

STATE OF OREGON

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MONITORING WELLS

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MONITORING WELLS

ABSTRACT

Ground water monitoring wells are installed so that accurate, periodic, measurements and/or samples of subsurface water can be obtained. Potentiometric surfaces and water table elevations can be measured in monitoring wells. Monitoring wells used in conjunction with aquifer performance tests, e.g. pump or slug tests, aid in evaluating such hydrologic constants as transmissivity, hydraulic conductivity or permeability, and storage coefficient or specific yield by making it possible to directly measure the reaction of the aquifer to an applied stress.

Spatial and temporal variations or changes in water quality can be quantitatively determined with the aid of water samples taken from monitoring wells. Vertical and horizontal attenuation rates of leachates or effluents emanating from waste disposal sites, lagoons, effluent sprinkling systems, etc. can be ascertained with the aid of properly placed, designed, and installed monitoring wells.

INTRODUCTION

Water well levels have been checked regularly for over forty years in Oregon. About fifteen years ago the State Engineer and the U. S. Geological Survey entered into a cooperative agreement to maintain the observation well net which now includes nearly 800 water wells throughout the state, see Figure 1 and Appendix A. The regular measurement of these wells makes it possible to observe and document problem or potential problem areas, e.g. where there is a falling water table, and to take the necessary remedial action.

The State Engineer in cooperation with the State Health Division, Radiation Surveillance Laboratory, has been sampling selected water wells for chemical and radiological analysis since 1971. This well net includes about 30 water wells and springs, see Figure 1.

In recent years there has been an increase in environmental awareness and concern. This is reflected in more stringent federal, state, and local regulations governing potentially detrimental environmental practices. Waste disposal is among the most obvious of these practices.

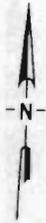
In general, the major threat to the environment presented by waste disposal is the contamination of ground and surface water with effluent or leachate emanating from the disposal site. The Oregon Department of Environmental Quality (D.E.Q.) and the Oregon State Engineer are charged with the protection of the waters of the state under ORS 449.079, 537.525, .730, and .775. Hydrogeologists with the office of the State Engineer have been actively working with the D.E.Q. and various local governmental bodies in evaluating the potential of specific disposal sites for ground and surface water contamination. The placement and design of monitoring wells has been an integral part of this cooperative effort. At the present time there are more than 100 water quality monitoring wells, existing or under construction, in Oregon. About 60 of these are associated with landfills.

Many engineers, sanitarians, well drillers, public officials, and citizens have expressed interest in monitoring wells. This report is written in an attempt to summarize the reasons for, theory behind, and methods for the construction of some typical monitoring wells.

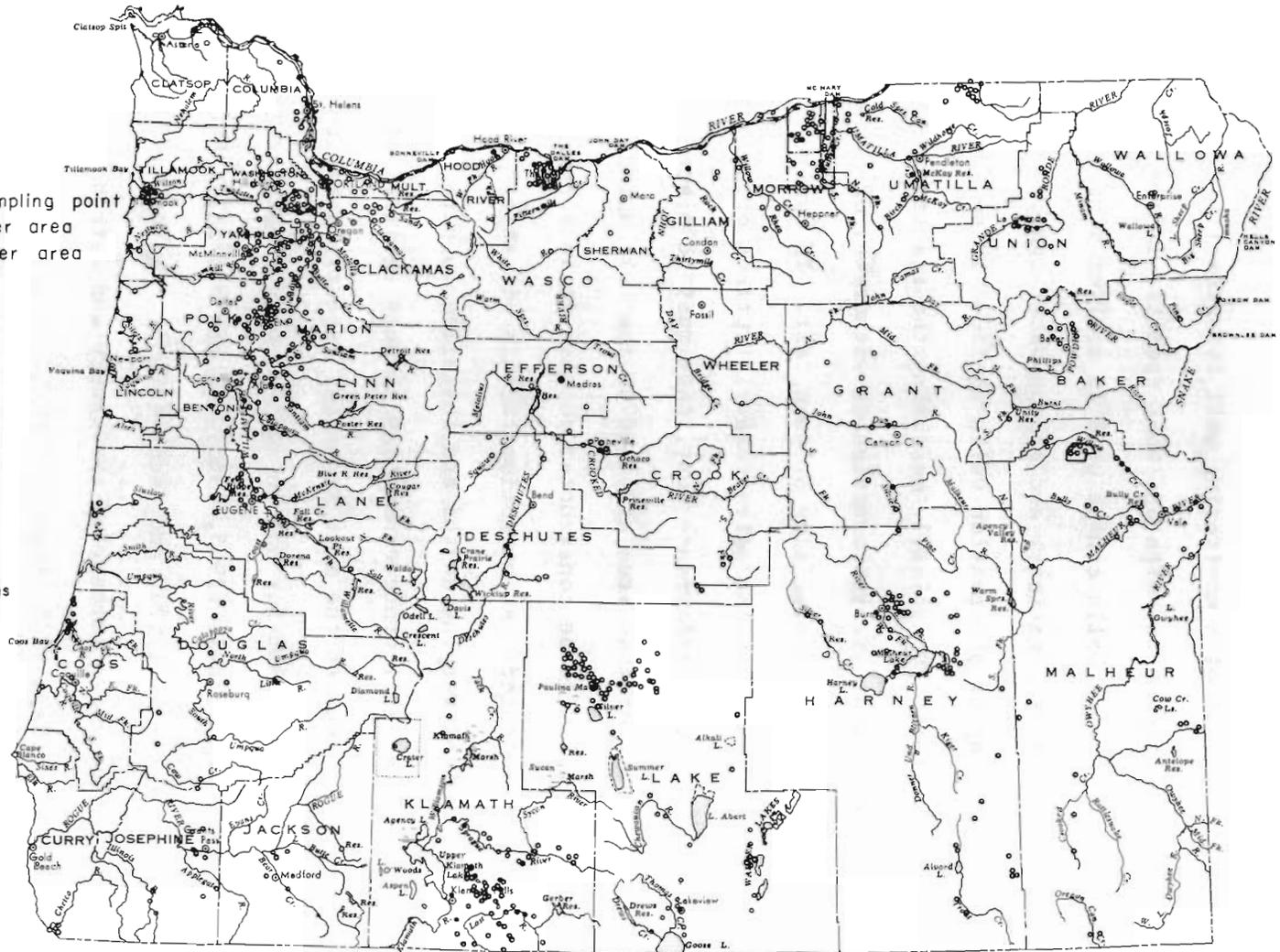
STATE OBSERVATION WELL NET

LEGEND

- Observation well
- Chemical and radiological sampling point
- Existing critical ground-water area
- - - Proposed critical ground-water area



Scale 10 0 10 20 30 40 Miles



USE, LOCATION, AND OPEN AREA

The location of a monitoring well is naturally dependent on the geology and hydrology of the area and what specific information is sought. For example, monitoring wells can be used in aquifer performance testing, subsurface sewage investigations, and ground-water quality studies.

Monitoring or observation wells employed in an aquifer performance test provide a method of physically measuring the reaction of the aquifer to an applied stress, that is, introduction or withdrawal of a known volume of water at a measured rate, e.g. slug or pump test. If monitors are to be used to measure changes in the potentiometric surface or water table elevation during an aquifer performance test, they must be placed within the radius of influence or that area affected by the well being tested. Generally, the monitoring well will be constructed so that it is screened or open to the aquifer being tested. An exception to this situation would be when multiple monitors at varying depths are installed to determine to what extent there is hydraulic interconnection between aquifer units. A number of authors have discussed the more technical aspects of monitoring or observation well placement for use in aquifer performance tests. These include Thiem (1906), Theis (1935, 1963), Cooper et. al. (1946), Brown (1953), Ferris et. al. (1962), and Jacob (1963). Walton (1970) among others presents a comprehensive summary of the theory and methodology involved.

Subsurface Sewage Disposal Systems

The most common shallow subsurface waste disposal system in Oregon is the septic tank and drainfield used for sewage disposal. This system is based on separation of solids, detention and anaerobic digestion of sewage in a water-tight, covered, receptacle (septic tank), followed by discharge

of the effluent to a drainfield or soil absorption system (U. S. Public Health Service, 1967). Treatment of the effluent in the soil is generally based on unsaturated flow and aerobic treatment of the effluent (Bouma et. al., 1972).

In order for a soil to be suitable for the absorption of septic tank effluent it must have an acceptable hydraulic conductivity, without interference from ground water and restrictive or impervious strata below the level of the absorption system. Wert (1969) has stated in his study of drainfield performance in Willamette Valley soils that "if failure is defined in degree of health hazard, then age and permeability of the drainfields studied are of minor importance when compared to water table fluctuations." Dickinson (1972) documented the presence of ground water pollution resulting from septic tank installations in an area northwest of Eugene where seasonally shallow ground water is encountered. The U. S. Public Health Service (1967) recommends and the D.E.Q. (1974) requires that the depth to the seasonal high water table (saturated zone) be at least four feet below the bottom of a disposal trench or six feet below the natural ground surface, respectively. An area where perched ground water would come into contact with the disposal field is also considered unacceptable. These recommendations and rules are obviously based on the fact that the previously mentioned unsaturated, aerobic conditions occur above the water table. Because of the range of geologic and hydrologic conditions encountered in the field it is often difficult to discern between a seasonal high water table or saturated zone, a permanently perched water table, and a temporarily perched water table as defined by Lohman et. al. (1972). It should be noted that deeper wells or monitors may not accurately reflect the water table surface, depending on their spatial distribution on and depth of penetration into

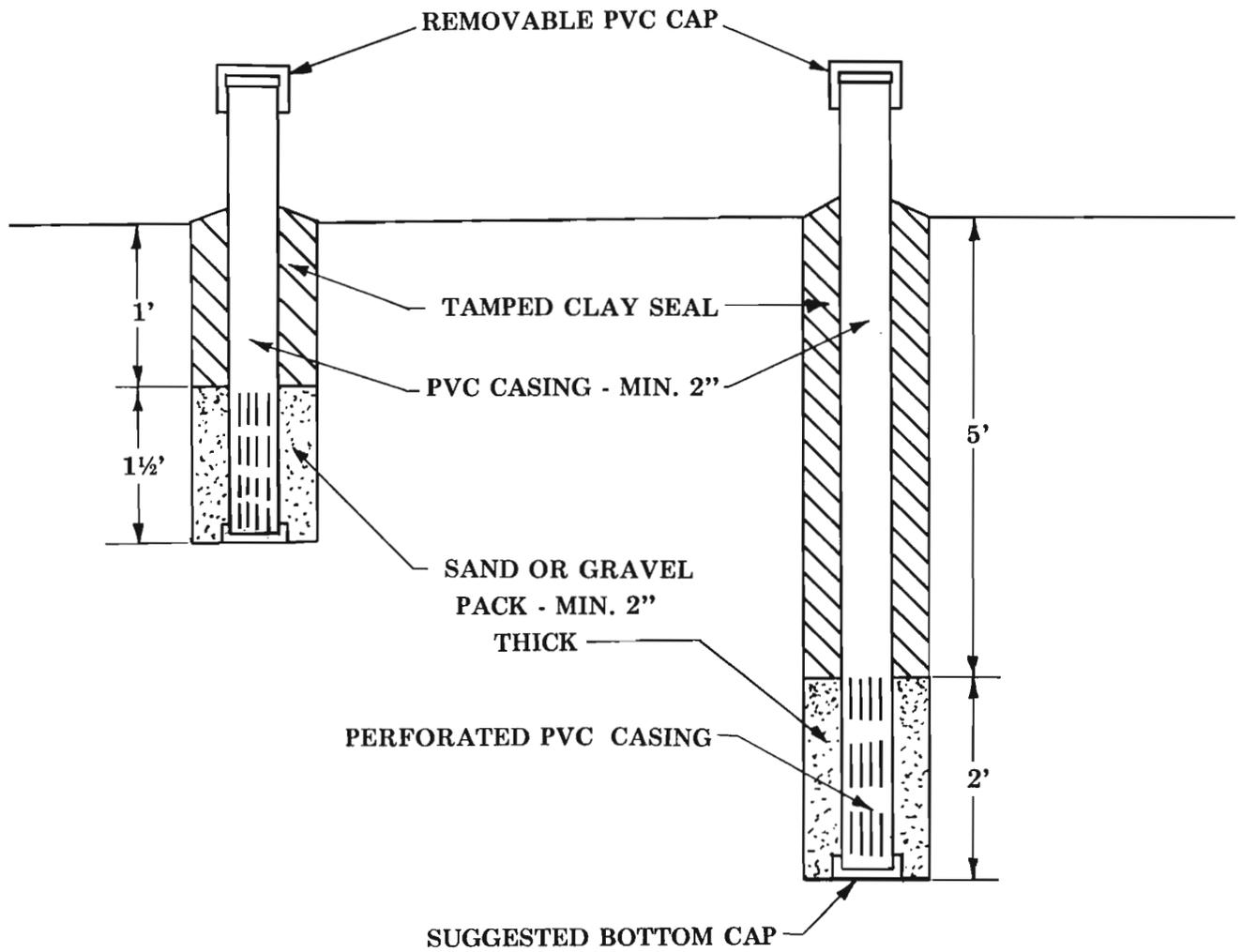
the regional, intermediate, and local ground-water flow systems. Also, many soils in western Oregon are poorly drained and have temporarily perched water tables which present problems to the operation of a subsurface disposal system (Boersma et. al., 1970).

A simple monitoring well system can be an invaluable aid in interpreting shallow subsurface conditions. If only the depth to the regional water table or saturated zone is in question, suitable monitoring wells open in the five to seven foot depth range can be installed in the proposed drainfield area, see Figure 2. On the other hand, if a perched or temporarily perched water table is suspected, paired shallow-deep monitoring wells, e.g. with depths of three to seven feet, may be in order. Another possible application of shallow and/or paired shallow-deep monitoring wells is in proving the capacity of cut-off or French drains to lower a perched water table below a proposed drainfield. In some instances it may be advisable to leave the monitoring well system in place, after the drainfield is installed, in order to sample for the affect of the effluent on the local ground water. A later section explains the design and construction of monitoring wells which may be suitable for subsurface sewage disposal investigative purposes. An example of a subsurface investigation monitoring system is included in Appendix B.

Ground Water Quality Monitors

The proper location and selection of the open area of ground-water quality monitors is perhaps the most difficult of those monitors described herein. Their location, aside from being determined by what information is desired, is dependent upon a number of interrelated environmental factors including:

1. hydrologic conditions,
2. native water quality,
3. type of contaminant or contaminants,
4. rate of leakage, and
5. capacity of the environment to treat and/or assimilate the contaminant.



*Note - PVC Casing may be perforated with saw cuts.

DRAINFIELD INVESTIGATION MONITORING WELL

Figure 2. Drainfield investigation monitors.

Many hydrologic parameters are pertinent to ground-water quality monitors. The position of the disposal area within the ground-water flow system has been discussed by Freeze (1972) and Hughes et. al. (1971, 1972). Other relevant hydrologic conditions include the direction and velocity of ground water flow, depth to the water table or saturated zone, hydraulic conductivity, and storage coefficient of the materials underlying and/or surrounding the disposal site. A knowledge of these parameters makes it possible to calculate the site underflow or the discharge of the contaminant receiving body. In solid waste disposal sites these hydrologic constants for the waste material are also an important consideration.

The native water quality of the receiving body is an important consideration. It is generally measured by placing at least one monitoring well up-gradient or out of the sphere of influence of the disposal site. There is generally a higher total dissolved solids concentration and increased geothermal gradient in ground water discharge zones, as compared to recharge or transition zones. Also in discharge zones there is little likelihood of deep percolation of waste disposal site contaminants since ground-water movement is directed vertically and/or obliquely toward the land surface. These facts should be taken into consideration when locating and designing disposal areas, monitoring systems, and determining background water quality. Furthermore, if the native ground water is of exceptionally poor quality, rich in particular constituents or nutrients, it may be difficult to assess the impact of the waste disposal site.

The ability to differentiate between native ground water and contaminated ground water is partially a function of the material being disposed of and the volume and quality of the leachate, effluent, or leakage

leaving the disposal site. Total volume, concentration, toxicity, solubility, biodegradation rate, vapor pressure, filtrative and sorptive capacity in the soils underlying the site, and other properties of the materials being disposed of all must be considered in the siting of ground-water quality monitoring wells.

The relationship of the disposal site to the water table and site design and operation are important considerations in locating ground-water quality monitoring wells. It has been demonstrated by a number of authors that leachate is not generated in a landfill unless water is added to the refuse, for example by ground water, flooding, and infiltrated precipitation. Therefore, good site drainage, compaction, and daily cover with a low permeability material is very important in the operation of a landfill. Likewise, a lagoon underlain by compacted soils with a low hydraulic conductivity will generally not leak too rapidly. If a solid waste site is located tens of feet above the water table, underlain by soils with only a moderate hydraulic conductivity, properly designed, and conscientiously operated, it may not be necessary to install ground-water quality monitors. On the other hand, when a site is situated on a flood plain, underlain by gravelly materials with a relatively high hydraulic conductivity, and/or periodically inundated by ground or surface water, the installation of a water quality monitoring system is generally advisable. An example of a landfill monitoring system is included in Appendix C. It should be emphasized that the water quality monitoring system is not installed to "prove" that a waste disposal site is a gross polluter but often indicates just the opposite, that is, the disposal site is releasing only an insignificant amount of contaminant to the environment and then only at a very slow and controlled rate.

The environment has the capacity to treat and/or assimilate large amounts of potential contaminants when they are properly introduced. Filtrative and sorptive properties of soils and the consumption of contaminants by soil bacteria are well documented. Dilution of contaminated discharge by a receiving body, mixing with either ground or surface waters, may reduce contaminant concentrations to acceptable levels. However, total reliance on the soils for "treatment" of wastes is not always prudent. For example, adsorption or cation exchange is a complicated and geochemically sensitive process. It is reversible and changes in the chemistry of the system can influence the degree of retention of specific cations. An extreme example is at the Hanford facility in eastern Washington where shallow trenches are used only once for the disposal of high salt wastes or for chemically complexed isotopes which may interfere with ground exchange reactions when mixed with other wastes (Belter, 1962). Gases such as carbon dioxide and leachate leaving the site can react with the native ground water and result in changes in the sorptive and filtrative capacities of the surrounding soils. Also, placement, design, and operation of the disposal site can disrupt the local hydrology and consequently alter the amount of dilution provided in the receiving body.

All of these considerations must be kept in mind when locating, designing, installing, and sampling ground-water quality monitors. In summary, ground-water quality monitors provide a method of directly observing the impact of a particular environmental change on the quality of the ground water. With the continued development of monitoring well systems and the ability to physically measure the validity of present site selection, design, and operational procedures, improvements in waste disposal practices will be made possible.

DESIGN AND CONSTRUCTION

The design and construction of water wells is regulated by the State Engineer as directed by the Oregon Ground Water Act of 1955, ORS 537. Construction standards are included in General Standards for the Construction and Maintenance of Water Wells in Oregon (1962). Copies of these standards are available from the State Engineer's Office.

Many authors have discussed and described the various procedures in the construction of water wells (Campbell et. al., 1973; Edward E. Johnson, 1966; Oregon Drilling Association, 1968; Department of the Army and Air Force, 1957; Gordon, 1958). The general procedure for the installation of monitors parallels that of water wells. However, monitors are designed and installed in an effort to measure or sample geologic, hydrologic, and/or hydrochemical conditions within a definite portion of the substrata. These special requirements necessitate deviations from standard water well design and construction procedures.

Casing Size

Casing size is an initial consideration in designing both water wells and monitoring wells. The ultimate use of the well generally determines casing specifications of monitoring wells. For example, the use of a monitor as a measuring point for depth to water, test well for pump or slug testing, or water quality sampling well, may require different casing specifications.

If a well is to be employed in measuring water levels, the smallest practicable well diameter is recommended. This is based on the fact that a small diameter well or piezometer will stabilize and react to potentiometric changes more rapidly than a larger diameter casing. Methods

have been developed to reduce the diameter of a piezometer stand pipe and thus increase its sensitivity (Hughes et. al., 1971).

The method by which the well is to be measured will also have a bearing on the casing size. For example, a steel or electric tape may require less than a one inch diameter hole for easy passage. Some crest gages and pressure transducers require about a two inch diameter casing. At least one float type maximum-minimum depth gage has been designed to operate in a three inch casing (Collins, et. al., 1967). Float type continuous water level recorders are available commercially. In using these instruments it should be kept in mind that the larger floats, e.g. six versus three inches, are more sensitive to water level fluctuations and thus more accurate in measuring small water level changes.

When a well is to be used for water quality sampling, allowance must be made for collection of the sample. Ideally, the well casing should be evacuated prior to sampling so that fresh water in the vicinity of the well can drain into the well and a representative sample of the formation water can be taken. Evacuation and sampling of the well is often accomplished through pumping. When the depth of the well, available pumping equipment, and the casing size are restrictive, the well may be bailed and/or sampled with a tube or thief sampler. Dudley and Stephenson (1973) have demonstrated that sampling bias can be a problem when bailed rather than pumped samples are taken for nitrogen and phosphorus testing. However, they point out that "in soils of low permeability or where the water table is deeper than 28 feet, bailing is the only convenient way to obtain a sample." Furthermore, "data from bailed wells are useful in observing trends of ground-water contamination, although the absolute concentrations may be exaggerated." Thief

samplers are available in a variety of sizes and styles and with varying degrees of sophistication. The least sophisticated of these is a weighted sample bottle holder which merely pulls the sampling container under the water level. More sophisticated thief sampling assemblies include flap valves, drain spigots, and stopper assemblies designed to close the sampler while submerged.

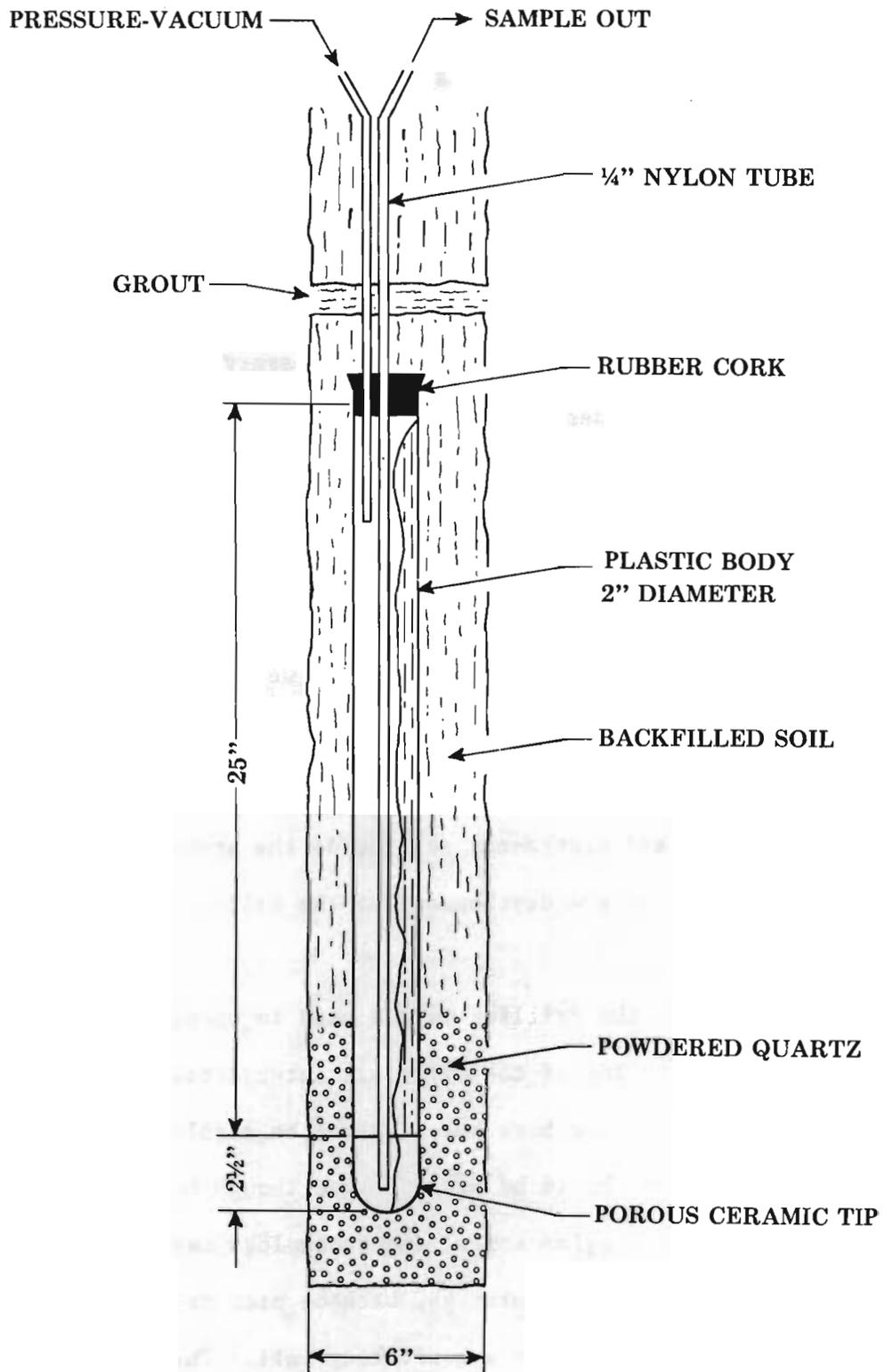
If there is a paucity of water in the well due to a low hydraulic conductivity and/or unsaturated conditions in the zone to be sampled it is sometimes possible to use a suction lysimeter to obtain a water sample. Suction lysimeters have been used in landfill studies to extract soil moisture samples (Apgar and Langmuir, 1971). Their design and operation have been described in detail by Lane and Parizek (1968) and Parizek and Lane (1970). A cross section of a lysimeter is shown in Figure 3. The lysimeter consists of a plastic cylinder 25 inches long and two inches in diameter, with a porous ceramic tip two and one-half inches long. It can hold one liter of water. Soil moisture is drawn through the powdered quartz and the ceramic tip into the lysimeter by the application of a vacuum, greater than the soil moisture tension, through the pressure-vacuum tube. Water samples are forced to the surface for collection by exerting a gas pressure on the pressure-vacuum tube, e.g. with a tire pump. The nature of the powdered quartz and ceramic tip may restrict the passage of larger colloidal sized particles, some particulates and bacteria, and perhaps some biochemical oxygen demanding (BOD) active substances. Apgar and Langmuir (1971) successfully recovered soil-water samples from depths as great as 70 feet with suction lysimeters.

Depth of completion or the depth zone for which information is sought is dependent on a number of hydrological variables. For example, near surface waste disposal in a regional ground-water discharge zone will generally not require a deep water quality monitoring system. On the other hand, disposal of a similar waste in a ground-water recharge zone may require deeper monitors. As mentioned earlier variables including the depth of the sampling zone, number of monitors, and frequency of sampling are dependent on the type and volume of waste, method of disposal, and geologic and hydrologic conditions at the disposal site. However, in all cases the sampling depth should be considered in determining the water level measuring and/or sample collection method and thus in deciding on the optimum casing size.

Casing Type

Casing type is also important in monitoring well design. Again the ultimate use of the well is the most important consideration. If the well is to be used for water quality monitoring, care should be taken to insure that the casing will not react with natural or contaminated ground water and unduly alter the chemistry of the water being sampled. For this reason most standard metal casing is not recommended in landfill monitoring wells. Leachate from a landfill often contains weak organic acids and other constituents which will attack the metal casing thus altering the water chemistry.

Casing cost is generally a prime consideration in well design. Well screen costs range from less than a dollar per foot for small diameter polyvinyl chloride (PVC), several dollars or more per foot for rolled steel well casing, to more than fifty dollars per foot for six inch diameter



SUCTION LYSIMETER

Figure 3. Cross-section of a suction lysimeter, modified after Parizek and Lane (1970).

stainless steel well screen. Sometimes there are other cost considerations such as installation, e.g. in a few cases drive points may be acceptable for a monitor. However, PVC casing would not be usable for a drive point since its strength is inadequate to withstand driving stresses.

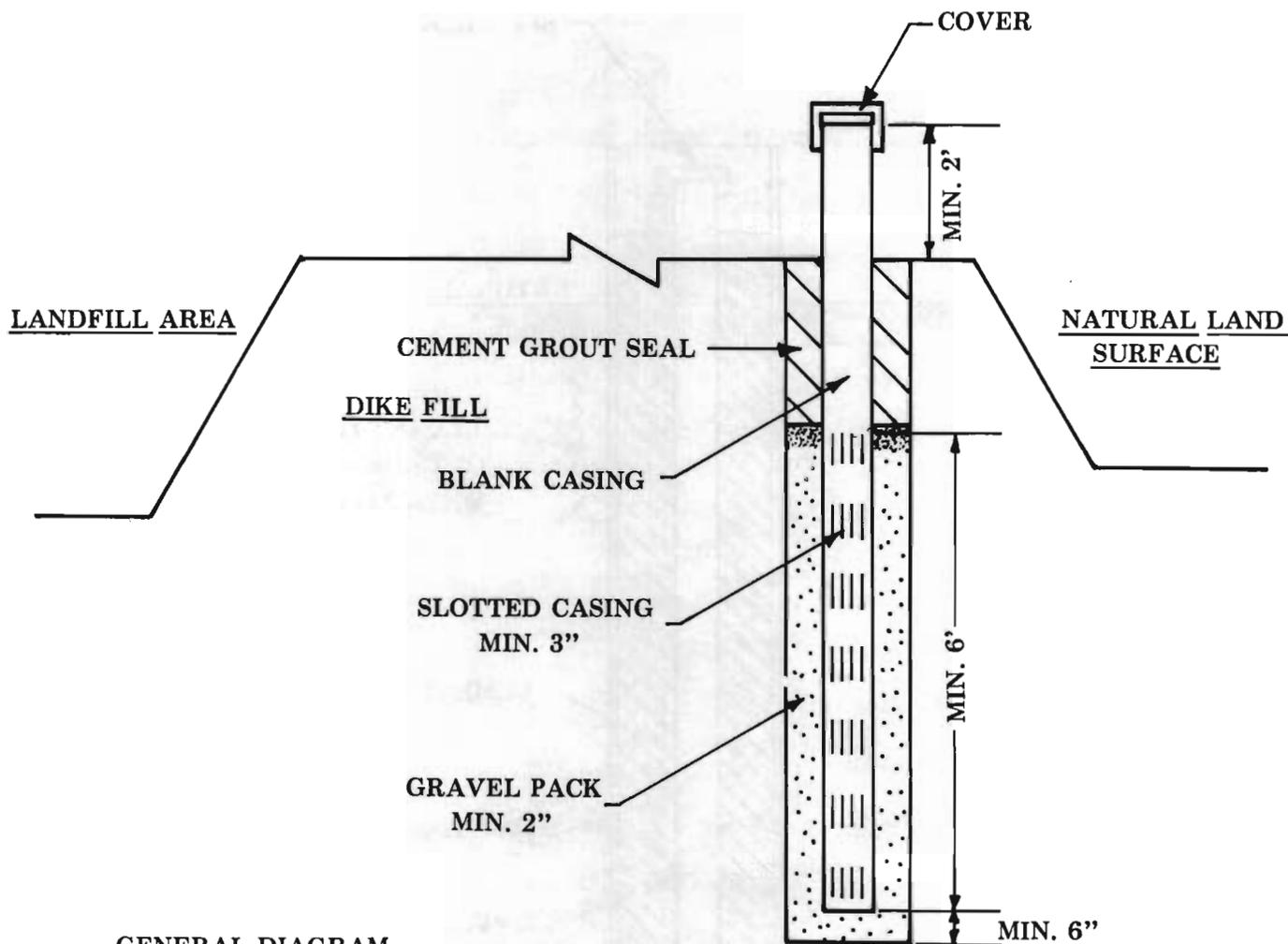
In many cases a combination of steel outer and PVC inner casing is recommended. This may be applicable when drilling and driving followed by jacking back of the outer, steel, casing is necessary. Examples of the various combinations or types of drilled monitors are included as Figures 2, 4, 5, and 6.

INSTALLATION

Installation of the monitoring well is perhaps the most critical element in determining the effectiveness of the well. It is in this phase that the design theory and existing conditions are matched. A number of general statements can be made concerning the logging of the bore hole, gravel pack selection and placement, sealing of the annular space between the bore hole and casing, and development of the well.

Drilling Log

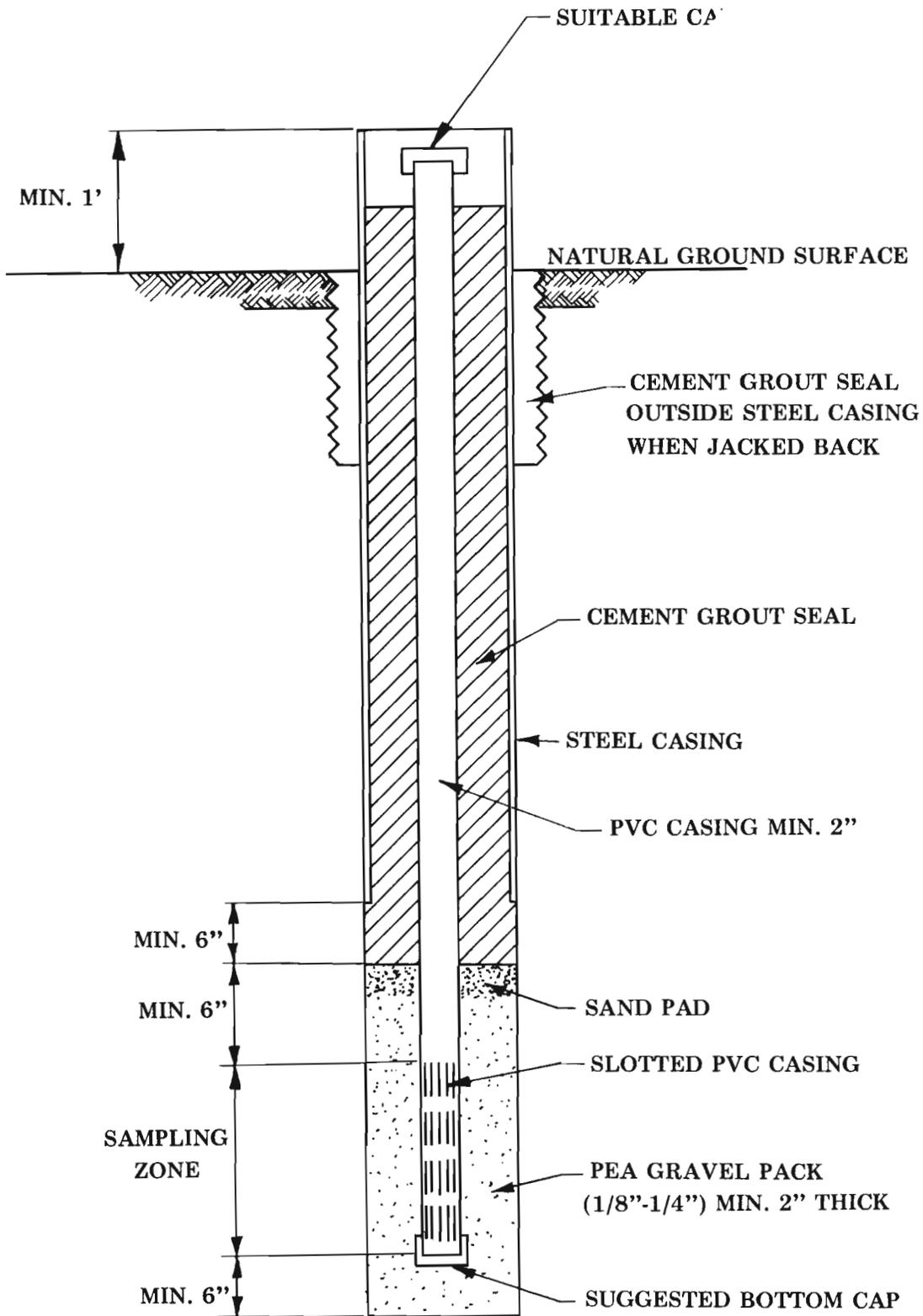
Regardless of the drilling method used in opening the bore hole, an accurate and complete log of the materials intersected during drilling is a must. In some cases the bore hole log may be supplemented by geophysical logging of the hole. It should be noted that although backhoe pits provide an excellent avenue for logging soils, and these logs may be of assistance in interpreting drill or auger cuttings, backhoe pits do not generally lend themselves to the installation of a monitoring well. This is because the relatively large backhoe hole when backfilled around a well casing is made up of disturbed materials and may not allow it to reflect conditions in the adjacent undisturbed materials.



GENERAL DIAGRAM
 NOT DRAWN TO SCALE
 PVC CASING IS ACCEPTABLE

SHALLOW MONITORING WELL FOR DIKED DISPOSAL AREAS

Figure 4. Dike monitor.



SINGLE COMPLETION MONITORING WELL

Figure 5. Single completion monitor.

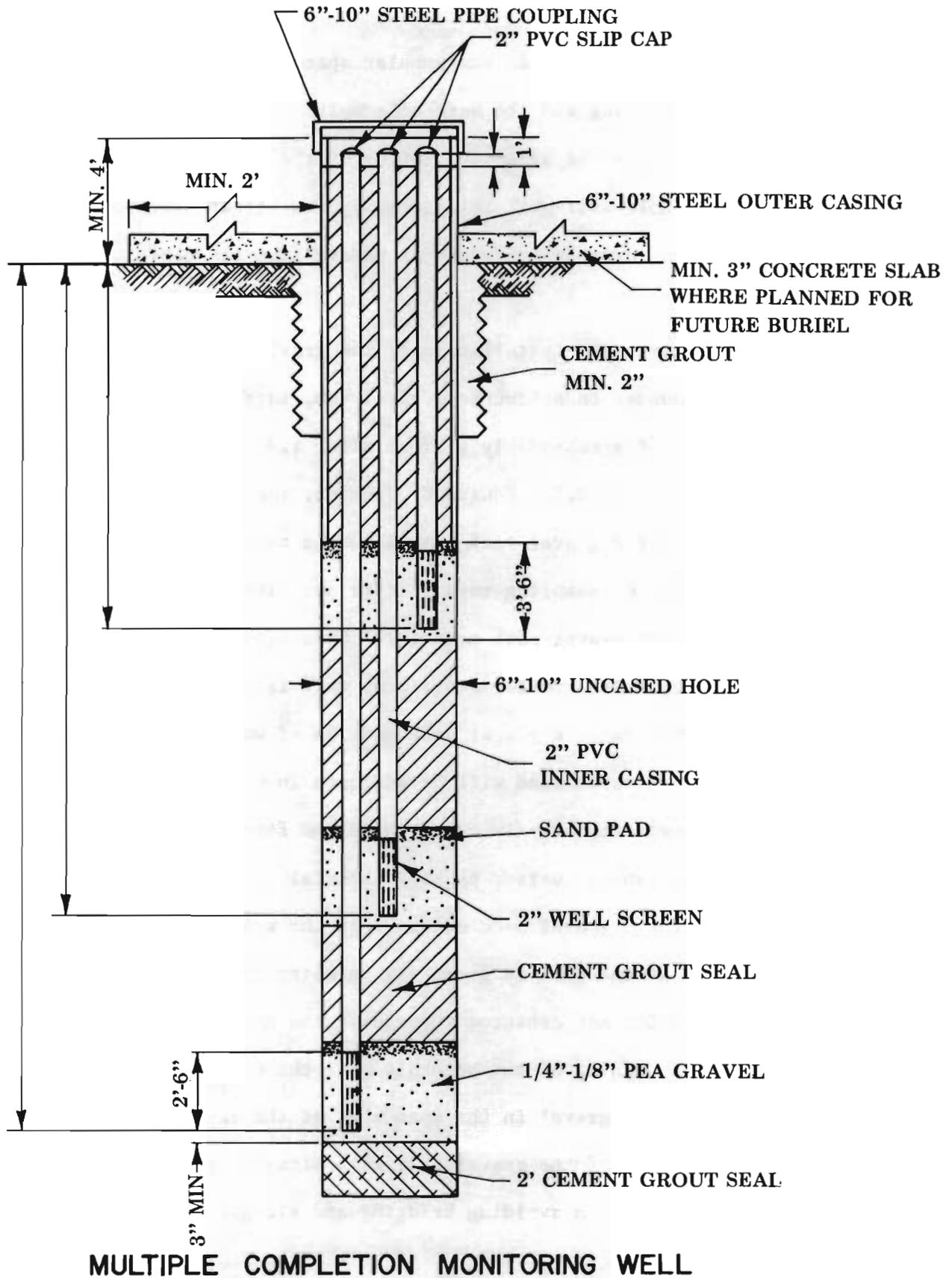


Figure 6. Multiple completion monitor.

Gravel Pack

Gravel packs are placed in the annular space between the screened or open portion of the casing and the bore hole walls. The purposes of the gravel pack are to increase the effective radius of the well, aid in maintaining a stable bore hole wall-well relationship, and filter fine sand, silt, and clay from the formation water as it passes from the undisturbed substrata into the well.

Proper selection and installation of the gravel pack material is important. Washed, rounded to subrounded, river run, sand or gravel should be used. It should be of a reasonably uniform size, i.e. with a uniformity coefficient of not more than 2.5. Edward E. Johnson, Inc. (1966) and Walton (1962) recommended that the gravel pack material be 6 to 9 times coarser than that in the aquifer or sampling zone. It is not always economical to sieve the aquifer and the gravel pack material. Consequently, most gravel packs installed in Oregon are grossly oversized, that is far too coarse gravel is used. In most cases a gravel pack made up of washed, fairly clean, quarter minus gravel or coarse sand will be adequate in monitoring wells. Coarser packs are recommended only in cases where the formation material is sieved and shown to warrant a coarser packing material.

Placement of the gravel pack material in the well annulus is of critical importance. If the well is deep, e.g. greater than twenty feet, bore hole rough, or casing not centered throughout the drillhole, bridging of the gravel and/or sloughing of the borehole wall can take place. This can result in little or no gravel in the open area of the casing, failure of the well seal, or plugging of the gravel pack with sloughed fines. The most common method employed in avoiding bridging and sloughing during gravel

pack installation is the use of a tremie pipe. In this method a suitable string of pipe is lowered to the zone to be gravel packed and the gravel poured or washed down the pipe. This method also has the advantage of minimizing differential settling and stratification or layering of the gravel as it passes through the water in the saturated portion of the borehole.

Well Seal

Water well seals are generally installed to inhibit cross-contamination of water in wells. Vertical leakage of surface or possibly contaminated ground water in the annular space around the well casing is the major problem. Another problem which can result from vertical leakage is the loss of water from one aquifer to another, e.g. artesian flow up the annular space. Vertical leakage is also a primary concern in monitoring wells. If a well is installed to measure conditions in a given zone, then it must be open to that zone and naturally the well must be sealed off from all other zones.

Usually, the annular space in the open zone is gravel or sand packed with suitably permeable materials. Placement of a fine sand on the top portion of the gravel pack, see Figures 4, 5, and 6, inhibits the drainage or settling of the sealing material into the gravel pack. If the sealing material gets into the gravel pack it may reduce its porosity and thus its hydraulic conductivity. The type of sealing materials for the remainder of the annular space are dependent on the type, final use and necessary permanency of the well. Backfill with cuttings or puddled clay may be adequate, e.g. in drainfield investigation monitoring wells, see Figure 2. However, in most wells a cement grout seal is recommended. Up to two per cent

bentonite can be added to the cement grout as a lubricant and in order to increase its fluidity. More bentonite than this may result in a reduction in the stability of the seal. In general, bentonite seals are not recommended for water quality monitoring wells. Bentonite is a clay which is pH sensitive and as mentioned in an earlier example, landfill leachate generally includes weak organic acids. In the presence of these acids the bentonite may lose some of its sealing properties. It should be emphasized that the well seal design and installation must not allow vertical migration of water in the annular space around the well casing or casings.

Development

Water and monitoring well development is directed at improving access from the drilled formation into the well. During drilling the borehole walls and later the gravel pack can become smeared, compacted, and/or partially plugged with drilling mud or native silts and clays. This is especially true during the installation of a drive point.

Bailing and/or pumping a well is the most common method of development. Bailing a water well usually removes the water at a relatively slow rate and is not commonly carried on for a long period of time. However, the surging action of the bailer entering and leaving the well aids in washing the gravel pack and thus developing the well.

Pumping or over-pumping a well is also commonly employed in development. Pumping the well at a rate in excess of the anticipated long term average discharge, over-pumping, results in a high entrance velocity of the water leaving the formation and flowing through the gravel pack and into the well. This high entrance velocity tends to wash and stabilize the gravel pack.

Proper employment of air compressors, when feasible, is perhaps the ideal tool in developing water wells. The compressor can be used to pump, over-pump, and surge the well. In addition, by raising and lowering the air and eductor lines the particular zone being pumped can be easily changed and more time spent on developing problematical zones, e.g. those areas with more fines in the formation.

The method and degree of development needed in monitoring wells will again be determined by the necessary amount of accuracy and type of information sought from the monitor as well as the hydrogeologic conditions in the area being monitored. For example, surging and/or limited bailing will generally be sufficient in developing subsurface sewage investigation monitors. On the other hand, complete development, equal to that of the pumping well, is recommended in aquifer performance tests. In water quality monitors sufficient development of the well to clear the hole and gravel pack of drilling mud and formation fines is necessary, especially when turbidity is one of the tests to be run.

CONCLUSION

The proper location, design, and installation of ground-water monitors is a complex and involved process. The validity of the data collected from the monitors is dependent on correctly tailoring well specifications to the needs and conditions at each disposal site. If these requirements are met, monitors allow physical sampling and measurement of field conditions. Interpretation of data collected from monitors can be invaluable in decision making processes.

A knowledge of geology, hydrology, soils, and chemistry are essential in the placement and design of monitors. Understanding drilling techniques and well hydraulics is also important in the correct installation and development of monitors.

The intention of this report has not been to discourage the use of monitors by those not having a full understanding of all the listed disciplines. On the contrary, it is merely an effort to point out many of the considerations involved in using ground-water monitoring wells. Like all types of field work, it is only through continued application of the work that one learns and becomes proficient at it.

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LIST OF OBSERVATION WELLS

WELL NO.	OWNER	RECORDS AVAILABLE	WELL NO.	OWNER	RECORDS AVAILABLE
GILLIAM COUNTY					
1N/22E-5db	Robert Root	1965-	26S/34E-19db	F. E. Jones	1959-
1N/22E-8ab	D. L. Lemon	1964-	27S/31E-1ach	Fred Briggs	1961-1968
3N/21E-28bb	City of Arlington	1964-	27S/33E-2bbb	R. F. Upton	1956-
3N/21E-34ab	E. M. Hulden	1964	27S/36E-33ac	Flolea Holly	1963-
GRANT COUNTY					
9S/30E-26ac	L. F. McGirr	1966-	31S/35E-1bb	Fred Fallock	1954-
10S/28E-8ac	Theron King	1966-	33S/34E-24bb	Alvord Ranch	1965-
11S/29E-25db	Cecil Bryant	1966-	34S/34E-15bc	Alvord Ranch	1962-
11S/30E-6aa	Frank McGirr	1966-	35S/33E-34dc	Andrew Shull	1958-
12S/26E-34da	Dayville Cemetery	1963-	36S/33E-3ab	Allied Properties Inc.	1962-
13S/29E-22ca	Curtice Martin	1962-	36S/36E-16dc	Gaylen Frazier	1969-
13S/30E-27aa	Oregon State Highway Dept.	1964-	37S/36E-10cb	Whitehorse Ranch	1962-
13S/31E-21cc	Mt. View Country Club	1963-	37S/36E-14aa	Whitehorse Ranch	1962-
13S/31E-23cb	City of John Day	1962-	37S/36E-15cb	Whitehorse Ranch	1962-
13S/31E-26bd	City of John Day	1962	39S/35E-4cc	Warren McLean	1964-
16S/30E-28dd	Robert Lemcke	1966-	39S/35E-25db	Trout Creek Ranch	1967-
17S/31E-2ba	Wesley Krieger	1966-	HOOD RIVER COUNTY		
18S/31E-12aa	Harry Pon	1966-	2N/8E-5ca	U. S. Forest Service	1965-
HARNEY COUNTY					
19S/31E-13ac	Harry Pon	1966-	2N/10E-4db(1)	W. B. Graham (White)	1964-
19S/31E-13da	Harry Pon	1966-	2N/10E-4db(2)	B. Ross Evans	1964-1969
22S/30E-27ddc	Werner Arntz	1966-	JACKSON COUNTY		
22S/31E-28dda	Harry Pon	1966-	36S/1W-21aa	Bud Hoover	1960-
22S/31E-34ccb	L. F. Lazaus	1930-	36S/2W-23ccb	U. S. Geological Survey	1953-
22S/31E-36bab	Robert Smith	1963-	36S/2W-25ad	Alex Legler	1960-
22S/32½E-30cdd	Jack McGee	1966-	37S/2W-28aab	So. Oregon Experimental Station	1960-
22S/33E-27adc	John Temple	1966-	38S/1W-15ca	City of Phoenix	1960-
23S/27E-30ab	Green Valley Ranch	1962-	JEFFERSON COUNTY		
23S/30E-36bbc	Baker Ranch	1971-	11S/12E-21aa	Portland General Electric Co.	1962-
23S/31E-3bbb	Harney County	1936-1971	11S/12E-26da	Portland General Electric Co.	1962-
23S/31E-5aac	Harry Pon	1962-	11S/13E-1bb	City of Madras	1964-
23S/31E-11dcc(1)	E. Sewell	1959-	12S/13E-6bb	Fred Degner	1964-1967
23S/31E-11dcc(2)	E. Sewell	1959-	12S/15E-21cc	Morrow Brothers	1965-
23S/31E-14aab	Harney County	1936-1971	13S/14E-33da	Buckner Brothers	1964-
23S/31E-16bcc	Harney County	1936-1971	JOSEPHINE COUNTY		
23S/31E-16dbb	T. Allen Jones	1930-	33S/6W-15cc	Southern Pacific Railroad	1963-
23S/31E-33ccc	Harney County	1936-1971	35S/6W-21aca	Bate Lumber Company	1961-
23S/32E-3aad	Meadowland Ranches Inc.	1965-	35S/6W-24bbd	Oregon State Highway Dept.	1963-
23S/32E-7cab	Dorland Ray	1928-	36S/5W-31cad	Grants Pass Golf Club Inc.	1964-
23S/32E-28aba	Bar Neg Ranch	1966-	36S/6W-27ac(1)	Fort Vancy Job Corps Center	1966-1967
23S/32E-29adb	Roy Duhaime	1968-	36S/6W-27ac(2)	Fort Vancy Job Corps Center	1966-
23S/32E-30ddd	Harney County	1936-1971	38S/5W-23ad	John Katzenbach	1961-1970
23S/32½E-1bbb	Dick & Ed McConville	1965-	38S/8W-33ad	J. R. Smith	1961-
23S/33E-36ad	Kenneth Mann	1966-1971	39S/5W-6db	Brown Bros. Lumber Co. Inc.	1962-
23S/34E-31add	Miller Bros. Ranch	1971-	39S/8W-17da	Joseph Ollis	1961-
24S/27E-11ba	Silver Creek Ranch	1968-	39S/8W-34caa	U. S. Geological Survey	1952-
24S/30E-7cdd	Adolf Kiele	1966-	KLAMATH COUNTY		
24S/30E-26ddc	John Campbell	1960-	25S/8E-1bb	U. S. Forest Service	1966-
24S/31E-28bcc	Harney County	1936-	27S/8E-20aa	U. S. Forest Service	1963-
24S/32½E-30ddd	Ansel Marshall (Catterson)	1963-	28S/8E-17dbd	Boise Cascade	1962-
24S/34E-31acb	John Rossberg	1959-	29S/8E-7ddd	Boise Cascade Corp.	1968-
24S/34E-31cbd	John Rossberg	1963-	30S/7E-11dab	Crown Zellerbach Inc.	1954-
24S/34E-31dcb	John Rossberg	1960-1969	31S/7E-1adc	Swanson Cattle Co.	1962-
24S/34E-31dda	John Rossberg	1959-1969	34S/7E-9adb	Collier State Park	1955-
25S/30E-34ab	Forrest Reed	1963-	34S/8E-28ddc	Rafter MD Ranch	1967-
25S/31E-4cba	James Stahl	1962-1970	35S/7E-34cb	H. C. Spicer	1965-
25S/31E-29ccb	Edgar Koeneman	1964-1970	35S/8E-1bcc	Henry Wolff	1954-
25S/34E-30dcc	Forrest Skinner	1960-	35S/10E-19aca	Ted Chrume	1954-
26S/31E-26bba	Harney Co. Land Development Corp.	1965-1968	35S/10E-29cab	G. D. Hazen	1967-
26S/31E-34ddd	Marcus J. Haines	1964-	36S/10E-14acc	C. J. Emmich	1962-
26S/33E-13daa	Lester Thompson	1962-	36S/12E-17dcc	Ernest Fireick	1967-
26S/33E-19ccc	De B. Forslund	1959-	36S/11E-20dca	Yanix Ranch (Bertran)	1962-
26S/33E-19ddc	De B. Forslund	1959-	36S/11E-36aab	Bill Gallaher (McBain)	1954-
26S/33E-28cbc	De B. Forslund	1958-1970	36S/12E-12dcc(1)	Weyerhaeuser Timber Co.	1962-
26S/33E-33baa	De B. Forslund	1958-			
26S/33E-34acc	Guy Leslie	1956			
26S/33E-34cca	George H. Merrick	1958-1970			
26S/34E-6ba	John J. Fecht	1960-			
26S/34E-6dab	H. C. Fitchett	1960-1968			

LIST OF OBSERVATION WELLS

WELL NO.	OWNER	RECORDS AVAILABLE	WELL NO.	OWNER	RECORDS AVAILABLE
36S/12E-14ccb	Jim Godowa (Rec. Hall)	1954-	26S/15E-20bb	U. S. B. L. M.	1940-
36S/12E-28adb	Mike Deely	1962-	26S/15E-20dc	Hugh L. Wahl	1956-1968
36S/14E-7add	Harry Obenchain	1962-	26S/15E-22abb	M. Y. Parke	1932-
36S/14E-25bcb	Gerber, Montgomery & Reed	1962-	26S/15E-28da	M. Y. Parks	1962-1964
36S/14E-27cdc	Henry Gerber	1962-	26S/15E-29ab	Delbert M. Wilson	1962-1964
36S/14E-35ddd	Ruth Hall	1962-1971	26S/15E-29bc	Hugh L. Wahl	1940-1970
37S/10E-8ccc	Edgewood Ranch	1961-	26S/15E-31aa(3)	Nick Klerk	1949-1970
37S/10E-18aca	Cliff McMillan	1961-1967	26S/15E-31bc	Nick Klerk	1949-1967
37S/10E-19ada	H. D. Whiteline	1961-	26S/15E-31da	Robert R. Tuttle	1956-1968
37S/10E-29dbb(1)	Edgewood Ranch	1949-	26S/15E-32bc(1)	Robert R. Tuttle	1956-
37S/10E-29dbb(2)	Edgewood Ranch	1949-	26S/15E-32bc(2)	Robert R. Tuttle	1956-
37S/10E-30abc	Fred Coleman	1961-	26S/15E-33ac	Merritt Parks	1962-1970
37S/11½E-36add	Donald Schreiner	1957-	26S/15E-34cc	Merritt Parks	1956-1970
38S/9E-28dc	Seventh Day Adventist Church	1968-	26S/15E-34dc	C. W. Boley	1956-1969
38S/10E-9cbc(1)	R. & S. Ranch	1949-	26S/18E-8ad	M. Penn Philips Co.	1968-
38S/10E-13bbb(1)	Swan Lake Ranch	1949-	26S/18E-26ab	Robert Bothner	1963-
38S/10E-16dcd	Mrs. Maude E. Liskey	1957-	26S/18E-30adc	Sam K. Morehouse	1959-
38S/10E-22baa	Mike Short	1957-	27S/15E-2bc(1)	C. C. Miles	1949-1968
38S/10E-23bdd	Lloyd Goldbek	1957-	27S/15E-2bc(2)	C. C. Miles	1956-
38S/10E-25aab(1)	Garrison Mitchell	1957-1971	27S/15E-2ad(2)	Lawrence B. Iverson	1949-
38S/11E-6dda	B. J. Jendrzejewski	1957-	27S/15E-3ab	Easton Claridge	1956-1968
38S/11½E-6cad	Swan Lake Ranch	1957-	27S/15E-3da	Easton Claridge	1949-1970
38S/11½E-7ddd	Swan Lake Ranch	1952-1970	27S/15E-4aca(1)	M. Y. Parks	1932-
38S/11½E-11ada	Frank & Mathilda Challis	1957-1971	27S/15E-4aca(2)	M. Y. Parks	1932-1968
38S/11½E-12cca(3)	Frank Challis	1949-	27S/15E-1lad	K. O. Butck	1961-1971
38S/11½E-13aca	R. M. Robertson	1948-1971	28S/15E-11da	K. O. Butck	1956-1970
38S/11½E-13ccc	Herman Pendegreft	1948-	27S/15E-13bb	Jess Miles	1949-
38S/11½E-15ddd	George McCollum	1948-	27S/15E-24ba	Darrell Bowen	1959-1970
38S/11½E-25cbb	Richard Hoefler	1949-1971	27S/15E-24cb	Darrell Bowen	1961-
38S/11½E-30cba	Swan Lake Ranch	1957-1971	27S/15E-24ccc	Darrell Bowen	1948-1969
38S/11½E-30ddd	David Moore (Wheeler)	1957-1971	27S/15E-25bc	Alfred Smith (Koehler)	1956-1970
38S/11½E-30dac	David Moore (Wheeler)	1948-	27S/16E-7bb	Wayne Dubois	1956-1968
39S/9E-34cd	Harry Wagner	1964-	27S/16E-7bd	Wayne Dubois	1961-1970
39S/10E-8da	Mt. Galvary Cemetery Ass'n	1965-	27S/16E-13aa	Robert Morehouse	1955-1970
39S/11E-10ddd	Lost River Cemetery	1963-	27S/16E-13abb	Robert Morehouse	1958- 1/
39S/11E-20aad	Robert Woods	1964-	27S/16E-13cb	Robert Morehouse	1955-1971
39S/11E-26abd(1)	H. D. Knox	1966-	27S/16E-26cd	John Beck	1963-
39S/11½E-8abc	L. J. Horton	1957-	27S/16E-32bd	Lowe & Whipple	1956-
39S/11½E-28ddd	B. E. Smith	1954-	27S/16E-32dc	Miller E. Follis	1957-
39S/12E-29dbc	Eva Adams	1954-	27S/16E-34cab	Dr. George E. Mallet	1957-1970
39S/12E-35add	Cummings	1954-	27S/17E-10dd	M. Penn Philips Co.	1961-
40S/9E-27cd	Otis Osborn	1968-	27S/17E-14cc	U. S. B. L. M.	1961-1970
40S/11E-11bad	A. W. Schaup	1954-	27S/17E-21dc	James Bobst	1962-1966
40S/11E-13bda(2)	Carl Rajnus	1963-	27S/17E-22bb	Helmar Gustafson	1962-1970
40S/11E-29ab	J. Randall Pope	1968-	27S/17E-22ddd	Century Ranch	1938-1970
40S/12E-33ba	Clark Unruh	1965-	27S/17E-27ddb	Century Ranch	1961-1968
41S/9E-12aa	O'Conner Livestock Co.	1968	27S/17E-27bdd	Century Ranch	1952-
41S/10E-10ca	Fotheringham Brothers	1964-1966	27S/18E-6bcb	Rose T. Morici	1940-
41S/12E-3cba	George Rajnus	1954-	27S/18E-6bdb	Marianne Aiasa	1959-1968
41S/12E-19ac	D. P. Reid	1954-	27S/18E-6ac	Clinton B. Carrico	1956-1968
41S/14E-8cca	Charles Kilgore	1954-	27S/18E-6ad	Hugh L. Wahl	1962-1964
			27S/18E-6db	Clinton B. Carrico	1956-
			27S/18E-12aaa	John Pettus	1961-1971
			27S/18E-18cc	Jack Gillette	1959-1970
			27S/18E-21aaa	U. S. B. L. M.	1955-
			27S/19E-18cc	View Point Ranch	1957-
			27S/19E-19dc	View Point Ranch	1957-
			27S/19E-29bd	View Point Ranch	1961-1968
			27S/19E-32ba	View Point Ranch	1958-1968
			28S/14E-20ab	Lawrence Iverson	1965-
			28S/14E-21dcb	U. S. Forest Service	1963-1970
			28S/14E-25ba	ZX Ranch	1964-
			28S/15E-13cd	View Point Ranch	1958-1969
			28S/15E-14adc	View Point Ranch	1956-
			28S/16E-4ab	U. S. B. L. M.	1958-1971
			28S/16E-4bb	U. S. B. L. M.	1956-1970
			28S/16E-5bd	Miller E. Follis	1959-1967
			28S/16E-5cd	Miller E. Follis	1959-1970
			28S/16E-5dc	Miller E. Follis	1959-1967
			28S/16E-19ba	View Point Ranch	1960-1970
			29S/23E-3dac	U. S. B. L. M.	1945-
			30S/16E-1bd	Oregon State Game Commission	1963-
			32S/23E-5aa	U. S. B. L. M.	1964-
			33S/18E-13aa	Alan & Van Withers	1963-
			34S/19E-16cc	Howard Beachler	1962-
			34S/19E-23ac	ZX Ranch	1959-
			35S/24E-9dad	U. S. B. L. M.	1949-
			36S/21E-6aba	S. V. Carroll	1950-
LAKE COUNTY					
25S/14E-15bcc	Ira A. Dutcher	1932-			
25S/14E-16ad	Marion Cook	1956-1971			
25S/14E-19dd	R. A. Long	1948-1967			
25S/14E-22bb	C. B. Webster	1949-1970			
25S/14E-29da	Banfield Veterans Hospital	1956-			
25S/14E-36cc	Joe & Minnie Stitz	1956-1970			
25S/18E-9cc	Alferd L. Prevost	1968-			
25S/19E-31dc	O. E. White	1963-			
26S/14E-2bb	U. S. B. L. M.	1948-1970			
26S/14E-3bb	Fred Meyers	1956-			
26S/14E-5aa	V. A. Wagers	1940-1970			
26S/14E-12aa	Sheldon D. Kelley	1948-1969			
26S/14E-12ba	Elmer Kohler	1958- 1/			
26S/14E-13cc	Andrew Bettencourt	1957-1971			
26S/14E-20cc	G. R. Boatwright	1940-1970			
26S/14E-21dd(2)	U. S. B. L. M.	1940-1970			
26S/14E-23bb	U. S. B. L. M.	1959-			
26S/14E-24cd	Jack Kittridge	1956-1970			
26S/14E-33ba	U. S. B. L. M.	1940-1970			
26S/15E-5bab	N. J. Widendja	1957-1970			
26S/15E-6ab	Glenn Irwin	1955-			
26S/15E-6ad	Glenn Irwin	1960-1967			
26S/15E-17dd	Morehouse Ranch	1940-1971			

LIST OF OBSERVATION WELLS

WELL NO.	OWNER	RECORDS AVAILABLE	WELL NO.	OWNER	RECORDS AVAILABLE
36S/24E-4ab	Joe Banasco	1962-	11S/3W-15bda	Joe Kennel	1962-
36S/24E-27bb	Con Lynch	1961-1971	11S/3W-26aaa	Leonard Roth	1962-
36S/24E-28cbb	Lloyd Grisel	1949-	11S/4W-24cbd	Raymond Maddy	1969-
36S/24E-28cc	Joe Rombo	1960-1968	12S/1W-29cca	Pineway Golf Club Inc.	1964-
36S/24E-32aab	James Kiely	1940-1970	12S/1W-29cd	Hubert J. Griffiths	1964-
36S/24E-33ab	Con Taylor Ranch Inc.	1960-	12S/1W-30ddb	Pineway Golf Club Inc.	1962-
37S/20E-34dd	Robert Weir Jr.	1962-	12S/2W-2bad	Ken Watters	1962-
38S/20E-33bd	Jack Stookesberry	1964-	12S/2W-14abc	Wilbur Parrish	1941-1967
38S/24E-27cbb	Charles Crump	1948-	12S/2W-18bad	Henry DeManette	1962-
39S/19E-13dc	Elmo Angeles	1968-	12S/3W-5caa	George N. Chandler	1962-
39S/19E-34add	William Hoffman	1960-	12S/3W-29cdd	R. L. Wirth	1962-
39S/20E-9aa	Lakeview Mining	1965-	12S/4W-21cab	R. C. Mang	1962-
39S/24E-21bd	J. G. Dyke	1948-	12S/4W-35cdc	Paul Pugh	1962-
40S/19E-19db	Clyde Fenimore	1965-	13S/3W-13aaa	Frank Cochran	1962-
40S/20E-14bba	Snider & Alexia	1957-1970	13S/3W-34ccd	Jacob Ogle	1928-1969
40S/20E-26ca	Neal Elliott	1959-	13S/3W-36ad	Thomas P. Irwin	1962-
LANE COUNTY					
16S/2W-34cdc	M. A. Nadiou	1962-	13S/1E-27ba(1)	Verdge Stephenson	1964-1969
16S/3W-acd	Leo Sidwell	1950-	13S/1E-27ba(2)	Les Austin	1964-1969
16S/3W-32dbd	Leon Funke	1966-	13S/1E-27ba(3)	U. S. Government	1964-1965
16S/4W-4ab	John Hentze	1965-	13S/1E-27bb(1)	Roy Lewis	1964-
16S/4W-16 cac	Shadow Hills Golf Course	1965-	13S/1E-27bb(2)	Denver Davis	1964-1969
16S/4W-29da	Elbert Hill	1965-	13S/1E-27bd	U. S. Government	1964-1969
17S/1W-29ac	Glen Vaughn	1965-	13S/1E-27ac(1)	U. S. Government	1964-1969
17S/1W-29dd	Ed Hull	1965-	13S/1E-27ac(2)	U. S. Government	1964-1966
17S/2W-32bad	Weyerhaeuser Timber Co.	1962-	13S/1E-27ac(3)	Paul Tucker	1964-1969
17S/3W-17bca	Smith Gardens	1962-	13S/1E-27da	U. S. Government	1964-
17S/3W-19cac	Gheen Irrigation Works	1966-	13S/1E-27db(1)	Warren Vasey	1964-1969
17S/3W-22aa	C. E. Blakely	1966-	13S/1E-27db(2)	U. S. Government	1964-1969
17S/3W-28dcb	Chase Gardens	1962-	13S/1E-27db(3)	Lawrence Alvin	1964-
17S/4W-4dcb	Seneca Sawmill	1962-	14S/2W-10bd	J. I. McCord	1970-
17S/4W-33aba	West Lawn Memorial Cemetery	1962-	14S/3W-7ddc	Henry H. Kirk	1962-
17S/5W-13bdd	Leo Burtis	1962-	15S/3W-19acd	Edgar B. Grimes	1962-
17S/6W-36dca	Oscar E. Williams	1963-	MALHEUR COUNTY		
18S/2W-27bd	George Potter	1965-	14S/38E-28aa	Jerry Farley	1964-
18S/2W-35ca	Richard Lyday	1965-	14S/39E-21bd(2)	Ralph Duncan	1962-
18S/4W-3cad	Nils Hult	1963-	14S/39E-21dc	Mary J. Molthan	1962-
18S/5W-19ac	Crow-Applegate School Dist. 66	1966-	14S/39E-29ba	John Molthan	1962-
18S/10W-11aa	United States Plywood	1963-	14S/39E-29bc	John Molthan	1962-
18S/12W-14cdd(4)	U. S. Geological Survey	1961-	14S/39E-32ad	Ray Duncan	1962-
18S/12W-34dc	Camp Florence	1964-	15S/40E-1bad	Altha Anderson	1953-
19S/2W-7dd	J. L. Getchell	1962-	15S/40E-2das	Max Holloway	1949-
19S/3W-3cc	Oregon State Game Commission	1962-	15S/40E-2cad	Max Holloway	1955-
19S/3W-11ba	O. C. Luchterhand	1963-	15S/40E-2cba	Max Holloway	1952-
20S/3W-11ba	Gettings Creek Rest Area	1966-	15S/40E-2ccb	Rankin Crow	1950-
21S/3E-16da	City of Oakridge	1966-	15S/40E-2dcc	Rankin Crow	1956-1969
LINCOLN COUNTY					
6S/10W-33bd	A. A. Corkhill	1964-	15S/40E-10abc	Rankin Crow	1950-
8S/10W-17dbd	A. E. Howard	1964-	15S/40E-10dbc	Rankin Crow	1953-
10S/10W-3cbb	Don Preasey	1964-	15S/40E-11cdb	Rankin Crow	1950-
11S/9W-9ad	Eddyville High School	1964-	15S/40E-12cbb	Guss Davis	1955-
11S/10W-20ca	Joe W. Brown	1964-	15S/40E-12dcc	Guss Davis	1954-
LINN COUNTY					
9S/1W-14dca	John Fery	1964-	15S/40E-13bba	Guss Davis	1955-
9S/1W-23cda	Charles Hecht	1962-	15S/40E-13acc	Guss Davis	1955-
9S/2E-27bdb	North Santiam Plywood Co.	1966-	15S/40E-14dcb	Rankin Crow	1951-
10S/1W-5cab	A. M. Hendrickson	1962-	15S/41E-6cbc	Rankin Crow	1958-
10S/1W-28bdb	Grant Farris	1962-	15S/41E-8cbc	Rankin Crow	1951-
10S/2W-8cca	William Uppstad	1957-	15S/42E-25ab	Mark J. Velsmeyer	1961-
10S/2W-12cc	Red Crown Mills	1962-	16S/43E-5db	Estel B. Moser	1966-
10S/2W-21ddd	H. C. Robertson	1962-	16S/43E-16dc	Ralph Altig	1962-
10S/3W-21cd	Walter Powell	1970-	17S/44E-11db	John Stringer	1961-
10S/4W-12bdd	Henry Hoefler	1928-	17S/44E-25ad	C. N. Durrett	1961-
11S/2W-3bbc	Sam Looney	1962-	18S/41E-8dca	Roy C. Stewart	1963-
11S/2W-6abd	George C. Scheler	1962-	18S/41E-15cc	Grady Romans	1968-
11S/2W-26bdd	Marvin Ufford	1966-	18S/44E-18ac	Paul Fleming	1965-
11S/2W-29adc	Neal Hollingsworth	1962-	18S/45E-21bb	K. T. Loomis	1962-
11S/3W-13aaa	Loren A. Nelson	1962-	18S/46E-19cc	Glen Hutchinson	1962-
			18S/46E-23dc	Kay Teramura	1962-
			18S/47E-17bbb	Earl Weaver	1950-
			19S/42E-35bbb	John E. O'Toole	1963-
			19S/43E-2cb	Trenkel Brothers	1961-
			19S/43E-3bc	Floyd Baughn	1961-
			19S/43E-3db	Floyd Baughn	1961-
			19S/43E-10ad	Thomas J. Davis	1961-
			20S/46E-28ac	George Mendazona	1962-
			21S/38E-17dca	Walter Bodkin	1955-

LIST OF OBSERVATION WELLS

WELL NO.	OWNER	RECORDS AVAILABLE	WELL NO.	OWNER	RECORDS AVAILABLE
MULTNOMAH COUNTY					
1N/1E-33bbd	Good Samaritan Hospital	1961-1970	2S/9W-5bc	Tillamook Water Comm.	1962-
1N/1E-33dbc	Fred Meyer Inc.	1961-1967	2S/9W-6ac	M. J. Jenck	1962-1971
1N/1E-33dc	U. S. National Bank	1946-	2S/9W-6da	Connie & Judy Dye	1962-1969
1N/1E-33dc	Weisfield's Inc.	1940-	2S/9W-21bc	Vern Darby	1962-
1N/1E-34cdc(3)	Equitable Savings & Loan	1961-	4S/10W-19da	Lloyd McKillip	1968-
1N/1E-34ccb(3)	Pittock Block Inc.	1961-1969	4S/10W-27bd	James C. Trent	1965-
1N/1E-34cdc(6)	Lipman Wolfe & Co.	1961-	UMATILLA COUNTY		
1N/1E-34ccb(1)	Dirks Medical Center	1961-	1N/32E-10db	Skyview Memorial Park	1965-
1N/1E-34ccb(3)	Federal Reserve Bank	1961-1967	1N/32E-22acc	J. L. Eldridge	1967-
1N/1E-34cca(13)	U. S. National Bank	1961-	2N/27E-1bda	Claussie Ammon	1961-
1N/1E-34cdc (7)	First National Bank	1959-1969	2N/27E-11add	Clarence L. Hansen	1961-
1N/2E-9cc	Port of Portland	1963-	2N/27E-12bba	Clarence L. Hansen	1961-
1N/2E-23aa	Joe A. Grosjaquea	1970-	2N/27E-14cc	Oscar D. McCarty	1968-
1N/3E-26dd	Kazuo Fujii	1963-	2N/27E-26cad	Sarvis Springs Ranch	1962-
1S/1E-3bbd	Pacific Power & Light Co.	1961-1967	2N/27E-27bca	John F. Kilkenny	1961-1969
1S/1E-3bba(2)	Pacific Service Building	1961-	2N/27E-27cbc	Thomas Ashbeck	1968-
1S/1E-3bcb(1)	Oregonian Publishing Co.	1961-	2N/28E-7aad	Oliver Abney	1969-
1S/1E-3bcb(2)	Oregonian Publishing Co.	1961-	2N/30E-28bdc	Cunningham Sheep Co.	1961-
1S/1E-3bcb(3)	Oregonian Publishing Co.	1961-1968	2N/32E-2ccc	City of Pendleton	1961
1S/1E-3cbb(1)	Pacific Northwest Bell	1963-	2N/32E-16ba	City of Pendleton	1967-
1S/1E-3cbb(2)	Pacific Northwest Bell	1963-	3N/27E-ddd	George Wallace	1961-
1S/1E-4aa(2)	Medical-Dental Building	1958-1969	3N/27E-36adc	George Wallace	1961-
1S/1E-10cd	J. Donald Kroeker	1961-	3N/28E-8aa	Henry Walker	1967-
1S/3E-7cc	Meadowland Dairy	1964-	3N/28E-28cbb	E. A. Betz	1967-
1S/3E-10cc	Forest Lawn Memorial Park	1963-	3N/34E-3bac	Berkley Davis	1953-
POLK COUNTY					
6S/3W-7ada	D. L. Gingerich	1963-	4N/27E-22cda	Umatilla Army Depot	1960-
6S/4W-17cba	John Romig	1963-	4N/27E-22dbd	Umatilla Army Depot	1960-
7S/3W-10bcd	Leland P. Brandt	1962-	4N/27E-27da(1)	Hansell Brothers	1965-
7S/3W-17db	Robert W. Straub	1969-	4N/27E-27da(2)	Hansell Brothers	1967-
7S/3W-18bbd(1)	Orchard Heights Water District	1965-	4N/27E-34bb	Marvin McDole	1966-
7S/3W-18bbd(2)	Orchard Heights Water District	1965-	4N/27E-36bba	Lyle Miller	1961-
7S/3W-30bd(1)	R. L. Forster	1965-	4N/28E-10bda	City of Hermiston	1964-
7S/3W-30bd(2)	R. L. Forster	1966-1968	4N/29E-5dda	John Hershey	1963-
7S/4W-6ddb	Art Leppin	1965-	4N/29E-17cc(2)	Milton Culp	1961-
8S/4W-3ab	Theodore C. Muller	1962-	4N/29E-18daa	Milton Culp	1961-
8S/4W-21bc(2)	Boise Cascade Corp.	1962-	4N/29E-32cac	City of Stanfield	1964-
8S/5W-13ad	Milo Jensen	1963-	4N/32E-2cb	L. King	1953-
8S/6W-22bd	Edward Bakke	1962-	5N/28E-22bba	L. J. Martin (Munson)	1953-
8S/6W-25dd	Max B. Morton	1963-	5N/34E-16ddd	R. M. Thompson	1953-
9S/4W-8cc	D. W. Christensen	1963-	5N/35E-1bad	William A. Bingman	1933-
9S/4W-11bd	Donald Gobine	1965-	5N/35E-1bcc	City of Milton-Freewater	1963-
SHERMAN COUNTY					
1N/17E-4ca	Philip G. O'Meara	1963-	5N/35E-2bad	K. A. Townsend	1933-
1N/17E-16ba	L. P. Haven	1964-	5N/35E-2ca	City of Milton-Freewater	1954-
2N/16E-8dc	Isami Taubota	1968-	5N/35E-2daa	City of Milton-Freewater	1954-
3N/17E-31da	Isami Taubota	1969-	5N/35E-3ada	Walter Miller	1933-
3S/16E-10cc	E. J. Hartley	1963-	5N/35E-12bdb	City of Milton-Freewater	1954-
TILLAMOOK COUNTY					
1S/9W-18cb	Green Acres Motel	1962-1970	5N/35E-12bdd	City of Milton-Freewater	1954-
1S/9W-19ba	Tillamook Veneer	1962-	5N/36E-18cb	City of Milton-Freewater	1964-1966
1S/9W-27ab	Gaylord Shively	1962-	6N/35E-14cad	Conrad Miller	1933-
1S/9W-27bc	Virgil Chadwick	1962-1971	6N/35E-20aca	J. E. Courtney	1933-
1S/9W-29bc	Norman Burdick	1962-1971	6N/35E-24dcc	George H. Ransom	1933-
1S/9W-29db	Fairview Water Dist. Inc.	1962-	6N/35E-26bad	Earl Ransom	1933-
1S/9W-29ca	Fairview Water Dist. Inc.	1962-1969	6N/35E-28ada	W. J. Rand	1933-1969
1S/9W-31bc	Lester E. Armstrong	1962-1971	6N/35E-28ccd	Lottie McKnight	1933-
1S/9W-31cd	Tillamook Water Comm.	1962-	6N/35E-30cbb	Dan Selleck	1933-
1S/9W-32cd	Elbert Leonnig	1962-	6N/35E-34baa	James A. Reese	1933-
1S/10W-12bb	Leo & Robert Wassmer	1962-	6N/35E-36bab	James Busch	1933-
1S/10W-26ab	Rudolph John Fenk	1962-	6N/35E-36add	Walter Herman	1933-
1S/10W-36dd	Frank Emmenegger	1962-1971	6N/36E-20db(2)	D. K. Smith	1966-
UNION COUNTY					
			1S/32E-9cad	Wayne Chapman	1961-1969
			1S/32E-17adb	City of Pilot Rock	1961-
			1S/32E-17dba	City of Pilot Rock	1961-
			1S/32E-19cd	Arnold Hoeft	1964-
			1S/32E-23daa	Hilmer Horn	1961-
			3S/30½E-1abb	Joseph Pedro	1953-
			1N/39E-4cb	Lee Smith	1964-

APPENDIX B

Subsurface Sewage Suitability Monitoring Example

The Camp Sherman - Metolius Meadows area is located in a ground-water discharge zone. Underlying the area are glacial outwash materials including buried stream channels. This combination has resulted in innumerable springs and seeps which discharge ground water to the surface and the streams in the area.

In early 1973, 58 backhoe test holes were excavated at the proposed Metolius Meadows Subdivision by the developer. These holes were excavated at the request of the State Health Division, then the state regulatory agency for subsurface disposal systems. The test holes were backfilled around four inch PVC pipes which were to be employed in measuring the depth to water throughout the winter months.

A number of problems were encountered in using the monitors. First, the winter of 1973 was one of the driest on record and did not reflect average conditions. A backfill seal around backhoe pit monitors is generally not adequate as explained in the text of this report. The monitors were completed at a variety of depths and were open to various depth zones. Therefore, they may have collected some perched water and/or not have accurately reflected the flow system and the water table (saturated zone) at the site. Many of the monitors were destroyed during development of the subdivision's water and road system in the summer of 1973.

As mentioned earlier, the Department of Environmental Quality was given the responsibility for regulating subsurface sewage disposal in early 1974. They requested that the developer carry out a detailed subsurface investigation of the property, including the installation of new

monitors in an acceptable manner. Twenty-five monitoring wells (as shown on Figure 2) were installed in early March of 1974. The monitor locations, new backhoe pits, those monitors not destroyed by construction, and the nearby Forest Service monitors are shown on Figure 7.

All monitors at the site are being measured weekly. The data collected here, together with the borehole logs, will be used to evaluate the acceptability of specific areas within the development for subsurface disposal. The developer has agreed to leave the monitors in place (they are generally located at property corners) so that any affect that future subsurface disposal might have on the local ground-water body can be measured.

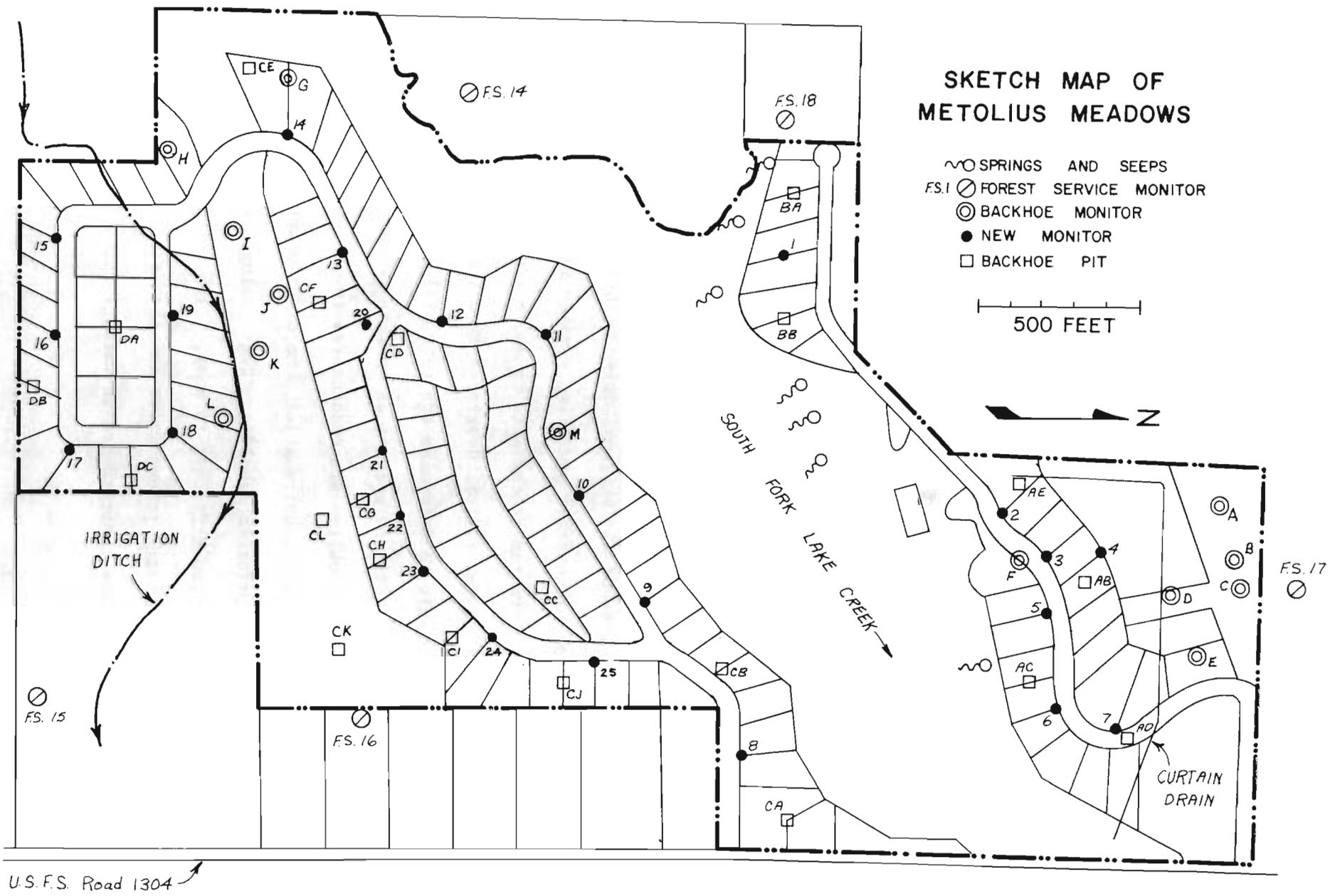


Figure 7. Sketch map of Metolius Meadows.

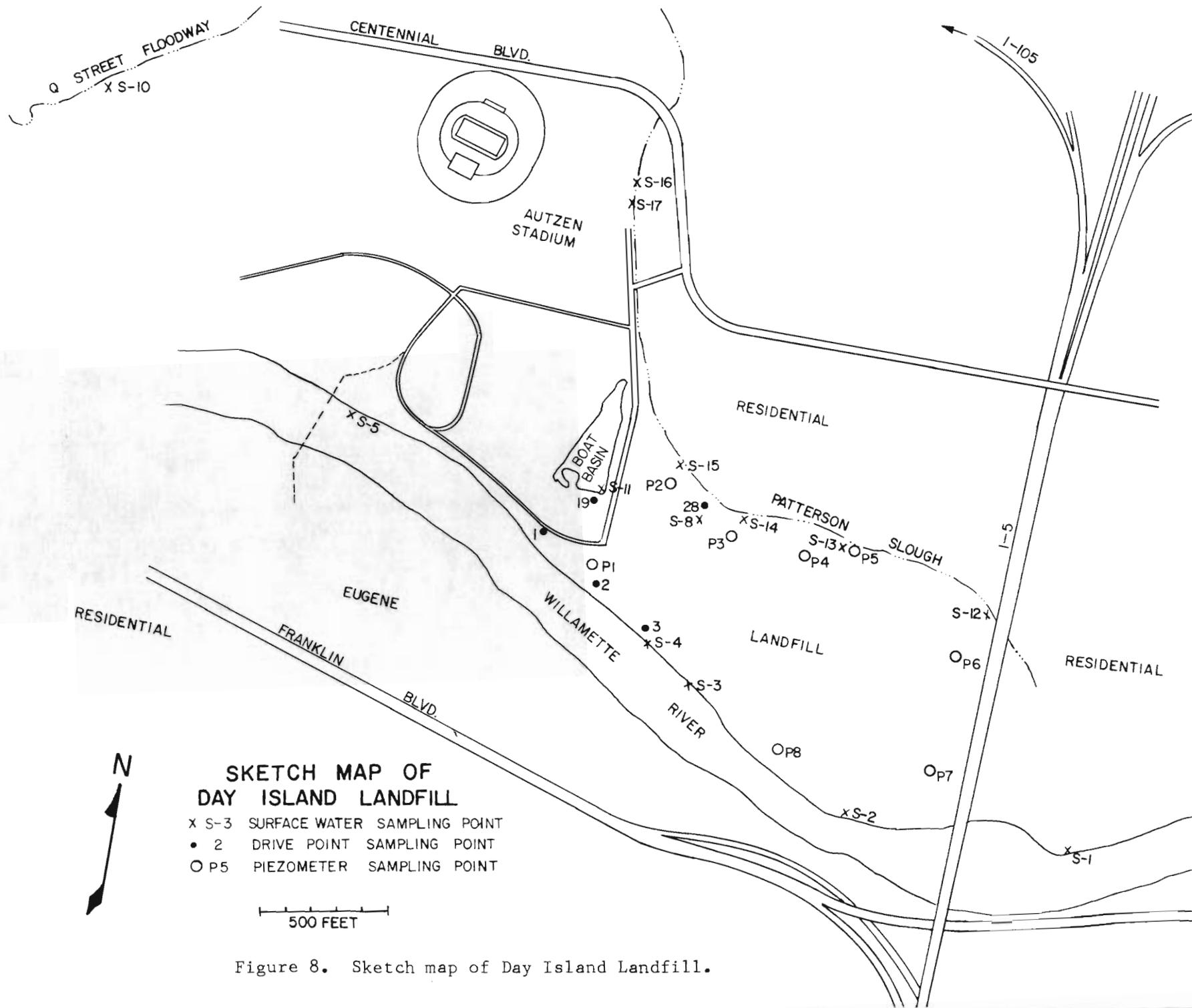
APPENDIX C

Landfill Monitoring Example

In 1969 a series of well points were installed at the Day Island Landfill in order to begin a subsurface leachate sampling program. The well point monitors consisted of six inch, pointed, black iron well pipe, perforated with one-half inch round holes on three sides at two foot intervals. They were installed employing a modified pile driver. When cobbles or boulder sized rocks were encountered the points tended to deflect.

Some of the original monitoring wells have been destroyed or buried as the landfill expanded. A contract with the D.E.Q. allowed the use of grant funds to expand and improve the monitoring well system at Day Island Landfill in 1973. This expansion included the installation of eight new double and triple completion monitoring wells similar to those shown in Figure 6 of this report. The present layout of monitors at Day Island showing the location of four of the original drive points as well as the eight multiple completion piezometers now being sampled is shown in Figure 8. A continuing sampling program at the site is under way and projected into the future as the completed landfill is developed into a park complex. This program will make it possible to determine the rate of leachate production, quality, direction of movement, and spatial as well as temporal attenuation.

Several sampling problems are encountered in the monitoring wells. Those wells deeper than 25 feet require a jet pump or draw cylinder to properly evacuate the well prior to sampling. Bent casings or those which deviate from vertical are often difficult to pump. Also, chemical reactions between the leachate contaminated ground water and the iron well casing may have biased test data for some constituents in water samples taken from the driven wells.



SKETCH MAP OF DAY ISLAND LANDFILL

- X S-3 SURFACE WATER SAMPLING POINT
- 2 DRIVE POINT SAMPLING POINT
- O P5 PIEZOMETER SAMPLING POINT

500 FEET

Figure 8. Sketch map of Day Island Landfill.