

TM400 Series: Signals

TM450 – Mast Arm Pole Details TM452 – Temporary Wood Strain Pole Details TM453 – Temporary Pedestrian Wood Post, Guy Wire/Anchor, & Luminaire Arm Details TM454 – Temporary Controller Cabinet, Service Cabinet, Meter Base, & Terminal Cabinet TM456 – Temporary Spanwire Mounting Details For Vehicle Signals, Signs & Fire Preemption TM457 – Pedestal Foundation and Traffic Signal Assembly TM460 – Vehicle Signal Details TM462 – Vehicle Signal Bracket & Sign Bracket (Type B) Details TM466 – Radar Mounting Details TM467 – Pedestrian Signal Mount and Pedestrian Pushbutton Details TM470 – Wire & Cable Installation TM471 – Trenching & Conduit Installation TM472 – Junction Boxes/Hand Holes TM482 – Controller Cabinet & Service Cabinet Foundation Details TM485 – Service Cabinet Wiring Details TM492 – Ramp Meter Assemblies TM493 – Rectangular Rapid Flashing Beacon (RRFB) Assemblies

TM600 Series: Sign, Illumination, and Signal Support Structures

- TM650 Traffic Signal Supports General Details & Design Criteria TM651 – Traffic Signal Supports Notes and Reactions TM652 – Traffic Signal Supports Steel Details TM653 – Traffic Signal Supports Foundation Requirements TM654 – Traffic Signal Pole Recessed Terminal Cabinet TM655 – Traffic Signal 60' through 75' Mast Arm Supports General Details & Design Criteria TM656 - Traffic Signal 60' through 75' Mast Arm Supports Notes TM657 – Traffic Signal 60' through 75' Mast Arm Supports Steel Details (SH. 1)
- TM658 Traffic Signal 60' through 75' Mast Arm Supports Steel Details (SH. 2)

RD700 Series: Curbs, Islands, Sidewalks, and Driveways

RD720 – Curb Line Sidewalks

TM628 – Std. Monotube Sign/VMS Support Drilled Shaft Details (only used for 60' through 70' mast arms supports)







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TM453



dgn 7M454

2023



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02-JUL-2018

FM460.dgn

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र	Red Circular Ball
(Yellow Circular Ball
Ĵ	Green Circular Ball
A	Red Arrow
A	Yellow Arrow
A	Green Arrow
Ά	Flashing Yellow Arrow
R	Flashing Red Circular Ball
Y	Flashing Yellow Circular Ball

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M470

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STANDARD SIGNAL ARM LOADS	SIGNAL ARMS DEFLECTION			
Signal Pole Type Signal Arm Length Signals Sign DS Max. for S2 SM1 or SM1L 15' 1 0 1 2 0 N/A SM2 or SM2L 20' 1 1 1 3 0 N/A SM3 or SM3L 30' 1 1 1 3 0 N/A SM3 or SM3L 30' 1 1 1 3 1 9'-1" SM4 or SM4L 40' 1 2 1 4 1 11'-1" SM5 or SM5L 50' 1 2 1 4 1 11'-1" SM5 or SM5L 50' 1 2 1 4 1 21'-1" * - Load location is the closest sign or signal of that type to the vertical post. signal of that type signal of that type signal of that type	Signal Arm Allowable Dead Total Load Length Load Load '5A" Deflection Deflection 15' or less 0.01"SA" 0.05"SA" 20' $2l_2$ " 12" 25' $3l_2$ " 15" 30' 5" 21" 35' 7" 29" 40' $9l_2$ " 38" 45' $1'-l_2$ " 48" 50' 1'-4" 60" 55' 1'-8" 74"		VERTICDescriptionMaximum Centerline ElevationHeig (Eac2-Ped. Push Buttons3'-6"7%Controller Cabinet5'-9"462-Pedestrian Signals $8'-3\sqrt[3]{2}$ "18%Terminal Cabinet10'-9"18%Guide Sign (S3)15'-0"72Photoelectric Cell38'-4"2%1. Physical fit of the loading must be verified	$\begin{array}{c c c c c c c c c c c c c c c c c c c $
 Camera mounted on 6 ft arm placed at any location on signal arm. Fire Pre-Emption may be placed at any location along the mast arm. Loads stated in the table produce reactions as shown in tables on TM651. Modifications to the loading shown require analysis to verify the structural adequacy of the pole. Physical fit of the loading must be verified. 	<i>Luminaire arm</i> <i>length "LA"</i> (20'-0" maximun) <i>Centroid of</i> <i>Design Luminaire</i>	Install removable steel or aluminum raintight pole cap.	$\begin{array}{c cccc} & & & & & & & \\ \hline & & & & & & \\ \hline & & & &$	+ + S2 ated Aluminum S3 5 ft) (Street Name Sign) (6 ft x 10 ft)
Install removable steel or aluminum raintight cap at the end of mast arm	Camera mounted on J 1'-6" max. arm Length (See Project Plans) nin. 9'-9" min. DS max. """"""""""""""""""""""""""""""""""""	See Luminaire Arm Connection Details on TM629. See Mastarm Connection Details drawing TM652 (Only for SM#L's) (Only for SM#L's) (Only for SM#L's) (Only for SM#L's)	SIGNAL POL APA Type From (sq. f 4L 12.4 2 8.67 4 11.0 5 13.3 S1 7.50 S2 21.0	PURTENANCE LOADS a Area Area Weight t Side Bottom 0" (ce t/ (sq. ft) (sq. ft) (lbs) 4 6.61 3.64 145 7 6.61 1.95 85.0 0 8.49 1.95 97.0 3 10.36 1.95 142 0 2.38 1.72 71.0 0 0.00 1.67 105
18'-0" minimum (19'- Vertical Cle	Type S3, (3)–S2, or (4)–S1 Recessed Terminal Cabinet, see TM654 See Handhole details on drawing TM652 (Orientation specified by signal designer) 1'–6"	Attachment Height = 1	Accompanied by dwgs. TM651, TM652, TM65 The selection and use of this Standard Drawing, while designed in accordance with generally accepted engineering principles and practices, is the	53, TM654, TM679 All materials shall be in accordance with the current Oregon Standard Specifications. OREGON STANDARD DRAWINGS TRAFFIC SIGNAL SUPPORT GENERAL DETAILS & DESIGN CRITERIA
TYPICAL POLE	ELEVATION <i>ELEVATION</i> <i>for base plate & footing details.</i>		sole responsibility of the user and should not be used without first consulting a Registered Professional Engineer.	ATE REVISION DESCRIPTION -2020 REPLACED HUB WITH RECESSED TERMINAL CABINET AND ADDED ACCOMPANIED BY DRAWING TM654

10-JUL-2020

TM650.dgn

GENERAL NOTES

- 1. Signal supports shall be designed in accordance with the AASHTO Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals 4th edition, 2001, 2002, and 2003 interim revisions.
- 2. All traffic signal supports shall conform to the design criteria and details shown on these drawings except as approved by the Engineer.
- 3. The design basic wind speed (3 second gust) shall be 110 mph, gust factor G = 1.14, Ir = 1.0 (50 year recurrence interval), Fatigue Category II, no galloping, and truck speed = 55 mph.
- Signal poles from this standard are not allowed over highways I-5, I-84, I-205, I-405, US 26 (Sunset Hwy.) between milepoints 64.3 – 73.0, I–105, and I–82. Signal poles on these highways require a Fatigue Category I.
- 5. Pole and arm shafts may be either round, hexdecagonal, dodecagonal, or octagonal but shapes shall not be mixed on a project. Dimensional tolerances of ASTM A595 shall apply to all tapered steel tubing members. Additionally, the diameter of round tapered steel tubing members or the dimensions across the flats of octagonal tapered steel tubing members shall not vary more than 2 percent from specified dimension. Two ply and fluted poles or arms are not permitted.
- Pole taper shall be equal to .0117 in/in. 6.

MG51

- Anchor rods shall conform to ASTM Specification F1554 Gr. 55, Supplementary Requirement "S2" that include grade and manufacturer's identification.
- *High strength bolts shall conform to ASTM F3125 Grade A325* 8. Type 1.
- 9. Nuts for high strength bolts shall be heavy hex and conform to ASTM A563 Grade DH with supplementary requirements "S1" and "S2".
- 10. Hardened steel washers shall conform to ASTM F436 Type 1.
- 11. Direct Tension Indicators (DTI) shall be the compressible washer type, mechanically galvanized, conforming to ASTM F959.
- 12. Steel sheet for poles and arm shall conform ASTM A595, Grades A or B, ASTM A572 Gr. 50, or approved equal. All other steel sheet and plate shall conform to AASHTO specification M223 (ASTM A572), or approved equal. Supplement S18 of ASTM A6 regarding maximum tensile strength shall apply.
- 13. All structural steel including fasteners shall be hot-dip galvanized after fabrication unless otherwise noted.
- 14. Galvanize-Control Silicon, typical. Silicon content of the base metal shall be in the range of 0 to 0.04 percent or 0.15 to 0.25 percent.
- 15. Footing concrete shall be Commercial Grade Concrete (fc=3000 psi) per Specification Section 440. Grout in grout pad shall be non-shrink high early strength grout (non-ferrous) with a minimum strength of 5000 psi.
- *16. Reinforcing steel shall conform to AASHTO M31, Grade 60* (ASTM A615 or A706). A minimum lap splice length of 32 bar diameters shall be used unless shown otherwise.

- 17. Computed deflection of these poles at full design loading shall be limited to 5 percent of the pole length. Computed dead load deflection of the poles shall be limited to 1 percent of the pole length. Pole shall be raked to offset the computed dead load deflection. Computed deflection (ignoring pole bending and/or rotation) of signal arms shall not exceed that listed in the Signal Arm Deflection Table on TM650. Additionally, the amplitude (maximum up to maximum down as measured at the tip of the arm) of wind induced vertical oscillations shall not exceed 1.5 percent of the signal arm length. Luminaire arms and pole extensions to support luminaire arms shall meet requirements of standard drawing TM629.
- 18. Hubs for cabinets and/or other appurtenances shall be welded into the pole prior to galvanizing. Poles may be tapped for up to 1" galvanized bolts after pole has been galvanized.
- 19. Longitudinal seam welds within 6 inches of a circumferential weld shall be complete penetration welds. Weld inspection shall be in accordance with AWS D1.1 and the special provisions. Inspect seam welds using cyclically loaded criteria. Hubs shall be 3000# threaded forged carbon steel flat weld hubs by Anvil Products Inc., Phoenix Forging Co., Bonney Forge & Tool Works or approved equal.
- 20. Grounding terminal shall be $\frac{1}{2}$ " UNC x $1\frac{1}{2}$ " Type 308, 309 or 310 threaded stainless steel weld studs.
- 21. Assemble support, tighten anchor bolts, tighten HS thru bolts and tighten HS bolts in tapped holes according to 00962.46(j)(2).
- 22. Round and smooth all edges along electrical way.
- 23. The minimum arm flange thickness shall be equal to the value where prying action is not included in the bolt calculation.

Standard Maximum Base Reactions (Unfactored)									
Signal	Signal		Wind Lo	oad Case II	/	Cor	ntrolling Fa	atigue	
Pole	Arm	Axial	Shear	Moment	Torque	Shear	Moment	Torque	
Туре	Lengths	(Kips)	(Kips)	(Kip-ft)	(Kip-ft)	(Kips)	Kip-ft)	(Kip-ft)	
SM1	15'	2.10	5.15	80.39	16.95	0.68	10.39	2.13	
SM2	20', 25'	2.66	6.23	105.41	42.54	0.82	13.35	<i>5.37</i>	
SM3	30', 35'	3.49	7.77	138.43	82.87	1.00	17.10	10.31	
SM4	40', 45'	4.51	9.00	173.46	132.72	1.16	20.54	16.50	
SM5	50', 55'	5.69	9.23	190.91	181.60	1.18	21.62	22.55	
SM1L	15'	2.96	6.09	113.28	23.22	0.79	14.08	2.84	
SM2L	20', 25'	3.69	7.23	139.41	48.81	0.94	17.17	6.08	
SM3L	30', 35'	4.39	8.80	176.51	87.88	1.14	21.43	11.02	
SM4L	40', 45'	5.94	10.14	215.11	136.97	1.31	25.27	17.21	
SM5L	50', 55'	7.34	10.56	241.17	187.96	1.34	26.49	23.26	

Standard Maximum Mast Arm Reactions								
Signal	Signal	Win	d Load C	ase II	Controlling Fatigue			
Pole	Arm	Axial	Shear	Moment	Shear	Moment		
Туре	Lengths	(Kips)	(Kips)	(Kip-ft)	(Kips)	(Kip-ft)		
SM1, SM1L	15'	0.06	1.98	18.44	0.23	2.18		
SM2, SM2L	20', 25'	0.10	3.14	46.20	0.37	5.48		
SM3, SM3L	30', 35'	0.15	4.51	89.42	0.53	10.51		
SM4, SM4L	40', 45'	0.23	5.91	146.67	0.67	16.82		
SM5, SM5L	50', 55'	0.34	6.78	211.94	0.70	22.99		

Luminaire Arm Reactions									
Arm	Win	d Load C	ase II	Controlling Fatigue					
Lenaths	Axial	Shear	Moment	Shear	Moment				
Lengths	(Kips)	(Kips)	(Kip-ft)	(Kips)	(Kip-ft)				
6'	0.03	0.31	1.49	0.03	0.15				
10'	0.06	0.38	2.85	0.04	0.29				
15'	0.08	0.47	4.96	0.05	0.51				
20'	0.05	0.55	7.24	0.06	0.74				

Accompanied by drawings TM650, TM652, TM653, TM654

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	CALC. BOOK NC) 5301	SDR DATE_ 06-JAN-202	²₃_ TM651					

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EXTERIOR PAD LOCK DETAILS No scale

SECTION B-B

No scale

Accompanied by dwg

The selection a Standard Drawi designed in acc generally accept principles and sole responsibi and should not first consulting Professional En

No scale (ODOT Supplied Post Construction)

See ODOT Lock Details

Exterior Pad Lock Connection

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		C 7	ANDARD SICN	AI ARMI	0405				DEELECTIONS												
		Signal	Signa	<u> /</u>		Sian			Estimated												
	Signal Pole	Arm	4L 2	5	* 51	52 ¹	* Horz.	- DS Max.	"defl"						V.	ERTICAL PO	<u>ST LOADS</u>	A	Araz	A	M/-1-1-1
		Length	Qty. Qty	/. Qt	v. Qty.	Qty.	Blank	101 52	End of Arm					Description	махітит Centerline	Height	Width Dep	th Front	Side	Area Bottom	weight 0" lce
	SM6L	60', 65'	1 2	1	4	1	58'-0"	21'-1"	2'-9"					2 Rad Buch Buttor-	Elevation	(Each) ($7^{3}/.''$	$\frac{Each}{5''} = \frac{2^{3/2}}{2^{3/2}}$	(sq. ft) (sq. ft)	(sq. ft)	(sq. ft)	(lbs)
							+	+						<i>2-rea. rusn Buttons</i> <i>Controller Cabinet</i>	3'-0" 5'-9"	46"	<i>3⁴</i> <i>24</i> " <i>22</i>	0.27	7.03	U.12 3.67	300
	SM7L	70', 75'	1 2	1	4	/ /	68'-0"	21'-1"	3'-9"					2-Pedestrian Signals	<u>8'</u> -3 ¹ / ₂ "	18 ³ /4"	19" 19	2.47	2.47	2.51	25.0
	L		I				•	•	-					Terminal Cabinet	10'-9"	181/8"	6 ³ / ₄ " 8 ³ / ₈	" 0.85	1.05	0.39	25.0
	* – Load locatio	n is the clo	sest sian or sid	nal of th	at type									Guide Sign (S3)	15'-0"	72"	120" 8 ³ / ₈	60.0	1.00	1.67	395
1	to the vertica	l post.		,										Photoelectric Cell	38'-4"	21/4"	<i>3\4" 3\4</i>	0.05	0.05	0.07	5.0
	1. Camera mo	unted on 6	ft arm placed .	at anv										1. Physical fit of the load	ing must be ve	erified.					
	location on	signal arm																			
77	2. Fire Pre-Em	ption may	be placed at an	y locatio	n																
1	3. Modification	ns to the In	ading shown re	quire																	
5	analysis to	verify the s	tructural adequ	acy																	
5	of the pole.	of the land	ing must have	rified											ର						
2	4. Physical fit 5. 60' and 70'	mast arm i	ing must be ver engths use the	same								<u>("(</u>			ğ	+	[Ŧ	
	design as ti	he longer 6	5' and 75' leng	ths								<u>17/6</u>			읽			+		т	
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D										Design	Luminaire arm	raintight pole cap.		<u>S</u>	<u>IGNAL F</u>	<u>'ULE A</u>	PPURT	<u>-NANC</u>	<u>.e iy</u> f	<u>45</u>	
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		6" —	s' = 0" min. (т <i>үр.)</i>				<i>ci:</i>			Luminaire Arm Extension \neg	See Mastarm									
			min.				Γ	Slip Joint See Dwa TM	657		\backslash	Connection Details			AP	PURTENANC	CE LOADS				
			(, , yp.	mera		-						drawing TM657			Туре Т	Area A Front C	rea Area	n Weight			
				/	2 1/2' x 3' Horizontal	Permanent Sign Blank					4 Bolt Pole Connection – See TM657	See Handhole	oles			(sq. ft) (se	q. ft) (sq. ft) (lbs)			
	Undeflected Arm Elev	_	- _ _	/	mounted at end of arm	o'from					\backslash	Details on	d 7#		<u>4L</u>	12.4 6	5.61 3.64	145			
	LICY.	defi				_ 					4° for 60' through 75' —		SM#		4	<u>8.67</u> 6	2.49 1.95	97.0			
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1	nstall removable — steel or aluminum	nmi'		e 2_/		ted L	Illuminated	'\ `	\backslash	- iypes			Hei <u>c</u> 7'-0	H Sin	orz. Blank	1.72 2	<u>.38 7.50</u> 255 n	45.0			
	raintight cap at the	nax e		 nated	iypest	/	iypesi	\	└─ Illuminated			Mir.	1 (4(Lu	m. Camera	0.65 1	.42 0	25			
1	end of mast arm	-0" /	Type S	57	Type 2-	/		L TVn	Iype S1 e 5, 4L, 4R.		Λ	rt =	entre		L		I				
		-19'- :leai	└─ <i>Type 4L, 4</i>	4, or 2				2, 5	1, 52		Type S3, (3)–52, -1		, G CE								
		im (<i>ا د-(+)</i> ان	<u>t He</u>	niar								
		iimu. ertii									Recessed Terminal ——	<u>nen</u>	umi								
		vim V									Cabinet, see TM654		to 11								
		<i></i>			_	Roadway	surface				See Handhole details on	Groundina	ase i	Accompanied by dwas. Th	4654. TM656	, ТМ657. Т	M658. TM62	8			
		18				(2% used	for Std. De	sign estimate	e)		drawing TM658 (Orientation	terminal	آ ا		,	,, , ,	All mat	erials shall be	in accorda	ance with	
												<u>₩</u> !			C		the curre	nt Oregon Sta	andard Spe	cification	5.
											See dwg. TM657 for base	See project plans for		The selection and use	ot this		OREGO	STAND	ARD DR	AWING	S
											plate details.	top of footing elevatio	on.	Standard Drawing, whi	ile	TRA	FFIC SI	GNAL (<u>;0' T</u>	IROU	GH 7
														designed in accordanc	e with	1	MAS		SUPP	ORT	5
												Install footing according to a start in the start is a start in the start	TM628.	generally accepted eng	gineering		GEN	ERAL [JETAL	LS &	
												Use the reinforcement steel, diameter. and number of C	, shaft SL	principles and practice	es, is the		DE	SIGNC	RITE	RIA	
									I Y PICAL	PULE ELEVATION	N	tubes according to the mono	otube	sole responsibility of t	he user			202			
										No Scale		cantilever design number 6. the reactions shown on TM6	. Use 656 and	and chauld not be use	d without	07-2020	REPLACED HUB W	TH RECESSED TEP	RMINAL CABIN	ET, ADDED	
												Project Plans footing length.		and should hot be use		07-2022	BY DRAWING THE	54, AND CHANGE	D SIGN 7' DIS	TANCE TO 6	
												_		TITST CONSULTING A Regis	stered	07-2022	DRAWING TM628	TABLE DETAIL RE	QUIREMENTS	CLARIFIED	
1														Professional Engineer.							

GENERAL NOTES

-2023

- 1. Signal supports shall be designed in accordance with the AASHTO Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals LRFD 1st edition with 2017 and 2018 interim revisions.
- 2. All traffic signal supports shall conform to the design criteria and details shown on these drawings except as approved by the Engineer.
- 3. The design basic wind speed (3 second gust) shall be 145 mph, gust factor G = 1.14, 50 year recurrence, Fatigue Category I, no galloping, and truck speed = 65 mph.
- The design service basic wind speed (3 second gust) shall be 4. 91 mph.
- Signal poles from this standard are not allowed over highways 1-5, 1-84, 1-205, 1-405, US 26 (Sunset Hwy) between milepoints 64.3 – 73.0, I–105, and I–82.
- Pole and arm shafts must be round. Dimensional tolerances of 6. ASTM A595 shall apply to all tapered steel tubing members. Additionally, the diameter of round tapered steel tubing members shall not vary more than 2 percent from specified dimension. Two plv and fluted poles or arms are not permitted.
- Pole taper shall be equal to .0117 in/in.
- Anchor rods shall conform to ASTM Specification F1554 Gr. 55, Supplemenetary Requirement "S2" that include grade and manufacturer's identification.
- *High strength bolts shall conform to ASTM F3125 Grade A325* 9. Type 1.
- 10. Nuts for high strength bolts shall be heavy hex and conform to ASTM A563 Grade DH with supplmentary requirements "S1" and "S2".
- 11. Hardened steel washers shall conform to ASTM F436 Type 1.
- 12. Direct Tension Indicators (DTI) shall be the compressible-washer type, mechanically galvanized, conforming to ASTM F959.
- 13. Steel sheet for poles and arm shall conform ASTM A595, Grades A or B, ASTM A572 Gr. 50, or approved equal. All other steel sheet and plate shall conform to AASHTO specification M223 (ASTM A572), or approved equal. Supplement S18 of ASTM A6 regarding maximum tensile strength shall apply.
- 14. All structural steel including fasteners shall be hot-dip galvanized after fabrication unless otherwise noted.
- 15. Galvanize-Control Silicon, typical. Silicon content of the base metal shall be in the range of 0 to 0.06 percent or 0.13 percent to 0.25 percent.
- 16. Footing concrete shall be according to TM628.
- 17. Reinforcing steel shall conform to AASHTO M31. Grade 60 (ASTM A615 or A706). A minimum lap splice length of 32 bar diameters shall be used unless shown otherwise.

- 18. Computed deflection of these poles at full design loading shall be limited to 5 percent of the pole length. Computed dead load deflection of the poles shall be limited to 1 percent of the pole length. Rake pole, apply mast arm and appurtenance loads, and verify final pole position is plumb.
- 19. Luminaire arms and pole extensions to support luminaire arms shall meet requirements of drawing TM629.
- 20. Hubs for cabinets and/or other appurtenances shall be welded into the pole prior to galvanizing. Poles may be tapped for up to 1" galvanized bolts after pole has been galvanized.
- 21. Longitudinal seam welds within 6 inches of a cirumferential weld shall be complete penetration welds. Weld inspection shall be in accordance with AWS D1.1 and the special provisions. Inspect seam welds using cyclically loaded criteria. Hubs shall be 3000# threaded forged carbon steel flat weld hubs by Anvil Products Inc., Phoenix Forging Co., Bonney Forge & Tool Works or approved equal.
- 22. Grounding terminal shall be $\frac{1}{2}$ " UNC x $1\frac{1}{2}$ " Type 308, 309 or 310 threaded stainless steel weld studs.
- 23. Assemble support, tighten anchor bolts, tighten HS thru bolts, and tighten HS bolts in tapped holes according to 00962.46(j)(2).
- 24. Round and smooth all edges along electrical way.

Note:

The base plate reactions shown in the table are worst case Extreme I and Service I loads. Engineer of Record to specify shaft depth and confirm shaft design for local soil conditions based on a site specific geotechnical study and loads shown in table. If shaft size or reinforcement shown in the table on TM628 for the required design number are not adequate for local soil conditions, Engineer of Record must adjust the shaft design accordingly.

The selection and use of this Standard Drawing, while designed in accordance with generally accepted engineering principles and practices, is the sole responsibility of the user and should not be used without first consulting a Registered Professional Engineer.

2	late	Reaction At Base Plate (Service)										
	Torsion (ft–lb)	Axial (lb)	Shear (lb)	Moment (ft-lb)	Torsion (ft–lb)							
	322,000	6,520	5,200	163,000	127,000							
	385,500	8,080	5,190	212,720	153,000							

All materials shall be in accordance with

PLAN – BASE PLATE No Scale

2" Tvt

8" max. dia. hole thru plates

2-¾" dia. zinc drain holes thru base plate.

The selection and use of this Standard Drawing, while designed in accordance with sole responsibility of the user first consulting a Registered Professional Engineer.

2020 0-IUL Backup weld ring-

(Round edges for

wire protection)

TM657

dgn TM658.

Effective Date: December 1, 2023 - May 31, 2024

	ASTM A615 Grade 60, ASTM A82 or ASTM A496 may be used if copies of the chemical composition analysis are submitted and approved as weldable by the Engineer.
10", 135° hooks, typ. May be field bent	Anchor spirals at each end or discontinuity with one extra turn and a splice to itself as shown. Where permitted on plans, provide closed hoops conforming to the requirements of this detail.
,	Securely tie CSL tubes to reinforcement.
	Use temporary casing as required. Permanent casing not permitted.
f pitch when nical splice is o anchor unit	Cap concrete shall be Class 3600 – $\frac{3}{4}$ " commercial grade, classified as a structural item. Remainder of shaft shall be Class 4000 – $\frac{3}{8}$ " without air entrainment and with 8 $\frac{1}{2}$ " ± 1 $\frac{1}{2}$ " slump.
	Contractor shall field verify elevations prior to installation.
- - 	The transition between the 3 $1/2$ " to 6" pitches may use two separate spiral cages with $1\frac{1}{2}$ horizontal turns at the start and end of each cage and the lapped splice details between the cages.

GENERAL NOTES:

Use ASTM A706 for all welded splices, except

Note:

The base plate reactions shown in the table are worst case Extreme I and Service I loads. Engineer of Record to specify shaft depth and confirm shaft design for local soil conditions based on a site specific geotechnical study and loads shown in table. If shaft size or reinforcement shown in table are not adequate for local soil conditions, Engineer of Record must adjust the shaft design accordingly.

The shafts designs shown in table were based on an analysis to encompass worst case soil conditions by applying Extreme I loads to the top of shaft and analyzing below ground shaft forces using Brom's method for two different soil types. The assumed cohesive soil minimum undrained shear strength, c, is 600 psf. The assumed non-cohesive soil friction angle is 25 degrees and bulk weight is 100 pcf.

Accompanied by dwgs. TM621, TM622, TM623, TM624, TM625, TM626, TM627

nd use of this
ing, while
ordance with
oted engineering
practices, is the
lity of the user
be used without
a Registered
gineer.

OREGON STANDARD DRAWINGS											
STD. MONOTUBE SIGN/VMS SUPPORT DRILLED SHAFT DETAILS											
		2024									
DATE	REVISI	ON DESCRIPTION									
07-2020	ADDED "MONOTUBE" TO T	HE DESIGN NUMBER COLUMNS									
01-2021	CHANGED CONDUIT NOTE										
01-2022	SLUMP WAS 8" +/- 1/2"										
07-2022	ADDED SPIRAL TIE NOTES /	AND CONDUIT BASE PLATE NOT	E								
CALC. BOOK NO	CALC. 6921-6930, 6969- BOOK NO. 6972, 6974 DATE_ 08-JUL-2022 _ TM628										

All materials shall be in accordance with

the current Oregon Standard Specifications

EXAMPLE 1

LEGEND

CONTROLLERS

Install model ATC controller. (Agency furnished)

<u>CABINE</u>TS

 $\left(\frac{C}{ATC}\right)$

Install a model 332S cabinet & control equipment (3323) with riser frame, orient louvered door as shown Install base mounted service cabinet, 120/240 volt metered, BMCL for signal and signal pole mounted illumination systems

(RTC) Install recessed terminal cabinet

POLES

 \overline{L}

 (\overrightarrow{PP})

 $\begin{pmatrix} LA \\ L \end{pmatrix}$

 $\begin{pmatrix} EX \\ 1 \end{pmatrix}$

 $\frac{SM}{T}$ Install (T=type) standard traffic signal mast arm pole (See, "Pole Entrance Chart") MA Install (L=length) foot traffic signal mast arm

Install pedestal with frangible base on (N=number)foundation.See TM457 for details.

Install (L=length) foot luminaire arm

Retain and protect existing power pole (Power source)

SIGNALS

 $\binom{V}{Ph}$ Install phase (Ph-phase) vehicle signal

 $\frac{P/B}{Ph}$ Install phase (Ph=phase) pedestrian signal with clamshell mount and pushbutton with mount

<u>signs</u>

 $\begin{pmatrix} AL \\ 4L \end{pmatrix}$ Install aluminum (30"x36") left and through arrow sign (R3-6L), ASTM type IX sheeting $\begin{pmatrix} AL \\ 5R \end{pmatrix}$ Install aluminum (30"x36") right arrow "ONLY" sign (R3-5R), ASTM type IX sheeting Install street name sign (See signing plans for details on sign and attachment type) $\langle \star
angle$ JUNCTION BOXES JB 2 Install 22"x12"x12" (min. dimension) precast concrete junction box Install tandem 30"x17"x12" (min.dimension) $\binom{JB}{3/T}$ precast concrete junction boxes (See TM472 for details) $\begin{pmatrix} JB \\ DP \end{pmatrix}$

Junction box (See Detector Plan)

<u>WIRES</u>

NG Install (N=number) No. (G=AWG wire size) type THWN wires

NG Install (N=number) No. (G=AWG wire size) type XHHW wires

 $\left(\frac{X-N}{G}\right)$ Install (X=number of cables) control cable(s) with (N=number) (G= AWG wire size) AWG conductors

LEGEND CONTINUED

CONDUITS

((cs

- (S Install (S=size) inch electrical conduit
 - Install 2" conduit stub (For future use-cap ends)
- ((DC) Detector conduit (See Detector Plan)
- (w)Install conduit and wire as required by power company

LUMINAIRES

- Install light emitting diode luminaire,
- (LED)(See special provisions). Bond luminaire
- to pole grounding terminal
- $\frac{PE}{1}$ Install photocontrol electronic relay on pole. as per Std. Drg. No. TM450

FIRE PREEMPTION

- Install channel(Ch=channel), (N=number) barrel
- (FN) Ch fire preemption detector unit
- \overline{FF} Ch Install channel (Ch=channel) fire preemption detector feeder cable

Detection Camera, See Detection Plans.

MISCELLANEOUS

Install removable bollard

(BD CAM DP

SIGNAL HEAD TYPES 2 = R:Y:G 3LCF = RA:YA/FYA:GA

HWY: 081

NOTE:

Traffic Section Approval

LEGEND

PACIFIC HWY EAST AT YOUNG ST.

OR99E.M.P. 32.87 WOODBURN

OR99E: YOUNG ST. SAFETY (WOODBURN) SEC. PACIFIC HIGHWAY EAST

MARION COUNTY

Designer: ARLO BONES Drafter: ARLO BONES

Review: VERN GEORGE

Checker: N/A

LEGEND

SHEET NO. M-01

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FINAL ELECTRONIC DOCUMENT AVAILABLE UPON REQUEST

POLE ENTRANCE CHART

		See TM650 thru TM653	EQUIF	PMEN	IT ON	I POL	.E	EQUIPMENT ON MAST ARM (Length in Feet and Equipment Type)						NDATION RMATION itd. Drg. TM653)					VIDEO DETECTIOI EQUIPMEN				
POLE NO.	DWG. NO.	TYPE	PED. SIGNAL & PUSHBUTTON DEG.	TERM. CABINET DEG.	SIGN DEG.	TRAFFIC SIGNAL DEG.	: Photo Electric Cell	ARM LENGTH	D 1	D 2	D 3	D 4	D 5	D 6	D 7	D 8	Foundation Number	REQUIRED FOUNDATION DEPTH	ARM LENGTH	ARM DEG.	MOUNTING HEIGHT	TYPE	LUMINAIRE ARM MOUNT
1	18251	SM-3		180				30	0.5 V2	<u>1.5</u> F	 CAM	<u>11.0</u> V2					3	18' - 0"					
2	18251	PP-2	180																				
3	18251	PP-2	270																				
4	18251	SM-5L		180				50	0.5 F	6.0 V3LCF	<u>18.0</u> V2	<u>30.0</u> V2	<u>38.0</u> SNS				6	20' - 0"	15.0	0	35.0	LED	
5	18251	PP-2	90																				
6	18251	PP-2	180																				
7	18251	SM-3L		180				35	<u>0.5</u> F	<u> 1.0 </u> SA	4.0 V2	<u> 16.0 </u> V2	<u>20.0</u> SA				4	18' - 0"	15.0	0	35.0	LED	CAM
8	18251	PP-2	180																				
9	18251	PP-2	270																				
10	18251	PP-2	90																				
11	18251	SM-5L		180			180	50	0.5 V3LCF	<u>3.0</u> F	<u>12.5</u> V2	24.0 V2	<u>37.0</u> SNS				6	20' - 0"	15.0	0	35.0	LED	
12	18251	PP-2	0																				

<u>NOT E</u>

Refer To Geotechnical Memo Dated August 5,20XX For Subsurface Information

BRACKET MOUNT

V2 = Traffic Signal Type 2.Vehicle Signal Bracket Mount V3LCF = Traffic Signal Type 3LCF.Vehicle Signal Bracket Mount SA = Sign, 30" x 36" Aluminum w/Sign Bracket Mount

MISC.ITEMS

F = Fire Preemtion CAM = Thermal Detection Camera SNS = Street Name Sign

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WIRING DETAILS PACIFIC HWY EAST AT YOUNG ST. OR99E.M.P. 32.87 WOODBURN

*Control cable conductor use for vehicle phases and pedestrian phases as per TM470

			PEDESTRIAN PHASES	VEHICLE PHASES
7 CONDUCTOR	CONTRO	L CABLE	1	1
CONDUCTOR NUMBER	BASE COLOR	FIRST TRACER	Pedestrian Phase	Vehicle Phase
1	WHITE		NEUTRAL	NEUTRAL
2	BLACK		WALK	YELLOW
3	RED		DONT WALK	RED
4	ORANGE	_	P.B. COMMON	SPARE
5	GREEN	—	PUSHBUTTON	GREEN
6	BLUE		SPARE	SPARE
7	WHITE	BLACK	SPARE	SPARE

Note: Control cable for pedestrian phases are terminated in mast arm pole terminal cabinet as per TM470

ILLUMINATION AND PHOTO CELL

6 #10 XHHW wires for Illumination: (2 wires from BMCL to Pole #4)

(2 wires from BMCL to Pole #7)

(2 wires from BMCL to Pole #11)

3 #12 THWN wires for photo cell from BMCL to Pole #11

26 POWER TO CONTROLLER FROM BMCL 2 #6 XHHW wires from BMCL to 332S Cabinet

<u>Y6-7 YFF YFF YFF YFF</u> 3 (DC 14 Å A

SIGNALS

4 Fire Cables From 332s to Each Detector Unit 14 control cables

T	control cables.								
	PHASE*	1 CABLE TO							
	Ph. 1	pole #4							
	Ph. 2	pole #11							
	Ph. 4	pole #1							
	Ph. 5	pole #11							
	Ph. 6	pole #4							
	Ph. 8	pole #7							
	Ped 2	pole #2							
		pole #12							
	Ped 4	pole #3							
		pole #5							
	Ped 6	pole #6							
		pole #8							
	Ped 8	pole #9							
		pole #10							

EXAMPLE 2

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		(TRAFF TEMP ROGUE V.	IC CON ORARY ALLEY OR99,	VTROL STAG SIGNAL PL HWY.AT BO M.P.11.25	E 3) AN DLZ RD.
	<u>ଁ</u> ଷ୍	\	(Рн	OENIXI	
		² ,			
		$\overline{\}$			
 	Note:				
	k ★= Ir Payment	nstall wood pedestri to relocate is incid	ian post as dental to ten	needed for construction nporary traffic signal	on. installation.
	Note: Leave sl moving l Enginee	'ack in Temporary , heads as needed to r to verify propose	signal head o accommoda ed staging lo	wiring to accommodate the for various staging ayouts,other than the	e for layouts. ones shown.
	FIRF				
)	<u>FN</u> <u>Ch</u>	Install,(N=numbe fire preemption	<u>!</u> *r) direction detector ur	n, channel (Ch=channel nit))
ונ	(FF) Ch WIRE	Install channel (Co detector feeder c c	h=channel) cable	fire preemption	
ent /n	38	<u> </u>	1 3/8″ messi	enger and $1/_4$ " tethe	er cables
	(42)	Install galv. steel	ı 1/4″ messi	enger cable	
l pole	N/G	Install (N=number	⁻) No.(G=AW	G wire size)type TF	WN wires
")	NG	Install (N=number	.) No. (G=AW	G wire size) XHHW	wires
	$\frac{X-N}{G}$	Install(X=number No.(G=AWG wire	of cables) size) condu) control cables with uctors	(N=number)
	COND	<u>UITS</u>			
	W	Install conduit as Power company t	required in the install wi	by power company. ire	
signal and	Ś	Install (S=size) in	nch electric	cal conduit	
		•		-	
signal					
'an h mount		<u>SIGNAL HE</u>	AD TYPE	<u>ES</u>	
· · · · -		2 = R:Y:G 31 = RA:YA;	· 64		
		3LCF = RA:	YA/FYA:G	A	
			Traffic	* *	
_			Hame	Section Approva	DEPARTAR
PROFFCA		OREGON	J DEPA	RTMENT	
INEEN		OF TRA	NSPOk	TATION	THANSPORTATION
(XXX) [7]					
NSTRUCTIO	JN				
EGON X	7	FF0-I-5: FE	RN VALLE PACIFIC	Y INTERCHAINGE, U HIGHWAY	ND 2
x. 1, 4		signer ARLO BONES	JACKSU	ON COUNTY Reviewer: VERN GEORGE	
VXXXXXXXX	Dra	after: ARLO BONES		Checker: N/A	
12-31-20XX		TEMPOR		IAL PLAN	SHEET/DWG. NO.
			(STAGE 3)	M-10

