\mathbf{7 3}$ | $\mathbf{5 6}$ | $\mathbf{6 3}$ | $\mathbf{7 1}$ | $\mathbf{8 1}$ | $\mathbf{7 7}$ | $\mathbf{7 6}$ | $\mathbf{1 . 2 2}$ |

### 3.3.2 Fatal and Injury A Crashes

Fatal and injury A crashes are presented for the control segments, increased speed limit segments, and the speed limit reduction segments. These crashes include only mainline crashes that occurred within the milepost boundaries presented in Table 2.4, Table 2.5, and Table 2.6. To ensure this, a visual inspection of all crashes was conducted.

### 3.3.2.1 Control Segments

Table 3.14 shows the total number of fatal and injury A (severe) crashes by segment for the control locations. In the $65 \mathrm{mi} / \mathrm{hr}$ group, an index value of 1.17 was computed and the three-year average of fatal/injury A crashes in the after period increased by 3 . It should be noted that the index for I-84 was 1.0; therefore, the average index is capturing the increase along the I-5 segment (index of 1.23 ). In the $55 \mathrm{mi} /$ hr group, an index value of 1.16 was computed and the three-year average of fatal/injury A crashes in the after period increased by 5 . Figure 3.7 shows the total number of fatal and injury A crashes for all control locations by year and posted speed limit.

Table 3.14: Fatal and Injury A Crashes (Before-to-After) on Control Segments

| Speed Group | Control Segment | Route | $\begin{array}{r} \hline 2013- \\ 2014 \\ \hline \end{array}$ | $\begin{array}{r} \hline 2014- \\ 2015 \\ \hline \end{array}$ | $\begin{gathered} \hline 2015- \\ 2016 \\ \hline \end{gathered}$ | 3-yr <br> Average | $\begin{array}{r} \hline 2016- \\ 2017 \\ \hline \end{array}$ | $\begin{aligned} & \hline 2017- \\ & 2018 \\ & \hline \end{aligned}$ | $\begin{gathered} \hline 2018- \\ 2019 \\ \hline \end{gathered}$ | 3-yr <br> Average | $\begin{gathered} \hline 3-\mathrm{yr} \\ \text { Index } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{gathered} 65 \\ \mathrm{mi} / \mathrm{hr} \end{gathered}$ | C-1 | I-84 | 7 | 4 | 5 | 5 | 8 | 4 | 4 | 5 | 1.00 |
|  | C-11 | I-5 | 12 | 17 | 14 | 14 | 19 | 15 | 19 | 18 | 1.23 |
|  | Subtotal |  | 19 | 21 | 19 | 20 | 27 | 19 | 23 | 23 | 1.17 |
| $\begin{gathered} 55 \\ \mathrm{mi} / \mathrm{hr} \end{gathered}$ | C-2 | OR-11 | 1 | 5 | 0 | 2 | 3 | 0 | 3 | 2 | 1.00 |
|  | C-3 | US-26 | 3 | 4 | 5 | 4 | 5 | 3 | 0 | 3 | 0.67 |
|  | C-4 | US-26 | 1 | 4 | 4 | 3 | 2 | 2 | 7 | 4 | 1.22 |
|  | C-5 | OR-58 | 8 | 5 | 7 | 7 | 11 | 13 | 10 | 11 | 1.70 |
|  | C-6 | US-20 | 4 | 3 | 6 | 4 | 5 | 3 | 3 | 4 | 0.85 |
|  | C-7 | OR-126 | 2 | 3 | 0 | 2 | 3 | 3 | 3 | 3 | 1.80 |
|  | C-8 | OR-19 | 2 | 2 | 1 | 2 | 2 | 2 | 0 | 1 | 0.80 |
|  | C-9 | OR-3 | 2 | 1 | 1 | 1 | 0 | 0 | 3 | 1 | 0.75 |
|  | C-10 | OR-126 | 9 | 4 | 7 | 7 | 7 | 7 | 6 | 7 | 1.00 |
|  | C-12 | OR-140 | 6 | 3 | 1 | 3 | 4 | 5 | 6 | 5 | 1.50 |
|  | Subtotal |  | 38 | 34 | 32 | 35 | 42 | 38 | 41 | 40 | 1.16 |
|  | Total |  | 57 | 55 | 51 | 54 | 69 | 57 | 64 | 63 | 1.17 |



Figure 3.7: Fatal and Injury A Crashes by Year and Speed Limit Group at Control Locations

### 3.3.2.2 Increased Speed Limit Segments

Table 3.15 shows the total number of fatal and injury A crashes by segment for the increased speed limit locations. In the group where the speed limit was increased to $70 \mathrm{mi} / \mathrm{hr}$, an index value of 1.20 was computed and the three-year average of fatal/injury A crashes in the after period increased by 5. I-82 experienced the highest increase with an index of 2.0. In the group where the speed limit was increased to $65 \mathrm{mi} / \mathrm{hr}$, an index value of 1.55 was computed and the three-year average of fatal/injury A crashes in the after period increased by 16. OR-78 (index of 4.5), US-395 (index of 2.0), and OR-205 (index of 2.0) experienced the highest increase in fatal/injury A crashes. Figure 3.8 shows the total number of fatal and injury A crashes for all speed limit increase locations by year and posted speed limit.

Table 3.15: Fatal and Injury A Crashes (Before-to-After) on Change Segments

| Speed <br> Group | Change Segment | Route | $\begin{array}{r} 2013- \\ 2014 \\ \hline \end{array}$ | $\begin{array}{r} 2014 \\ 2015 \\ \hline \end{array}$ | $\begin{gathered} \hline 2015- \\ 2016 \\ \hline \end{gathered}$ | 3-yr <br> Average | $\begin{array}{r} 2016- \\ 2017 \\ \hline \end{array}$ | $\begin{array}{r} 2017- \\ 2018 \\ \hline \end{array}$ | $\begin{gathered} \hline 2018- \\ 2019 \\ \hline \end{gathered}$ | 3-yr <br> Average | $\begin{gathered} \hline 3-\mathrm{yr} \\ \text { Index } \\ \hline \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $70 \mathrm{mi} / \mathrm{hr}$ | ORS-2 | I-84 | 14 | 24 | 22 | 20 | 20 | 28 | 23 | 24 | 1.18 |
|  | ORS-3 | US-95 | 4 | 5 | 3 | 4 | 4 | 5 | 4 | 4 | 1.08 |
|  | ORS-12 | I-82 | 0 | 3 | 0 | 1 | 4 | 1 | 1 | 2 | 2.00 |
|  | Subtotal |  | 18 | 32 | 25 | 25 | 28 | 34 | 28 | 30 | 1.20 |
| $65 \mathrm{mi} / \mathrm{hr}$ | ORS-4 | US-26/US-20 | 11 | 7 | 6 | 8 | 9 | 13 | 9 | 10 | 1.29 |
|  | ORS-5 | US-97 | 11 | 15 | 23 | 16 | 31 | 29 | 18 | 26 | 1.59 |
|  | ORS-6 | OR-31 | 1 | 2 | 0 | 1 | 1 | 1 | 2 | 1 | 1.33 |
|  | ORS-7 | OR-78 | 0 | 1 | 1 | 1 | 3 | 4 | 2 | 3 | 4.50 |
|  | ORS-8 | US-395 | 1 | 0 | 0 | 0 | 1 | 1 | 0 | 1 | 2.00 |
|  | ORS-9 | US-395 | 1 | 1 | 1 | 1 | 0 | 2 | 2 | 1 | 1.33 |
|  | ORS-10 | OR-205 | 2 | 0 | 0 | 1 | 1 | 2 | 1 | 1 | 2.00 |
|  | ORS-11 | US-26 | 0 | 2 | 2 | 1 | 1 | 2 | 1 | 1 | 1.00 |
|  | Subtotal |  | 27 | 28 | 33 | 29 | 47 | 54 | 35 | 45 | 1.55 |
| Total |  |  | 45 | 60 | 58 | 54 | 75 | 88 | 63 | 75 | 1.39 |



Figure 3.8: Fatal and Injury A Crashes by Year and Speed Limit Group at Increased Speed Limit Locations

### 3.3.2.3 Temporary Speed Limit Reduction Segments

Table 3.16 shows the total number of fatal and injury A crashes by segment for the speed reduction locations. Compared to the control and speed limit increase segments, these segments are short and experience fewer crashes. With such varying behavior between segments (e.g., index of 13, to an index of 0.17), it is difficult to draw conclusions with a high level of confidence. Further analysis into these speed reduction segments is recommended.

Table 3.16: Fatal and Injury A Crashes (Before-to-After) on Reduction Segments

| Reduction <br> Segment | Route | $\mathbf{2 0 1 3 -}$ <br> $\mathbf{2 0 1 4}$ | 2014- <br> $\mathbf{2 0 1 5}$ | 2015- <br> $\mathbf{2 0 1 6}$ | 3-yr <br> Average | 2016- <br> $\mathbf{2 0 1 7}$ | $\mathbf{2 0 1 7}$ <br> $\mathbf{2 0 1 8}$ | $\mathbf{2 0 1 8}-$ <br> $\mathbf{2 0 1 9}$ | 3-yr <br> Average | 3-yr <br> Index |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 13 | US-97 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | - |
| 14 | US-97 | 0 | 0 | 1 | 0 | 7 | 4 | 2 | 4 | 13.00 |
| 15 | US-20 | 1 | 2 | 0 | 1 | 1 | 2 | 4 | 2 | 2.33 |
| 16 | US-20 | 4 | 2 | 0 | 2 | 0 | 0 | 1 | 0 | 0.17 |
| Total |  | $\mathbf{5}$ | $\mathbf{4}$ | $\mathbf{1}$ | $\mathbf{3}$ | $\mathbf{8}$ | $\mathbf{7}$ | $\mathbf{7}$ | $\mathbf{7}$ | $\mathbf{2 . 2 0}$ |

### 3.3.3 Total Truck-Involved Crashes

This section of the safety performance analysis focuses on the total number of truck-involved crashes on the control segments, the increased speed limit segments, and the speed reduction segments. Crashes considered include only mainline crashes within the milepost boundaries presented in Table 2.4, Table 2.5, and Table 2.6. To ensure this, a visual inspection of all crashes was conducted.

### 3.3.3.1 Control Segments

Table 3.17 shows the total number of truck-involved crashes on the control segments. In the 65 $\mathrm{mi} / \mathrm{hr}$ group, an index value of 1.30 was computed and the three-year average of crash counts in the after period increased by 33. In this group, the most truck-involved crashes occurred on I-5. In the $55 \mathrm{mi} /$ hr group, an index value of 1.02 was computed and the three-year average of crash counts in the after period increased by just one. In this group, three segments experienced a decrease in truck-involved crashes based on the three-year index. Figure 3.9 shows the total number of crashes for all control segments by year and posted speed limit.

Table 3.17: Total Truck-Involved Crashes (Before-to-After) on Control Segments

| Speed <br> Group | Control Segment | Route | $\begin{gathered} 2013- \\ 2014 \\ \hline \end{gathered}$ | $\begin{gathered} \hline 2014- \\ 2015 \\ \hline \end{gathered}$ | $\begin{array}{r} 2015- \\ 2016 \\ \hline \end{array}$ | 3-yr <br> Average | $\begin{array}{r} 2016- \\ 2017 \\ \hline \end{array}$ | $\begin{gathered} \hline 2017- \\ 2018 \\ \hline \end{gathered}$ | $\begin{array}{r} 2018- \\ 2019 \\ \hline \end{array}$ | 3-yr <br> Average | $\begin{gathered} \hline \text { 3-yr } \\ \text { Index } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{gathered} 60 \\ \mathrm{mi} / \mathrm{hr} \end{gathered}$ | C-1 | I-84 | 33 | 58 | 41 | 44 | 46 | 48 | 53 | 49 | 1.11 |
|  | C-11 | I-5 | 64 | 58 | 76 | 66 | 81 | 99 | 101 | 94 | 1.42 |
|  | Subtotal |  | 97 | 116 | 117 | 110 | 127 | 147 | 154 | 143 | 1.30 |
| $\begin{gathered} 55 \\ \mathrm{mi} / \mathrm{hr} \end{gathered}$ | C-2 | OR-11 | 0 | 4 | 1 | 2 | 3 | 0 | 2 | 2 | 1.00 |
|  | C-3 | US-26 | 3 | 1 | 2 | 2 | 1 | 3 | 5 | 3 | 1.50 |
|  | C-4 | US-26 | 1 | 4 | 13 | 6 | 4 | 7 | 3 | 5 | 0.78 |
|  | C-5 | OR-58 | 18 | 22 | 12 | 17 | 20 | 16 | 21 | 19 | 1.10 |
|  | C-6 | US-20 | 1 | 3 | 3 | 2 | 2 | 2 | 4 | 3 | 1.14 |
|  | C-7 | OR-126 | 1 | 0 | 3 | 1 | 4 | 1 | 0 | 2 | 1.25 |
|  | C-8 | OR-19 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | - |
|  | C-9 | OR-3 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | - |
|  | C-10 | OR-126 | 6 | 1 | 3 | 3 | 3 | 3 | 2 | 3 | 0.80 |
|  | C-12 | OR-140 | 7 | 4 | 1 | 4 | 3 | 2 | 5 | 3 | 0.83 |
|  | Subtotal |  | 37 | 40 | 38 | 38 | 40 | 34 | 43 | 39 | 1.02 |
| Total |  |  | 134 | 156 | 155 | 148 | 167 | 181 | 197 | 182 | 1.22 |



Figure 3.9: Total Truck-Involved Crashes by Year and Speed Limit Group on Control Segments

### 3.3.3.2 Increased Speed Limit Segments

Table 3.18 shows the total number of truck-involved crashes by segment for the increased speed limit segments. In the group where the speed limit was increased to $65 \mathrm{mi} / \mathrm{hr}$, an index value of 1.58 was computed and the three-year average of truck-involved crash counts in the after period increased by 80 . Each segment in this group experienced a moderate increase in truck-involved crashes, all with indices of 1.45 or greater. In the group where the speed limit was increased to $60 \mathrm{mi} / \mathrm{hr}$, an index value of 1.24 was computed and the three-year average of truck-involved crash counts in the after period increased by 19. Based on the indices, the highest increases in this group were observed on OR-78 and US-395. Figure 3.10 shows the total number of crashes for all control locations by year and posted speed limit.

Table 3.18: Total Truck-Involved Crashes (Before-to-After) on Change Segments

| Speed Group | Change Segment | Route | $\begin{array}{r} 2013- \\ 2014 \\ \hline \end{array}$ | $\begin{array}{r} 2014- \\ 2015 \\ \hline \end{array}$ | $\begin{gathered} 2015- \\ 2016 \\ \hline \end{gathered}$ | 3-yr <br> Average | $\begin{aligned} & \hline 2016- \\ & 2017 \\ & \hline \end{aligned}$ | $\begin{array}{r} \hline 2017 \\ 2018 \\ \hline \end{array}$ | $\begin{aligned} & 2018- \\ & 2019 \\ & \hline \end{aligned}$ | 3-yr <br> Average | $\begin{gathered} \hline \text { 3-yr } \\ \text { Index } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $65 \mathrm{mi} / \mathrm{hr}$ | ORS-2 | I-84 | 129 | 115 | 131 | 125 | 255 | 151 | 179 | 195 | 1.56 |
|  | ORS-3 | US-95 | 4 | 11 | 14 | 10 | 19 | 18 | 16 | 18 | 1.83 |
|  | ORS-12 | I-82 | 4 | 6 | 5 | 5 | 12 | 6 | 4 | 7 | 1.47 |
|  | Subtotal |  | 137 | 132 | 150 | 140 | 286 | 175 | 199 | 220 | 1.58 |
| $60 \mathrm{mi} / \mathrm{hr}$ | ORS-4 | US-26/US-20 | 25 | 14 | 13 | 17 | 24 | 19 | 18 | 20 | 1.17 |
|  | ORS-5 | US-97 | 42 | 48 | 67 | 52 | 79 | 54 | 58 | 64 | 1.22 |
|  | ORS-6 | OR-31 | 1 | 4 | 2 | 2 | 3 | 3 | 3 | 3 | 1.29 |
|  | ORS-7 | OR-78 | 1 | 2 | 2 | 2 | 5 | 6 | 3 | 5 | 2.80 |
|  | ORS-8 | US-395 | 2 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | --- |
|  | ORS-9 | US-395 | 2 | 0 | 1 | 1 | 3 | 4 | 1 | 3 | 2.67 |
|  | ORS-10 | OR-205 | 0 | 1 | 0 | 0 | 0 | 0 | 1 | 0 | 1.00 |
|  | ORS-11 | US-26 | 2 | 3 | 4 | 3 | 1 | 3 | 5 | 3 | 1.00 |
|  |  | btotal | 75 | 72 | 89 | 79 | 115 | 89 | 89 | 98 | 1.24 |
|  | Total |  | 212 | 204 | 239 | 218 | 401 | 264 | 288 | 318 | 1.45 |



Figure 3.10: Total Truck-Involved Crashes by Year and Speed Limit Group on Increased Speed Limit Segments

### 3.3.3.3 Temporary Speed Reduction Segments

Table 3.19 shows the total number truck-involved crashes by segment for the speed reduction segments. Compared to the control and speed limit increase segments, these segments are short and experience fewer crashes. Overall, each segment experienced a decrease in truck-involved crashes and an index of 0.67 was computed.

Table 3.19: Total Truck-Involved Crashes (Before-to-After) on Reduction Segments

| Reduction <br> Segment | Route | 2013- <br> $\mathbf{2 0 1 4}$ | 2014- <br> $\mathbf{2 0 1 5}$ | 2015- <br> $\mathbf{2 0 1 6}$ | 3-yr <br> Average | 2016- <br> $\mathbf{2 0 1 7}$ | $\mathbf{2 0 1 7}-$ <br> $\mathbf{2 0 1 8}$ | $\mathbf{2 0 1 8}-$ <br> $\mathbf{2 0 1 9}$ | 3-yr <br> Average | 3-yr Index |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 13 | US-97 | 2 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | - |
| 14 | US-97 | 3 | 8 | 3 | 5 | 4 | 3 | 4 | 4 | 0.79 |
| 15 | US-20 | 0 | 2 | 2 | 1 | 1 | 0 | 1 | 1 | 0.50 |
| 16 | US-20 | 4 | 2 | 1 | 2 | 1 | 2 | 2 | 2 | 0.71 |
| Total |  | $\mathbf{9}$ | $\mathbf{1 2}$ | $\mathbf{6}$ | $\mathbf{9}$ | $\mathbf{6}$ | $\mathbf{5}$ | $\mathbf{7}$ | $\mathbf{6}$ | $\mathbf{0 . 6 7}$ |

### 3.3.4 Fatal and Injury A Truck-Involved Crashes

Fatal and injury A truck-involved crashes are presented for the control segments, increased speed limit segments, and the speed limit reduction segments. These crashes include only mainline crashes within the milepost boundaries presented in Table 2.4, Table 2.5, and Table 2.6. To ensure this, a visual inspection of all crashes was conducted.

### 3.3.4.1 Control Segments

Table 3.20 shows the total number of fatal and injury A truck-involved crashes on the control segments. In the $65 \mathrm{mi} / \mathrm{hr}$ group, an index value of 1.67 was computed and the three-year average of crash counts in the after period increased by 2 . The most truck-involved crashes occurred in this group on I-5, but the greater index is on I-84 (4.0). In the $55 \mathrm{mi} / \mathrm{hr}$ group, an index value of 0.75 was computed and the three-year average of crash counts in the after period decreased by one. It should be noted that for these crash types, the number of crashes are small and indices for various segments could not be determined due to zero counts in the before or after period (denoted as "- in Table 3.20). Figure 3.11 shows the total number of crashes for all control segments by year and posted speed limit.

Table 3.20: Fatal and Injury A Truck-Involved Crashes (Before-to-After) on Control Segments

| Speed Group | Control <br> Segment | Route | $\begin{array}{r} 2013- \\ 2014 \\ \hline \end{array}$ | $\begin{aligned} & 2014- \\ & 2015 \\ & \hline \end{aligned}$ | $\begin{array}{r} 2015- \\ 2016 \\ \hline \end{array}$ | 3-yr <br> Average | $\begin{gathered} 2016- \\ 2017 \\ \hline \end{gathered}$ | $\begin{aligned} & \hline 2017- \\ & 2018 \\ & \hline \end{aligned}$ | $\begin{gathered} 2018- \\ 2019 \\ \hline \end{gathered}$ | 3-yr <br> Average | $\begin{gathered} \text { 3-yr } \\ \text { Index } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{gathered} 60 \\ \mathrm{mi} / \mathrm{hr} \end{gathered}$ | C-1 | I-84 | 1 | 0 | 0 | 0 | 2 | 0 | 2 | 1 | 4.00 |
|  | C-11 | I-5 | 5 | 2 | 1 | 3 | 4 | 4 | 3 | 4 | 1.38 |
|  | Subtotal |  | 6 | 2 | 1 | 3 | 6 | 4 | 5 | 5 | 1.67 |
| $\begin{gathered} 55 \\ \mathrm{mi} / \mathrm{hr} \end{gathered}$ | C-2 | OR-11 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 1.00 |
|  | C-3 | US-26 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | - |
|  | C-4 | US-26 | 0 | 1 | 1 | 1 | 0 | 1 | 0 | 0 | 0.50 |
|  | C-5 | OR-58 | 3 | 2 | 0 | 2 | 2 | 1 | 1 | 1 | 0.80 |
|  | C-6 | US-20 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | - |
|  | C-7 | OR-126 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | - |
|  | C-8 | OR-19 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | - |
|  | C-9 | OR-3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | - |
|  | C-10 | OR-126 | 2 | 0 | 1 | 1 | 0 | 0 | 1 | 0 | 0.33 |
|  | C-12 | OR-140 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0.00 |
|  | Sub | tal | 6 | 4 | 2 | 4 | 4 | 3 | 2 | 3 | 0.75 |
|  | Total |  | 12 | 6 | 3 | 7 | 10 | 7 | 7 | 8 | 1.14 |



Figure 3.11: Fatal and Injury A Truck-Involved Crashes by Year and Speed Limit Group on Control Segments

### 3.3.4.2 Increased Speed Limit Segments

Table 3.21 shows the total number of fatal and injury A truck-involved crashes by segment for the increased speed limit segments. In the group where the speed limit was increased to $65 \mathrm{mi} / \mathrm{hr}$, an index value of 1.24 was computed and the three-year average of fatal/injury A truck-involved crash counts in the after period increased by two. In the group where the speed limit was increased to $60 \mathrm{mi} / \mathrm{hr}$, an index value of 1.27 was computed and the three-year average of truckinvolved crash counts in the after period increased by one. Just two segments in this speed group had before and after crashes to compute an index. Figure 3.12 shows the total number of crashes for all control locations by year and posted speed limit.

Table 3.21: Fatal and Injury A Truck-Involved Crashes (Before-to-After) on Change Segments

| Speed Group | Change Segment | Route | $\begin{array}{r} 2013 \\ 2014 \end{array}$ | $\begin{gathered} \hline 2014- \\ 2015 \\ \hline \end{gathered}$ | $\begin{gathered} 2015- \\ 2016 \\ \hline \end{gathered}$ | 3-yr <br> Average | $\begin{array}{r} 2016- \\ 2017 \\ \hline \end{array}$ | $\begin{gathered} 2017- \\ 2018 \\ \hline \end{gathered}$ | $\begin{gathered} \hline 2018- \\ 2019 \\ \hline \end{gathered}$ | 3-yr <br> Average | $\begin{gathered} \text { 3-yr } \\ \text { Index } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $70 \mathrm{mi} / \mathrm{hr}$ | ORS-2 | I-84 | 3 | 9 | 6 | 6 | 5 | 9 | 7 | 7 | 1.17 |
|  | ORS-3 | US-95 | 1 | 1 | 1 | 1 | 0 | 1 | 3 | 1 | 1.33 |
|  | ORS-12 | I-82 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | --- |
|  | Subtotal |  | 4 | 10 | 7 | 7 | 6 | 10 | 10 | 9 | 1.24 |
| $65 \mathrm{mi} / \mathrm{hr}$ | ORS-4 | US-26/US-20 | 5 | 0 | 0 | 2 | 3 | 2 | 0 | 2 | 1.00 |
|  | ORS-5 | US-97 | 1 | 4 | 5 | 3 | 4 | 4 | 3 | 4 | 1.10 |
|  | ORS-6 | OR-31 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | --- |
|  | ORS-7 | OR-78 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | --- |
|  | ORS-8 | US-395 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | --- |
|  | ORS-9 | US-395 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | --- |
|  | ORS-10 | OR-205 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | --- |
|  | ORS-11 | US-26 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | --- |
|  | Subtotal |  | 6 | 4 | 5 | 5 | 7 | 7 | 5 | 6 | 1.27 |
|  | Total |  | 10 | 14 | 12 | 12 | 13 | 17 | 15 | 15 | 1.25 |



Figure 3.12: Fatal and Injury A Truck-Involved Crashes by Year and Speed Limit Group on Increased Speed Limit Segments

### 3.3.4.3 Temporary Speed Reduction Segments

Table 3.22 shows the total number of fatal and injury A truck-involved crashes by segment for the speed reduction segments. Compared to the control and speed limit increase segments, these segments are short and experience fewer crashes. Ultimately, it is difficult to draw any conclusion due to the small number of crashes.

Table 3.22: Fatal and Injury A Truck-Involved Crashes (Before-to-After) on Reduction Segments

| Reduction <br> Segment | Route | $\mathbf{2 0 1 3 -}$ <br> $\mathbf{2 0 1 4}$ | $\mathbf{2 0 1 4}-$ <br> $\mathbf{2 0 1 5}$ | $\mathbf{2 0 1 5}$ <br> $\mathbf{2 0 1 6}$ | 3-yr <br> Average | $\mathbf{2 0 1 6}-$ <br> $\mathbf{2 0 1 7}$ | $\mathbf{2 0 1 7}-$ <br> $\mathbf{2 0 1 8}$ | $\mathbf{2 0 1 8}-$ <br> $\mathbf{2 0 1 9}$ | 3-yr <br> Average | 3-yr <br> Index |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 13 | US-97 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | - |
| 14 | US-97 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 1 | - |
| 15 | US-20 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | - |
| 16 | US-20 | 4 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | - |
| Total |  | $\mathbf{4}$ | $\mathbf{0}$ | $\mathbf{0}$ | $\mathbf{1}$ | $\mathbf{1}$ | $\mathbf{1}$ | $\mathbf{0}$ | $\mathbf{1}$ | $\mathbf{0 . 5 0}$ |

### 3.3.5 Crash Proportions by Crash Type (All Vehicles)

The total number of crashes by crash type were determined for all control segments, increased speed limit segments, and speed reduction segments. In this analysis, the control segments and speed reduction segments are considered together.

### 3.3.5.1 Control and Speed Reduction Segments

Table 3.23 shows the proportion of crashes by crash type on the control and speed reduction segments. In both the before and after periods, fixed-object crashes and rear-end crashes account for the largest proportion; however, the proportion of fixed-object crashes decreased $3.3 \%$ in the after period while the proportion of rear-end crashes increased $5.6 \%$. All other changes were marginal in comparison. Still, the overall proportions remain fairly consistent in both the before and after periods.

Table 3.23: Proportions of Crashes by Crash Type on Control and Reduction Segments

|  | ANGL | BACK | FIX | HEAD | NCOL | OTH | PED | REAR | SS- <br> $\mathbf{M}$ | SS-O | TURN | Total |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Before | 39 | 1 | 1,308 | 58 | 164 | 366 | 12 | 1,208 | 55 | 487 | 109 | 3,807 |
| After | 41 | 6 | 1,440 | 51 | 173 | 371 | 12 | 1,729 | 72 | 577 | 161 | 4,633 |
| Before (\%) | $1.0 \%$ | $0.0 \%$ | $34.4 \%$ | $1.5 \%$ | $4.3 \%$ | $9.6 \%$ | $0.3 \%$ | $31.7 \%$ | $1.4 \%$ | $12.8 \%$ | $2.9 \%$ | $100 \%$ |
| After (\%) | $0.9 \%$ | $0.1 \%$ | $31.1 \%$ | $1.1 \%$ | $3.7 \%$ | $8.0 \%$ | $0.3 \%$ | $37.3 \%$ | $1.6 \%$ | $12.5 \%$ | $3.5 \%$ | $100 \%$ |
| Change | $\mathbf{- 0 . 1 \%}$ | $0.1 \%$ | $\mathbf{- 3 . 3 \%}$ | $\mathbf{- 0 . 4 \%}$ | $\mathbf{- 0 . 6 \%}$ | $\mathbf{- 1 . 6 \%}$ | $\mathbf{- 0 . 1 \%}$ | $5.6 \%$ | $0.1 \%$ | $\mathbf{- 0 . 3 \%}$ | $0.6 \%$ |  |



Figure 3.13: Number of Crashes by Crash Type on Control and Speed Reduction Segments

### 3.3.5.2 Increased Speed Limit Segments

Table 3.24 shows the proportion of crashes by crash type on the increased speed limit segments. Like the control and reduction segments, fixed-object crashes account for the largest proportion in both the before and after periods. However, unlike the control and reduction segments, noncollision accounts for the second-highest proportion, followed by rear-end crashes. Non-collision representing a higher proportion can be explained by these segments primarily being rural highways. Again, the overall proportions remain relatively consistent in both the before and after periods.

Table 3.24: Proportions of Crashes by Crash Type on Increased Speed Segments

|  | ANGL | BACK | FIX | HEAD | NCOL | OTH | PED | REAR | SS-M | SS-O | TURN | Total |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Before | 26 | 4 | 1211 | 68 | 526 | 547 | 9 | 333 | 92 | 185 | 85 | 3,086 |
| After | 49 | 10 | 1704 | 61 | 581 | 604 | 8 | 486 | 95 | 294 | 134 | 4,026 |
| Before (\%) | $0.8 \%$ | $0.1 \%$ | $39.2 \%$ | $2.2 \%$ | $17.0 \%$ | $17.7 \%$ | $0.3 \%$ | $10.8 \%$ | $3.0 \%$ | $6.0 \%$ | $2.8 \%$ | $100 \%$ |
| After (\%) | $1.2 \%$ | $0.2 \%$ | $42.3 \%$ | $1.5 \%$ | $14.4 \%$ | $15.0 \%$ | $0.2 \%$ | $12.1 \%$ | $2.4 \%$ | $7.3 \%$ | $3.3 \%$ | $100 \%$ |
| Change | $0.4 \%$ | $0.1 \%$ | $3.1 \%$ | $\mathbf{- 0 . 7 \%}$ | $\mathbf{- 2 . 6 \%}$ | $\mathbf{- 2 . 7 \%}$ | $\mathbf{- 0 . 1 \%}$ | $1.3 \%$ | $\mathbf{- 0 . 6 \%}$ | $1.3 \%$ | $0.6 \%$ |  |



Figure 3.14: Number of Crashes by Crash Type on Increased Speed Limit Segments

### 3.4 Empirical-Bayes Crash Analysis

With additional years of ODOT crash data available, this updated analysis includes crash modifications factors (CMFs) computations. This is accomplished through an Empirical Bayes approach, in which a series of count-data models were estimated to generate safety performance functions. Due to differences in roadway characteristics, CMFs were computed for interstates and non-interstates. For this analysis, the following count-data model specifications were tested:

- Poisson and Negative Binomial pooled models.
- Zero-inflated Poisson and Negative Binomial models.
- Traditional Poisson and Negative Binomial models based on cross-sectional data.

This approach determined that the most reliable estimates were obtained from the traditional cross-sectional Negative Binomial model. This model being the preferred approach indicates data dispersion in the crash data. This was true for both interstate models and non-interstate models.

### 3.4.1 Interstates

Final SPF model specifications for interstates are shown in Table 3.25.

Table 3.25: Negative Binomial Model Specifications for Total Interstate Crashes

| Variable | Coefficient | Std. <br> Error | z- <br> statistic |
| :--- | ---: | ---: | ---: |
| Constant | 3.90 | 2.41 | 1.62 |
| Natural Logarithm of AADT | -0.29 | 0.26 | -1.13 |
| Alpha | 0.39 | 0.07 | 5.79 |
| Model Summary |  |  |  |
| Number of Observations | 213 |  |  |
| Log-Likelihood at Convergence | -476.89 |  |  |

Using the model specifications from Table 3.25, the following interstate SPF is obtained:

$$
\begin{equation*}
\text { Expected Crashes }=e^{(3.90-(0.29) \text { LNAADT })} \tag{3.1}
\end{equation*}
$$

where Expected Crashes is the predicted number of crashes based on SPF estimates and AADT is the average annual daily traffic. The Empirical Bayes summary is shown in Table 3.26.

Table 3.26: Empirical Bayes Summary for Total Interstate Crashes

| Time Period | Observed Crashes | SPF Predicted Crashes |
| :---: | :---: | :---: |
| Before | 689 | 702.67 |
| After | 977 | 900.69 |

The EB estimates are show in Table 3.27. From the SPF, an estimated CMF of 1.10 was determined using the Empirical Bayes before-after approach. This CMF indicates that the speed increase on interstates in Eastern Oregon is expected to increase the number of crashes. The standard error of the CMF is 0.048 , and the $95 \%$ confidence interval ranges from 1.00 to 1.19 (although the range includes 1.00, it is the lower bound).

Table 3.27: Estimates for Empirical Bayes Interstate Crash Analysis

| Parameter | Estimate |
| :--- | :--- |
| $N_{\text {Expected,T,B }}$ | 694.33 |
| $N_{\text {Expected,T,A }}$ | 890.00 |
| Var $\left(N_{\text {Expected,T,A }}\right)$ | 695.90 |
| CMF | 1.10 |
| Var(CMF $)$ | 0.002 |
| SE(CMF) | 0.048 |
| $95 \%$ C. I. | $1.00,1.19$ |

### 3.4.2 Non-Interstates

In addition to generating a CMF for interstates, non-interstate roadways were assessed independently. Final SPF model specifications for non-interstates are shown in Table 3.28.

Table 3.28: Negative Binomial Model Specifications for Total Non-Interstate Crashes

| Variable | Coefficient | Std. <br> Error | z- <br> statistic |
| :--- | ---: | ---: | ---: |
| Constant | -2.54 | 0.36 | -6.98 |
| Natural Logarithm of AADT | 0.50 | 0.05 | 9.93 |
| Alpha | 0.74 | 0.09 | 8.20 |
| Model Summary |  |  |  |
| Number of Observations | 365 |  |  |
| Log-Likelihood at Convergence | -802.92 |  |  |

Using the model specifications from Table 3.25, the following interstate SPF is obtained:

$$
\begin{equation*}
\text { Expected Crashes }=e^{(-2.54+(0.50) \text { LNAADT })} \tag{3.2}
\end{equation*}
$$

where Expected Crashes is the predicted number of crashes based on SPF estimates and AADT is the average annual daily traffic. The Empirical Bayes summary is shown in Table 3.29.

Table 3.29: Empirical Bayes Summary for Total Non-Interstate Crashes

| Time Period | Observed Crashes | SPF Predicted Crashes |
| :---: | :---: | :---: |
| Before | 1,199 | $1,213.12$ |
| After | 1,748 | $1,709.48$ |

The EB estimates for non-interstates are shown in Table 3.30. From the SPF, an estimated CMF of 1.49 was determined using the Empirical Bayes before-after approach. This CMF indicates that the speed increase on non-interstates in Eastern Oregon resulted in an increase in crashes. The standard error of the CMF is 0.0 .044 , and the $95 \%$ confidence interval ranges from 1.40 to 1.58 (this range does not include the value 1.00).

Table 3.30: Estimates for Empirical Bayes Interstate Crash Analysis

| Parameter | Estimate |
| :--- | :--- |
| $N_{\text {Expected,T,B }}$ | 831.72 |
| $N_{\text {Expected,T,A }}$ | $1,172.02$ |
| $\operatorname{Var}\left(N_{\text {Expected,T,A }}\right)$ | 429.41 |
| CMF | 1.49 |
| $\operatorname{Var}(\mathrm{CMF})$ | 0.002 |
| SE(CMF) | 0.044 |
| $95 \%$ C. I. | $1.40,1.58$ |

### 4.0 SUMMARY

This analysis compares the changes in speed, volume, and safety performance on highway segments where the speed limit was increased in Eastern Oregon, select control locations, and a small number of segments in which there was a speed limit reduction. Three years of before and after data were available and used to assess the changes for average speed, percent of vehicles traveling over 65,75 , and $85 \mathrm{mi} / \mathrm{hr}$, traffic volume, total crashes, fatal and injury crashes for both total vehicles and trucks. An Empirical-Bayes before and after safety analysis was conducted for total crashes on interstates and non-interstate roadways. A summary of the analysis is provided in Table 4.1. The values in Table 4.1 are the change before to after for the speed measures and the index values for the safety measures. An index of 1.0 indicates no change, less than 1.0 indices a decrease, and greater than 1.0 indicates an increase. The change in frequency of crashes (denoted in parentheses) represents the difference in the three-year average. The results of the EB analysis are noted in the key observation section.

Table 4.1: Analysis Summary (Change in Measure or Index)

| Speed Group |  | Avg. Speed (mi/hr) | $\begin{gathered} \text { Pct. }> \\ 65 \\ \mathrm{mi} / \mathrm{hr} \end{gathered}$ | $\begin{gathered} \text { Pct. > } \\ 75 \\ \mathrm{mi} / \mathrm{hr} \end{gathered}$ | $\begin{gathered} \text { Pct. > } \\ 85 \\ \mathrm{mi} / \mathrm{hr} \end{gathered}$ | Total Monthly Volume | Total Crashes (All) | Fat/Inj. A Crashes (All) | Total Crashes (Trucks) | Fat/Inj. A <br> Crashes <br> (Trucks) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \hline \text { SPEED } \\ & \text { LIMIT } \\ & 70 \end{aligned}$ | Control | 0.46 | 2.41 | 2.44 | 0.12 | 1.06 | $\begin{gathered} 1.27 \\ (+216) \end{gathered}$ | $\begin{aligned} & 1.17 \\ & (+3) \end{aligned}$ | $\begin{gathered} 1.30 \\ (+33) \end{gathered}$ | $\begin{aligned} & 1.67 \\ & (+2) \end{aligned}$ |
| TRUCKS 65 | Change | 3.21 | 16.85 | 12.96 | 1.01 | 1.10 | $\begin{gathered} 1.34 \\ (+172) \end{gathered}$ | $\begin{aligned} & 1.20 \\ & (+5) \end{aligned}$ | $\begin{gathered} 1.58 \\ (+80) \end{gathered}$ | $\begin{aligned} & 1.24 \\ & (+2) \end{aligned}$ |
| Change |  | $\uparrow$ | $\uparrow$ | $\uparrow$ | $\uparrow$ | $\uparrow$ | $\uparrow$ | $\uparrow$ | $\uparrow$ | $\downarrow$ |
| Key Observation |  | There is an increase in speeds and higher proportion of vehicles traveling at higher speeds. |  |  |  |  | EB estimate for total crashes is 1.11 (1.00, 1.19, confidence interval). The descriptive analysis highlights the increase in total crashes and truck-involved crashes. There is a slight change in fatal/injury A crashes, and based on the index, a decrease in fatal/injury A truck-involved crashes. |  |  |  |
| SPEED <br> LIMIT <br> 65 | Control | 0.36 | 2.09 | 0.24 | 0.02 | 1.09 | $\begin{gathered} 1.11 \\ (+45) \end{gathered}$ | $\begin{aligned} & 1.16 \\ & (+5) \end{aligned}$ | $\begin{aligned} & 1.02 \\ & (+1) \end{aligned}$ | $\begin{aligned} & 0.75 \\ & (-1) \end{aligned}$ |
| $\begin{gathered} \hline \text { TRUCKS } \\ 60 \\ \hline \end{gathered}$ | Change | 3.56 | 16.09 | 1.91 | 0.50 | 1.11 | $\begin{gathered} 1.27 \\ (+142) \end{gathered}$ | $\begin{gathered} 1.55 \\ (+16) \end{gathered}$ | $\begin{gathered} 1.24 \\ (+19) \end{gathered}$ | $\begin{aligned} & 1.27 \\ & (+1) \end{aligned}$ |
| Change |  | $\uparrow$ | $\uparrow$ | $\uparrow$ | $\uparrow$ | $\uparrow$ | $\uparrow$ | $\uparrow$ | $\uparrow$ | $\downarrow \uparrow$ |
| Key Observation |  | There is an increase in speeds and higher proportion of vehicles traveling at higher speeds. |  |  |  |  | The safety performance decreased more on these roadways than the interstates. The EB estimate for total crashes is 1.49 (1.40, 1.58, confidence interval). The descriptive analysis highlights the increase in all considered crash types and severities. |  |  |  |

Concerning speed changes, the analysis in this report with the additional years of data confirms the findings of the preliminary analysis conducted in 2018. Vehicle speeds have increased on the highways where the posted speed limits were increased. The data show that the average and percentage of vehicles exceeding 65,75 , and $85 \mathrm{mi} / \mathrm{hr}$ have increased. The average speeds have increased approximately $3 \mathrm{mi} / \mathrm{hr}$, and these changes are statistically significant. More importantly, the percentage of high-speed vehicles (e.g., over $85 \mathrm{mi} / \mathrm{hr}$ ) has also increased. A significant increase in vehicles traveling at very high speeds (i.e., more than $15 \mathrm{mi} / \mathrm{hr}$ above the posted speed limit) is a concern since these are the vehicles typically associated with worse crash outcomes. The increases 1 ) in the stopping distance, 2 ) in the required detection distance to make reactions in time for collision avoidance, and 3) the kinetic energy in these vehicles can contribute to increased crashes and severity for vehicles traveling at higher speeds.

The safety performance also mostly confirms the preliminary analysis. On the interstates where speeds were increased to $70 \mathrm{mi} / \mathrm{hr}$ and on the network of rural two-lane highways where speeds were increased from $55 \mathrm{mi} / \mathrm{hr}$ to $65 \mathrm{mi} / \mathrm{hr}$, the descriptive analysis of crash frequency suggests an increase in the total number of crashes, fatal/injury A crashes, and truck-involved crashes. The index change for fatal and injury A crashes more closely matched the trends on the control sections for the interstate speed changes but increased on the $55 \mathrm{mi} / \mathrm{hr}$ to $65 \mathrm{mi} / \mathrm{hr}$ increase sections.

The crash analysis finds that the total truck-involved crashes have increased at a rate higher than both the total traffic volume and the truck-related crash performance change on the control sections. On the sections where the speed limit was raised to $65 \mathrm{mi} / \mathrm{hr}$ for trucks (primarily the interstates), the index for truck-involved crashes was very close to the control sections. On the sections where the speed limit was increased to $60 \mathrm{mi} / \mathrm{hr}$ for trucks, crashes also increased more than the truck-involved crashes on the control sections. There are fewer fatal and injury A crashes that involve trucks. On the $65 \mathrm{mi} / \mathrm{hr}$ segments (interstates) the increase was similar to the control sections. On the $55 \mathrm{mi} / \mathrm{hr}$ segments, the increase was more than the control sections.

The more robust Empirical Bayes before-after method confirms the basic descriptive analysis for total crashes. To be consistent with roadway characteristics, a CMF was computed for interstates and for non-interstates. A CMF of 1.10 was calculated for interstates, with 1.00 being the lower bound of the $95 \%$ confidence interval. This suggests that total crashes increased due to the speed limit increase. This is in line with the descriptive analysis, where crashes increased on nearly all segments for all types of crashes. For non-interstates, a CMF of 1.49 was computed, where the $95 \%$ confidence interval does not include 1.00. These results confirm that crashes increased due to the speed limit increase.

Overall, the changes observed in this updated analysis confirm the preliminary analysis and results of other speed changes found in the literature. The increase in crashes on the network of rural two-lane highways where speeds were increased from $55 \mathrm{mi} / \mathrm{hr}$ to $65 \mathrm{mi} / \mathrm{hr}$ was significantly more than the increase observed on the interstate segments.

### 4.1 LIMITATIONS

As stated in the previous analysis, a common limitation of using ATR data is the localized nature of ATRs (speeds are only recorded at the location of ATR and may not reflect behavior up- or downstream). Additionally, there were stations with missing data that had data in the previous analysis. It is anticipated that this may be a result of the change in the data interface. Although it may not have impacted the current analysis, this issue may exist at other ATRs across Oregon. Lastly, all speeds were averaged (e.g., passenger vehicles, trucks, etc.), which may result in speeds that overrepresent a specific vehicle type depending on the ATR location. In the data's current form, unless at a WIM station, parsing out vehicle types to assess speeds is difficult. This is particularly true in Eastern Oregon, where there are inherently fewer ATRs and just two WIM stations on the eastern portion of I-84. Finally, it should be noted that subtle differences were observed in some cases but may be a result of the new speed data format and updates made to the reported crash data

For the safety performance analysis, exposure-based data was limited to average annual daily traffic. It is expected that with additional exposure-based covariates, the generated SPFs can predict crashes at a higher rate. The EB analysis did not extend to the fatal and injury A crashes and only included total crashes.


[^0]:    ${ }^{1}$ Numbering starts at (2) to match the numbering scheme in ORS 811.111.

[^1]:    ${ }^{\text {a }}$ All speeds in $\mathrm{mi} / \mathrm{hr}$

