Standard Distribution
Date of Issue: January 2023

William Lee Woods, PE Senior Standards Engineer

This is the January 2023 release of the 2021 Oregon Standard Drawings.

For ODOT Projects the details in the standard drawings will be effective on the **June 1, 2023** bid opening where these drawings are called for in the project plans.

These drawings are for use with projects using the **2021 Oregon Standard Specifications**.

You will notice an "effective date" on the lower right bottom of each Standard Drawing. The bid opening date of a project should be in the effective date window of the drawings. This will ensure the correct drawings are being used on the projects.

Electronic PDF files with the effective date for each drawing are on the Web site at:

http://www.oregon.gov/ODOT/Engineering/Pages/Standards.aspx

The Standard Drawing Baseline Reports for the drawings contain useful information for the designer as well as updates that occur on the drawing. The link to the report is the title of the specific drawing on the webpage.

These Standard Drawings are the ones that have updates:

Drawing Number	Comment
RD322	
RD324	
RD325	
RD326	
RD327	
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RD339	
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RD352	
RD354	
RD406	
RD435	
RD502	Title Change
RD515	
RD516	

Drawing Number	Comment
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TM521	
TM651	
TM656	
TM855	New Drawing

DRAWING	REVISION	DRAWING	REVISION	DRAWING	REVISION
NUMBER	DATE	NUMBER	DATE	NUMBER	DATE

RD100		RD344		RD420	07/2021
RD101		RD345	01/2023	RD421	
RD110		RD346		RD435	01/2023
RD115		RD348		RD436	
RD120		RD350		RD437	
RD130		RD352	01/2023	RD438	
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RD286		RD373		RD503	01/2023
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RD319		RD393	07/2021	RD546	01/2023
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RD326	01/2023	RD403	07/2021	RD580	
RD327	01/2023	RD404	07/2021	RD581	
RD328	01/2023	RD405	04/0000	RD590	07/0000
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RD332		RD407	07/2021	RD596	07/2022
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RD335		RD409		RD610	07/0004
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RD339	01/2023	RD415		RD701	04/0055
RD340		RD416		RD702	01/2022
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RD343		RD419	07/2021	RD706	

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RD707		RD1005	01/2021	BR241	07/2022
RD710	01/2022	RD1006	01/2021	BR242	
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RD771		RD1070	01/2021	BR290	01/2023
RD780	07/2021	RD1140	01/2022	BR291	01/2023
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RD782	07/2021	BR115		BR310	01/2022
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RD922	0.72022	BR216	01/2023	BR520	
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RD960	07/2021	BR236	01/2022	BR730	
RD1000	01/2022	BR240	01/2022	BR740	
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BR750 01/2021 TM482 07/2021 TM627 BR751 TM485 07/2021 TM628 BR760 TM492 07/2022 TM629 BR800 TM493 07/2022 TM630	01/2021 07/2022 07/2021
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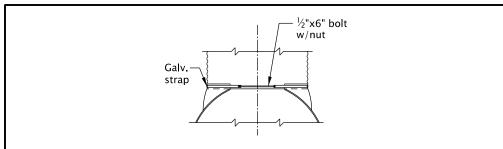
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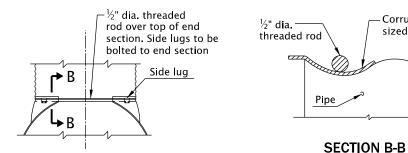
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Stairway, Concrete	RD120	Thrust Blocking, Water Systems	RD250

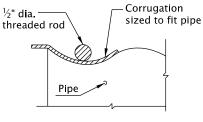
Tire Wash Facility Traffic Island Separator, Concrete	RD1060 RD705 RD706	Trench Backfill Truck Aprons on Roundabouts Trucks and Bus Stop Lanes At Railroad Crossing Truck Scale Pit Truncated Dome	RD300 RD170 RD445 BR182 RD902
Traffic Signals Color Code Chart Controller Cabinet and Foundation Fire Preemption Details Junction Boxes Maintenance Pad Details Mast Arm Pole Details Mounting Details Adjustable Signal Head Spanwire Pedestrian Signal	TM470 TM482 TM456 TM472 RD160 TM450 TM462 TM456 TM457, TM467	Valve Box And Operator Extension Assembly VMS Walk-In Bridge -W-	RD258 TM698
Pole Footing Details Mast Arm Pole Strain Pole Pole Mounts Ramp Meter Details Rectangular Rapid Flashing Beacon Service Cabinet Spanwire Design Strain Pole Details Supports Temporary Trenching & Conduit Installation Vehicle Signal Details Vehicle Signal Pedestal	TM450 TM452 TM680 TM492 TM493 TM485 TM456 TM452 TM650, TM651, TM652, TM653, TM654, TM655, TM656, TM657, TM658 TM453, TM454, TM456 TM471 TM460 TM457	Retaining, Concrete Soundwall, Masonry Pile Footing Spread Footing Soundwall, Precast Water Systems Air Release Assembly, Manual Air Release/Air Vacuum Valve Assembly Hydrant Installation Main Dead-End Blowoff Assembly Root Barrier Thrust Blocking Valve Box And Operator	BR705, BR706, BR707, BR708, BR709 BR750, BR751 BR730 BR740 RD266 RD270 RD254 RD262 RD286 RD250

Extension Assembly	RD258
Water Meter Assembly	RD278
Water Sampling Station	RD282
Water Service Connection	RD274
Wingwalls, Concrete Box Culverts	BR800
Wind Pressure Map	TM671
Wind Speed Map	TM672

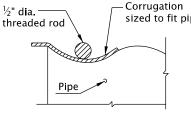


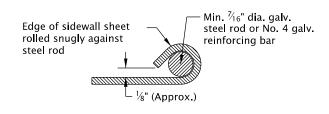
TYPE #1 CONNECTOR DETAIL





TYPE #2 CONNECTOR DETAILS





SECTION A-A

Safety bars (Typ.) *

→ Direction of traffic → →

* Number of bars required will

of the end section.

SIDE VIEW

vary depending on the length

48" max. span for cross drainage structure 24" max. span for parallel drainage structure

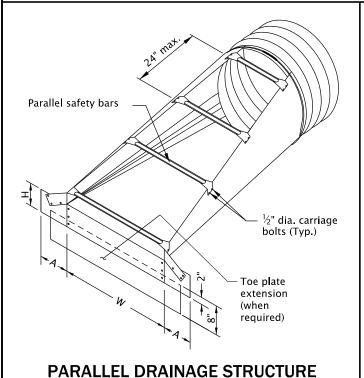
Nom. slope

Reinforced edge full

length of end section (See Section A-A)

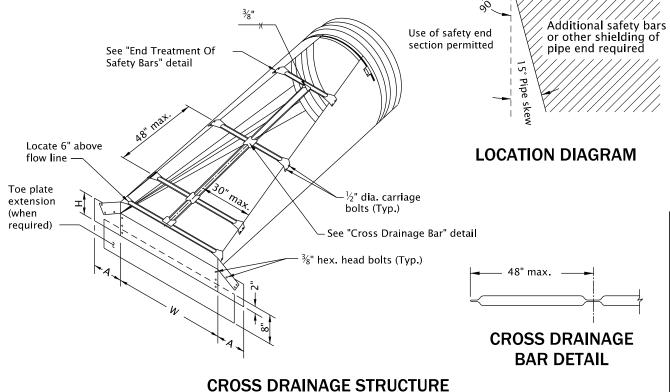
3" galv. pipe: flatten end, then bend outside 4" to match end section sides.

END TREATMENT OF SAFETY BARS DETAIL



Use with single pipe installations 30" dia. or larger

Use with multiple pipe installations 15" dia. or larger



Use with pipe installations 36" dia. and larger

STEEL END SECTIONS FOR ROUND PIPE CULVERT							
PIPE DIAMETER	METAL THICK	DIMENSIONS (Inches)					
(In)	(MIN.)	^	н	w	OVERALL		L
(,	(In/Gage)	Α		VV	WIDTH	Slope=4	Slope=6
15	0.064/16	8	6	21	37	20	30
18	0.064/16	8	6	24	40	32	48
21	0.064/16	8	6	27	43	44	66
24	0.064/16	8	6	30	46	56	84
30	0.109/12	12	9	36	60	80	120
36	0.109/12	12	9	42	66	104	156
42	0.109/12	16	12	48	80	128	192
48	0.109/12	16	12	54	86	152	228
54	0.109/12	16	12	60	92	176	264
60	0.109/12	16	12	66	98	200	300

*** See general note 5

STEEL END SECTIONS FOR ARCH PIPE CULVERT									
PIPE SIZE	E (Inche	s)	METAL			DI	MENSIONS (Inches)	
EQUIVALENT ROUND	***	***	THICK (MIN.)		н	14/	OVERALL	L	
DIAMETER	SPAN	RISE	(In/Gage)	A H W		Slope 1:4	Slope 1:6		
18	21	15	0.064/16	8	6	27	43	20	30
21	24	18	0.064/16	8	6	30	46	32	48
24	28	20	0.064/16	8	6	34	50	40	60
30	35	24	0.109/12	12	9	41	65	56	84
36	42	29	0.109/12	12	9	48	72	76	114
42	49	33	0.109/12	16	12	55	87	92	138
48	57	38	0.109/12	16	12	63	95	112	168
54**	64	43	0.109/12	16	12	70	102	132	198
60 **	71	47	0.109/12	16	12	77	109	148	222
72 **	83	57	0.109/12	16	12	89	121	188	282

** Requires 2 cross drainage bars.

GENERAL NOTES FOR ALL DETAILS ON THIS SHEET:

- . For round pipes with diameters 24" or less use type #1 connector. All arch pipes equivalent round diameter, and round pipes over 24" diameter use Type #2 connector.
- . Toe plate extensions are to be the same min. thkn. as end section. Dimensions shall be overall width less 6" by 8" high.
- . Cross drainage and safety bars shall be 3" dia. Schedule 40 galv. steel pipe.
- 4. Slotted holes for safety bar attachment shall be provided for all end sections.
- 5. Cross-sectional dimensions of attaching pipe may vary with different materials.
- 6. Open ends of pipes normally require a site specific design, and may require special treament (Slope ends, culvert embankment protection, paved end slopes, safety end sections, or other measures). See special details or Standard Drawings as called for on plans.
- 7. See Std. Dwg. RD317 for culvert embankment protection and riprap pads (When reqd.).

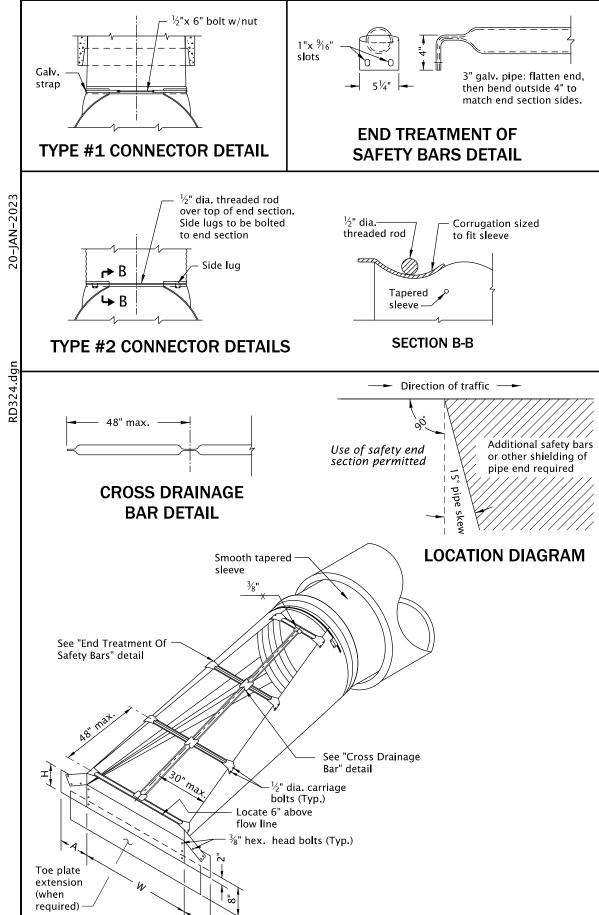
The selection and use of this Standard Drawing, while designed in accordance with generally accepted engineering principles and practices, is the sole responsibility of the user and should not be used without first consulting a Registered Professional Engineer.

All materials shall be in accordance with the current Oregon Standard Specifications. **OREGON STANDARD DRAWINGS**

SAFETY END SECTION FOR METAL PIPE

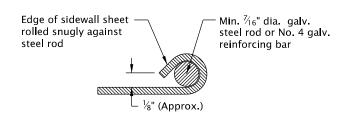
2021

	REVISION DESCRIPTION	DATE
	REVISED DETAILS AND NOTES	01-2023
RD322	D N/A SDR DATE _ 20-JAN-2023 _	CALC.

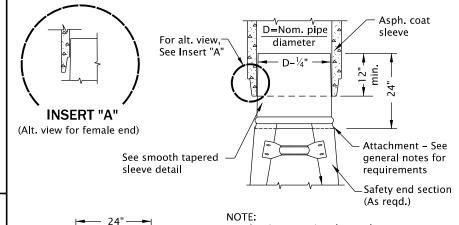


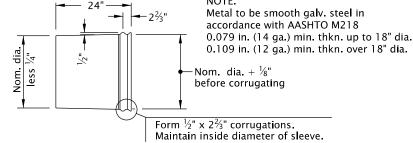
CROSS DRAINAGE STRUCTURE

Use with pipe installations 36" dia. and larger

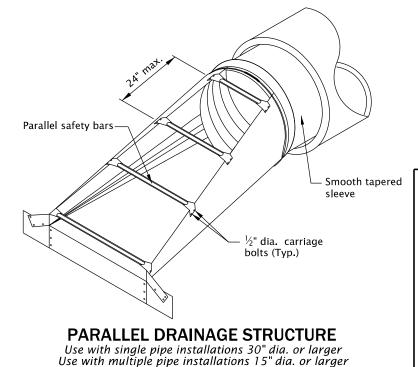


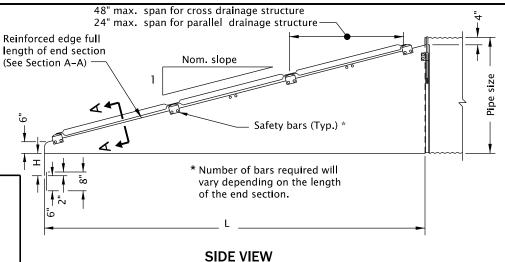
SECTION A-A





SMOOTH TAPERED SLEEVE FOR ATTACHING SAFETY END SECTIONS TO SMOOTH PIPE





STEEL END SECTIONS FOR ROUND PIPE CULVERT							
PIPE	METAL THICK		DIMENSIONS (Inches)				
DIAMETER (In)	(MIN.)	Α	н	\A/	OVERALL		L
(,	(In/Gage)	_ A	"	_ vv	WIDTH	Slope=4	Slope=6
15	0.064/16	8	6	21	37	20	30
18	0.064/16	8	6	24	40	32	48
21	0.064/16	8	6	27	43	44	66
24	0.064/16	8	6	30	46	56	84
30	0.109/12	12	9	36	60	80	120
36	0.109/12	12	9	42	66	104	156
42	0.109/12	16	12	48	80	128	192
48	0.109/12	16	12	54	86	152	228
54	0.109/12	16	12	60	92	176	264
60	0.109/12	16	12	66	98	200	300

GENERAL NOTES FOR ALL DETAILS ON THIS SHEET:

- For round pipes with diameters 24" or less use Type # 1 connector.
 All arch pipes equivalent round diameter and round pipes over 24" diameter use Type # 2 connector.
- 2. Toe plate extensions are to be the same min. thkn. as end section. Dimensions shall be overall width less 6" by 8" high.
- 3. Cross drainage and safety bars shall be 3" dia. Schedule 40 galv. steel pipe.
- 4. Slotted holes for safety bar attachment shall be provided for all end sections.
- 5. Open ends of pipes normally require a site specific design, and may require special treament (Slope ends, culvert embankment protection, paved end slopes, safety end sections, or other measures).
- See special details or Standard Drawings as called for on plans.
- 6. See Std. Dwg. RD317 for culvert embankment protection and riprap pads (When reqd.).

The selection and use of this Standard Drawing, while designed in accordance with generally accepted engineering principles and practices, is the sole responsibility of the user and should not be used without first consulting a Registered Professional Engineer.

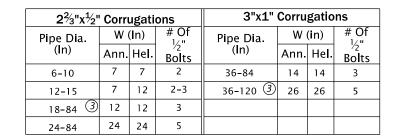
OREGON STANDARD DRAWINGS SAFETTY END SECTION FOR CONCRETE,PVC, HDPE & POLYPROPYLENE PIPE

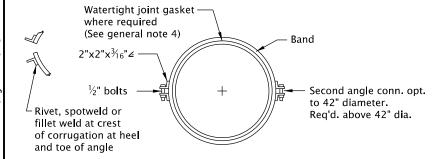
All materials shall be in accordance with

the current Oregon Standard Specifications.

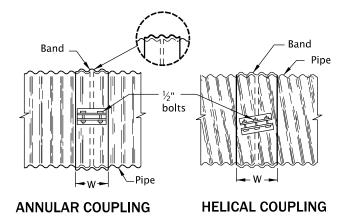
2021

DATE	REVISION DESCRIPTION		
01-2023	REVISED DETAILS AND NOTES		
		•	
CALC. BOOK NO	<u>N/A</u> SDR DATE 20-JAN	²⁰²³ _ RD3	24

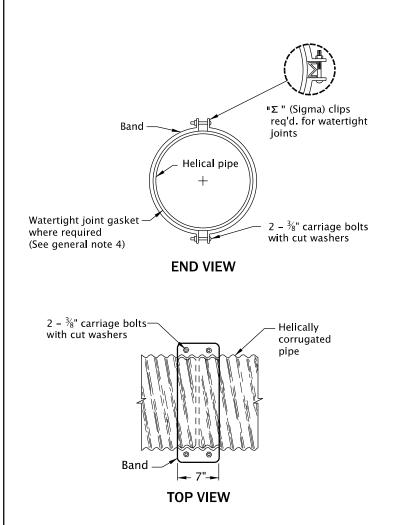




CONNECTION ANGLE DETAIL



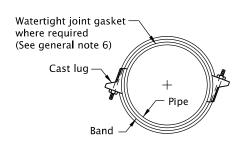
TYPE A



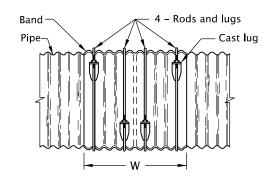
TYPE B

(For 6"-10" dia. pipes)

		Pipe Dia.	Pod Dio	Narrow	Band	Wide	Band
				W	# Of	W	# Of
		(ln)	(ln)	(in)	Rods	(in)	Rods
	2"	12-21	3/8	12	2		
2	3"x½"	24-54 ③	1/2	12	2	24	4
ations	2%	60-84 ③	5/8	12	2	24	4
Orriga	= =	36-54 ③	1/2	14	2	26	4
5	3 X	60-84 ③	5/8	14	2	26	4
	1	84-120	5/8			26	4



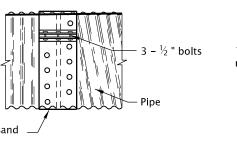
END VIEW

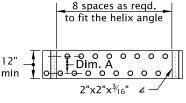


TYPE D

GENERAL NOTES FOR ALL DETAILS ON THIS SHEET:

- 1. All steel coupling bands and connection hardware shall be galvanized or aluminum coated.
- 2. Coupling bands for corrugated iron and steel pipes may be two numerical thicknesses lighter than that used for the pipe but not more than 0.109" nominal thickness nor less than 0.052" nominal thickness. Coupling bands for corrugated aluminum pipe shall be of the same thickness as that used for the pipe.
- 3. Under conditions where concrete pipe may be used as an acceptable alternate, the minimum width coupling band indicated for the band type, corrugation and pipe diameter shown may be used.
- 4. Watertight joints with gaskets are required for irrigation pipes, storm sewers, and other installations when shown on the plans. Gaskets for all coupling bands shall be (butt-cemented or vulcanized) synthetic, closed-cell sponge rubber ³/₈" thick of a width equal to the band width and centering on the joint. For pipes 12" or less in diameter, the gasket thickness may be ³/₁₆".
- 5. Joints for sanitary sewers and siphons are to be tested for water tightness in accordance with the Standard Specifications.
- 6. One or two piece coupling bands are optional for pipe diameters up to and including 42". Coupling bands of two or more pieces are required for pipe diameters over 42".
- 7. To prevent galvanic action when unlike metals are connected, the connecting band shall be used with a full width neoprene gasket or coated with asphalt or other insulating material as approved by the engineer.
- . See Std. Dwgs. RD330 and RD332 for pipe slope anchors, when required.







COUPLING BAND DIMPLE DETAIL

TYPE E
(For 12"-30" dia. pipes)

Dimension A: 7" min. between dimples

as regd. to fit the helix angle.

The selection and use of this
Standard Drawing, while
designed in accordance with
generally accepted engineering
principles and practices, is the
sole responsibility of the user
and should not be used without
first consulting a Registered
Professional Engineer.

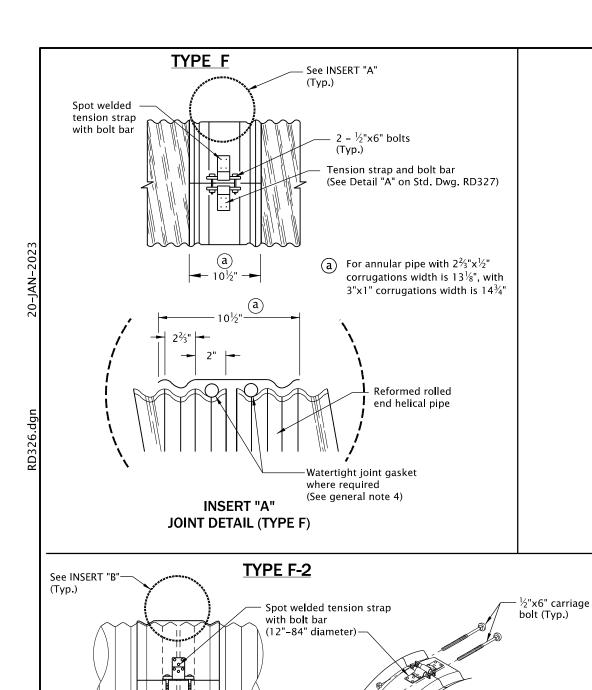
All materials shall be in accordance with the current Oregon Standard Specifications.

OREGON STANDARD DRAWINGS

COUPLING BANDS FOR

CORRUGATED METAL PIPE

	2021		
	REVISION DESCRIPTION	REVISI	DATE
	AND NOTES	REVISED DETAILS AND NO	1-2023
RD325	SDR 20-JAN-2023 _) <mark>N/A</mark>	CALC. OOK NO



SINGLE BAR & STRAP CONNECTOR

DOUBLE BAR & STRAP CONNECTOR

Spot welded tension

(48"-144" diameter) —

strap with bolt bar

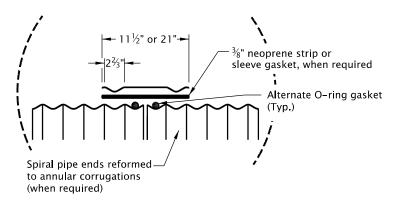
½"x6" carriage

Tension strap and

(See Std. Dwg. RD327)

bolt bar

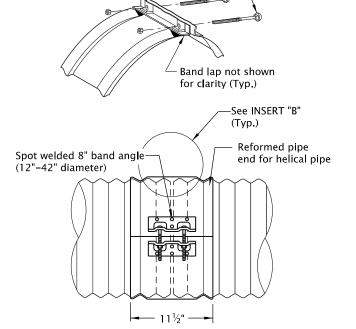
TYPE F-1



INSERT "B" JOINT DETAIL (TYPE F1 & F2)

8" Band angle -

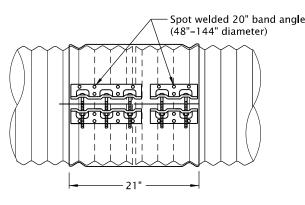
connector



½"x6" carriage

bolt (Typ.)

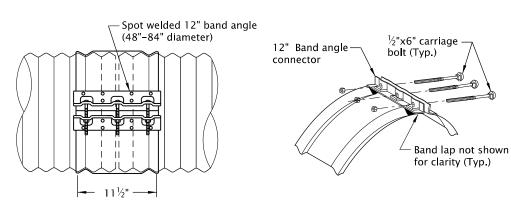
SINGLE 8" BAND ANGLE CONNECTOR



DOUBLE BAND ANGLE CONNECTOR

GENERAL NOTES FOR ALL DETAILS ON THIS SHEET:

- 1. See Std. Dwg. RD325 for general notes, and details not shown.
- 2. Type F coupling bands shown for pipes 15 inch to 72 inch in diameter are typical to arch pipes of equal peripheral measurement.
- 3. Gaskets for the Type F coupling band shall be "O" rings conforming to ASTM C443 and a mastic sealant strip $\frac{1}{8}$ inch by $1-\frac{1}{2}$ inch wide by 5 inch ($10-\frac{1}{2}$ inch band) or 8 inch ($13-\frac{1}{8}$ inch band) or 9 inch ($14-\frac{3}{4}$ inch band) shall be placed in lap between bands. "O" ring gaskets shall be $\frac{13}{16}$ inch minimum diameter ($10-\frac{1}{2}$ inch and $13-\frac{1}{8}$ inch bands) and $1-\frac{3}{8}$ inch minimum diameter ($14-\frac{3}{8}$ inch bands).
- 4. Watertight joints with gaskets are required for irrigation pipes, storm sewers, and other installations when shown on the plans. Gaskets for all coupling bands shall be (butt-cemented or vulcanized) synthetic, closed-cell sponge rubber $\frac{3}{8}$ inch thick of a width equal to the band width and centering on the joint. For pipes 12 inches or less in diameter, gasket thickness may be $\frac{3}{16}$ inch.
- 5. Joints for sanitary sewers and siphons are to be tested for water tightness in accordance with the Standard Specifications.
- 6. One or two piece coupling bands are optional for pipe diameters up to and including 42 inches. Coupling bands of two or more pieces are required for pipe diameters over 42 inches.
- 7. To prevent galvanic action when unlike metals are connected, the connecting band shall be used with a full width neoprene gasket or coated with asphalt or other insulating material as approved by the engineer.
- 8. See Std. Dwgs. RD330 and RD332 for pipe slope anchors, when required.



SINGLE 12" BAND ANGLE CONNECTOR

The selection and use of this
Standard Drawing, while
designed in accordance with
generally accepted engineering
principles and practices, is the
sole responsibility of the user
and should not be used without
first consulting a Registered
Professional Engineer.

COUPLING BANDS FOR CORRUGATED METAL PIPE

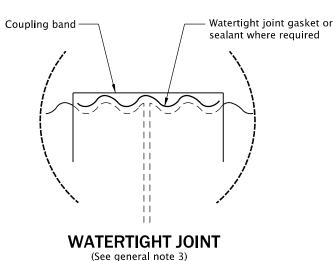
All materials shall be in accordance with the current Oregon Standard Specifications.

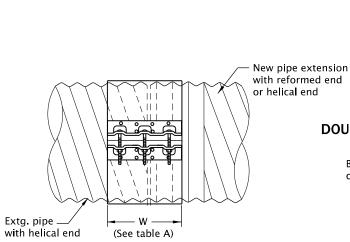
OREGON STANDARD DRAWINGS

2021

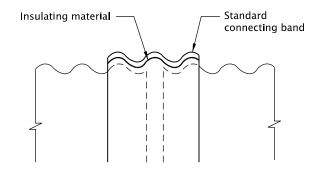
DATE REVISION DESCRIPTION
01-2023 REVISED DETAILS AND NOTES

CALC.
BOOK NO. ____N/A ____ SDR DATE 20-JAN-2023 RD326



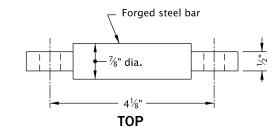


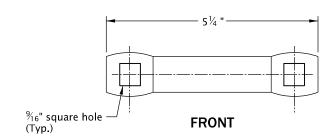
TYPE K FLAT BAND OR DIMPLE BAND (FOR PIPE EXTENSIONS ONLY)



CONNECTION DETAILS

Extending existing pipe culvert with pipe of unlike material Type varies (See general note 2)





BOLT BAR DETAIL

NOTE

½"x6" carriage

bolt (Typ.)

½"x6" carriage

bolt (Typ.)

Bar & Strap

connector

DOUBLE BAR & STRAP CONNECTOR DETAIL

BAND ANGLE CONNECTOR DETAIL

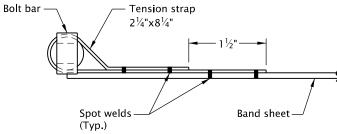
Band angle

connector

Design variations in fasteners (Straps, bars, & welds) which provide a tensile strength of 7500 lbs are permissible.

GENERAL NOTES FOR ALL DETAILS ON THIS SHEET:

- 1. See Std. Dwgs. RD325 and RD326 for general notes, and details not shown.
- 2. To prevent galvanic action when unlike metals are connected, the connecting band shall be used with a full width neoprene gasket or coated with asphalt or other insulating material as approved by the engineer.
- 3. Watertight joints with gaskets are required for irrigation pipes, storm sewers, and other installations when shown on the plans. Gaskets for all coupling bands shall be (butt-cemented or vulcanized) synthetic, closed-cell sponge rubber $\frac{3}{8}$ " thick of a width equal to the band width and centering on the joint. For pipes 12 inches or less in diameter, the gasket thickness may be $\frac{3}{16}$ ".
- 4. One or two piece coupling bands are optional for pipe diameters up to and including 42 inches. Coupling bands of two or more pieces are required for pipe diameters over 42 inches.



DETAIL A TENSION STRAP

Design variations in fasteners (Straps, bars, & welds) which provide a tensile strength of 7500 lbs are permissible.

TABLE A

.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,							
Corrugations	Pipe Dia. (In)	Min. W (In)	Gasket Type				
2 ² / ₂ "x ¹ / ₂ "	12-48	12					
2/3 //2	54-84	24	Sleeve				
* 3"x1" 5"x1"	54-144	24					
22/"v1/ "	12-48	12					
273 X72	54-84	24	Sleeve				
* 3"x1" 5"x1"	54-96	24					
	2 ² / ₃ "x ¹ / ₂ " * 3"x1" 5"x1" 2 ² / ₃ "x ¹ / ₂ "	Corrugations (In) $2\frac{2}{3}"x\frac{1}{2}"$ $* 3"x1" 5"x1" 54-144$ $2\frac{2}{3}"x\frac{1}{2}"$ $12-48$ $54-84$	Corrugations (In) (In) $2\frac{2}{3}"x\frac{1}{2}"$ $\frac{12-48}{54-84} = \frac{12}{24}$ $\frac{* \ 3"x1" \ 5"x1"}{54-144} = \frac{12}{24}$ $\frac{2\frac{2}{3}"x\frac{1}{2}"}{54-84} = \frac{12}{24}$				

* Arch pipe only

The selection and use of this Standard Drawing, while designed in accordance with generally accepted engineering principles and practices, is the sole responsibility of the user and should not be used without first consulting a Registered Professional Engineer.

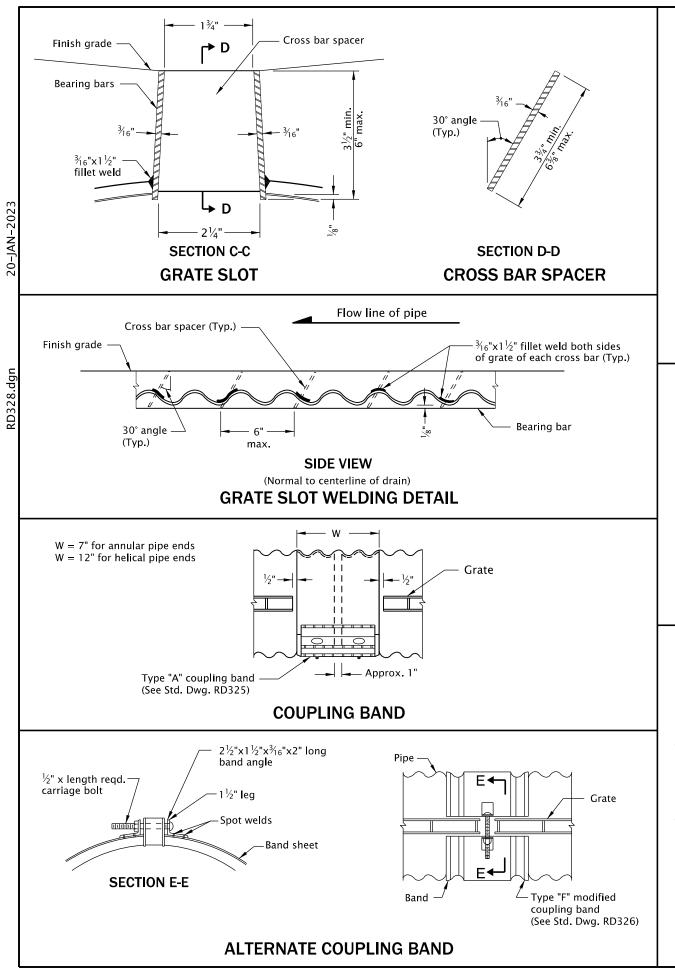
COUPLING BANDS FOR CORRUGATED METAL PIPE

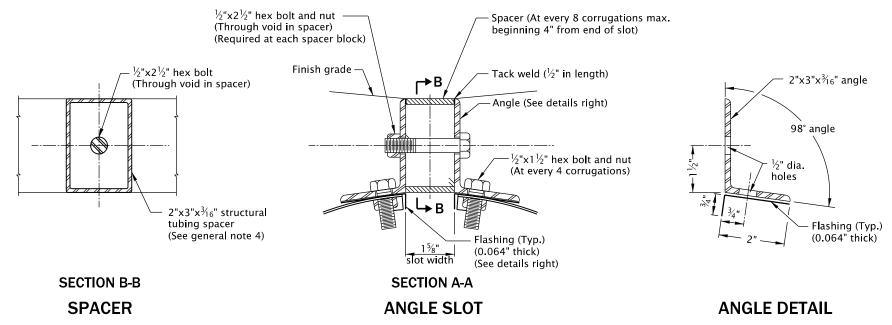
All materials shall be in accordance with the current Oregon Standard Specifications.

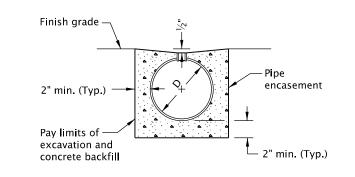
OREGON STANDARD DRAWINGS

2021

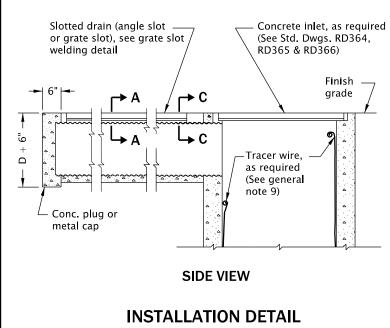
	REVISION DESCRIPTION	DATE
	REVISED DETAILS AND NOTES	01-2023
RD327	SDR 20-JAN-2023	CALC. OOK NO







END VIEW CONCRETE ENCASEMENT



GENERAL NOTES FOR ALL DETAILS ON THIS SHEET:

- 1. Drain pipe seams may be continuous helical lock seam or helical weld seam.
- 2. Drain sections shall be assembled with either of the coupling bands shown.
- 3. Units of slotted CMP drains shall be fabricated and all nuts and bolts tightened prior to beginning field installation.
- 4. Structural tubing to be 5.5 lb/ft or heavier. Structural tubing shall conform to ASTM ASO1
- 5. The cross bar spacer shall be welded to the bearing bars in such a manner as to develop a minimum tensile capacity of 12000 lbs normal to the longitudinal axis of the bearing bars.
- 6. The maximum variance from a straight line between the extreme top corners of the bearing bars shall be $\frac{1}{2}$ " in 20'.
- Grate slot assemblies shall conform to the provisions of ASTM A36 and shall be galvanized after fabrication per ASTM A123. Corrugated pipe shall conform to AASHTO M36.
- 8. Concrete used in encasement shall be commercial grade concrete.
- 9. See Std. Dwg. RD336 for tracer wire details.
- 10. See ODOT's QPL for alternates.

The selection and use of this
Standard Drawing, while
designed in accordance with
generally accepted engineering
principles and practices, is the
sole responsibility of the user
and should not be used without
first consulting a Registered
Professional Engineer.

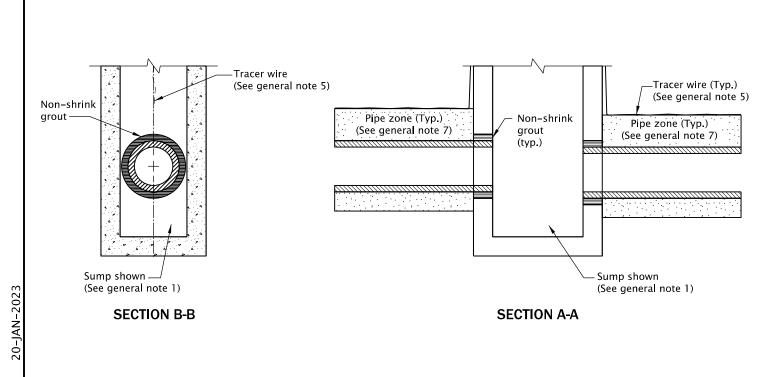
OREGON STANDARD DRAWINGS
SLOTTED CORRUGATED METAL
PIPE (CMP)
DRAIN DETAILS

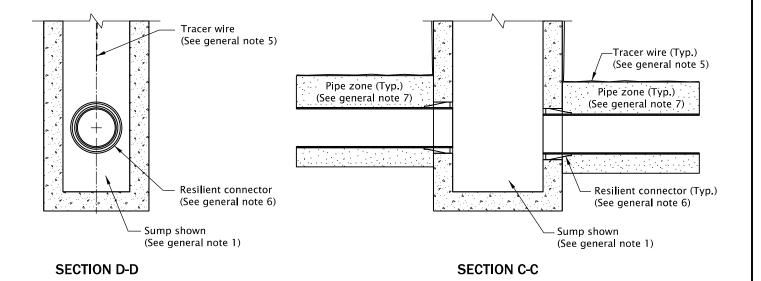
All materials shall be in accordance with

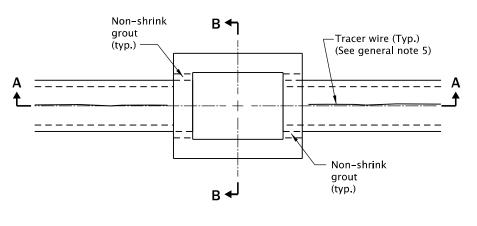
the current Oregon Standard Specifications.

DATE REVISION DESCRIPTION
01-2023 REVISED NOTE

CALC.
BOOK NO. _ N/A _ DATE 20-JAN-2023 RD328







Resilient connector (Typ.)

(See general note 6)

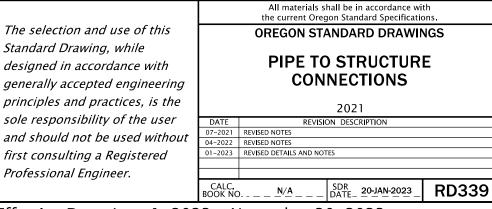
PLAN

CONNECTION OF RIGID PIPE TO STRUCTURE

CONNECTION OF FLEXIBLE PIPE TO STRUCTURE

GENERAL NOTES FOR ALL DETAILS ON THIS SHEET:

- 1. See Std. Dwgs. RD364, RD365, and RD366 for inlet details not shown.
- 2. See appropriate standard drawings or special project details for other similar structures.
- 3. Location, elevation, diameter, slope, and number of pipe(s) varies, see project plans.
- 4. Maximum pipe diameter varies with pipe material.
- 5. All connecting pipes shall have a tracer wire, or approved alternate. See Std. Dwg. RD336 for tracer wire details.
- 6. When flexible pipe is used, install resilient connectors conforming to requirements of ASTM C923.
- 7. Pipe zone varies, see Std. Dwg. RD300.

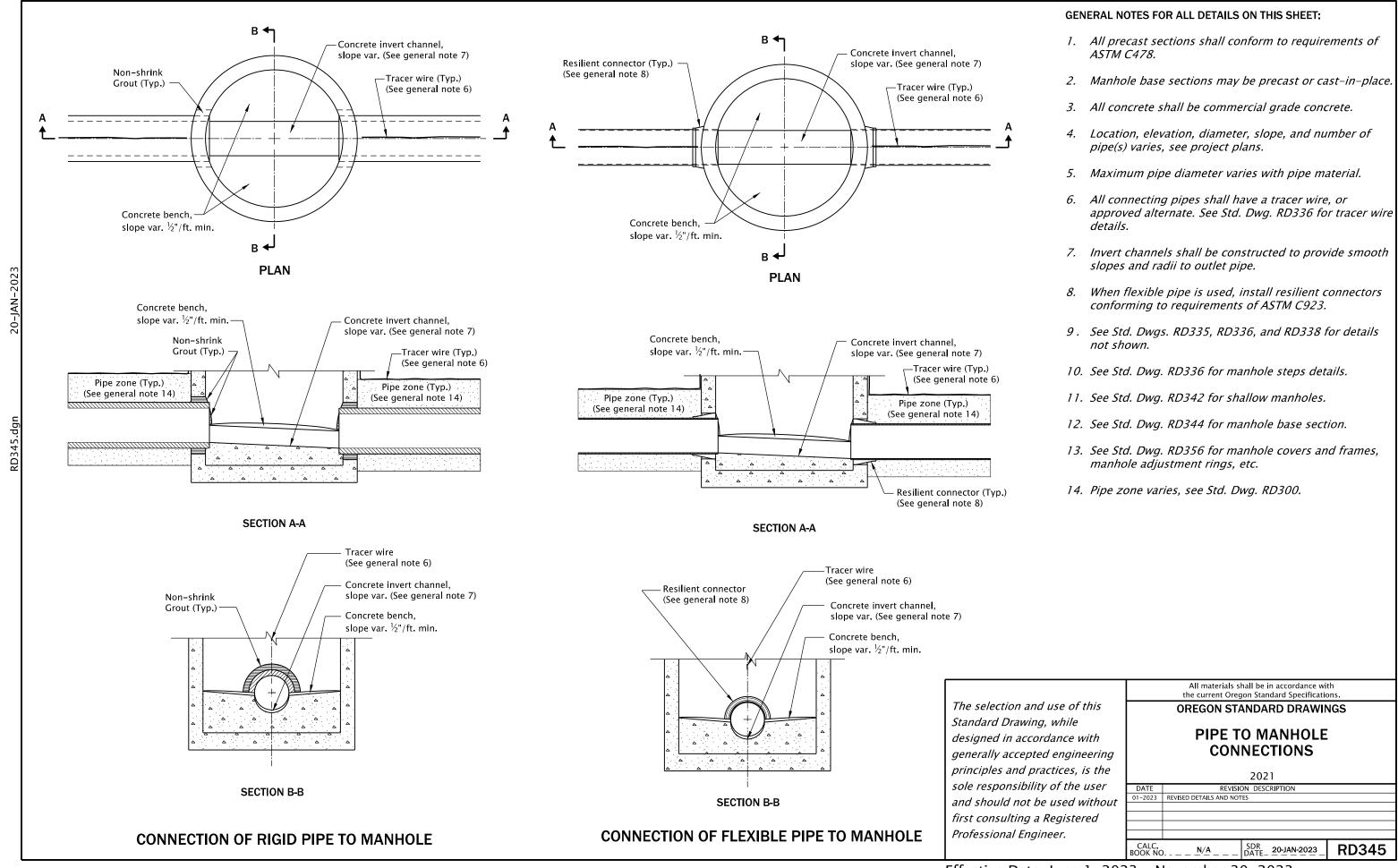


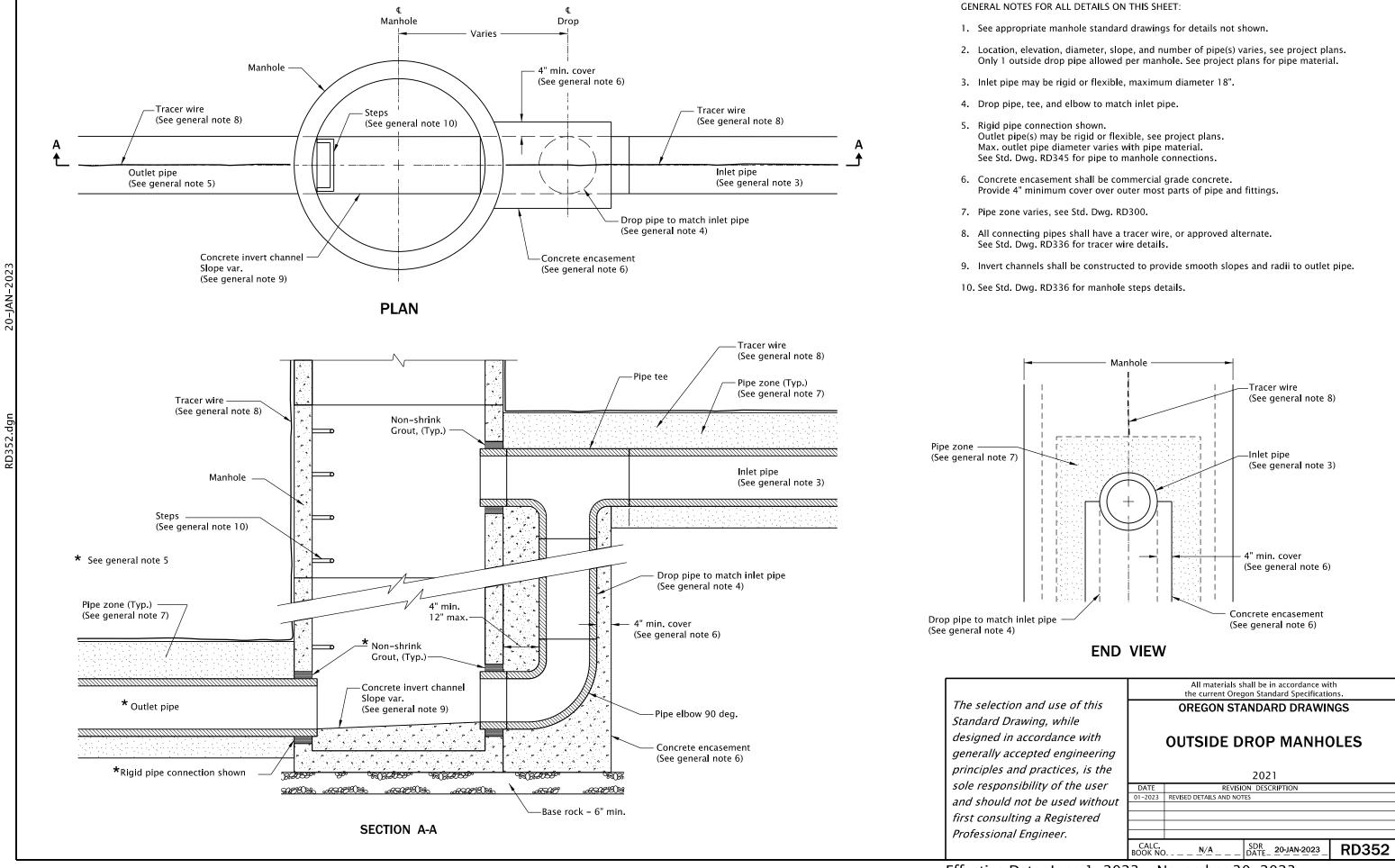
PLAN

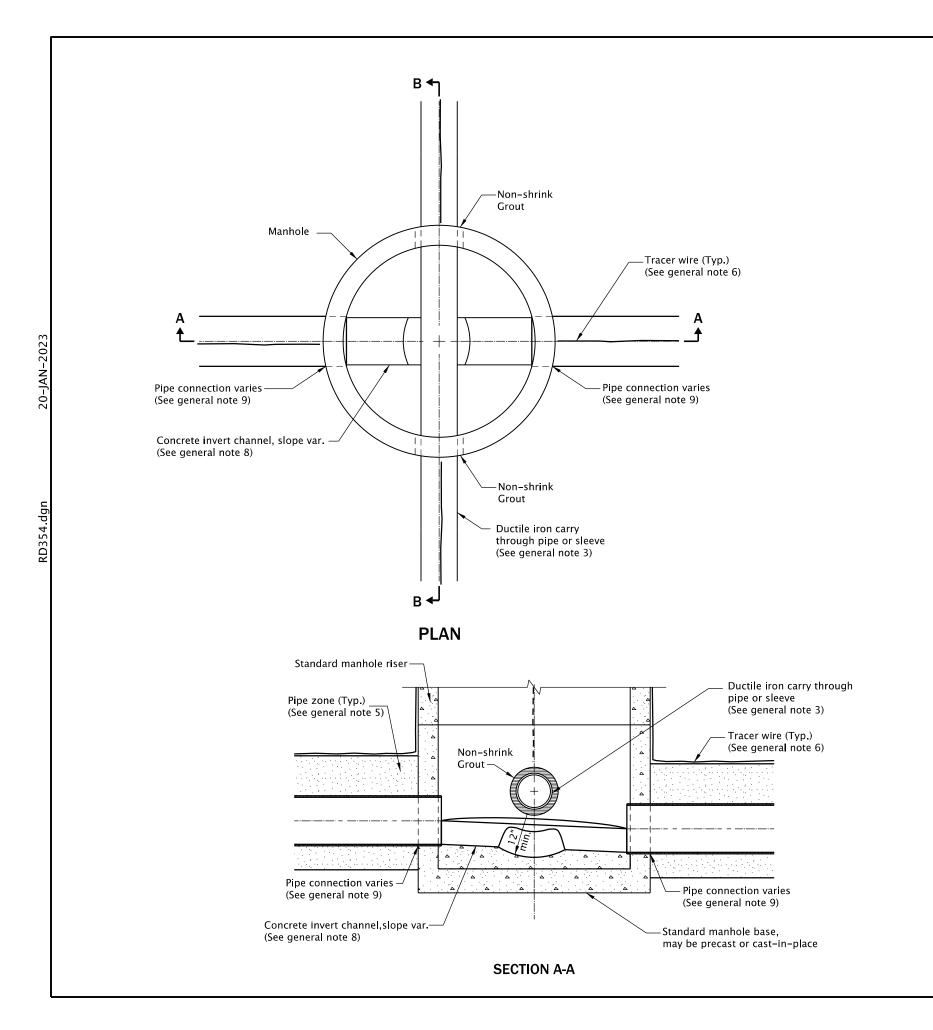
D ←

-Tracer wire (Typ.)

(See general note 5)

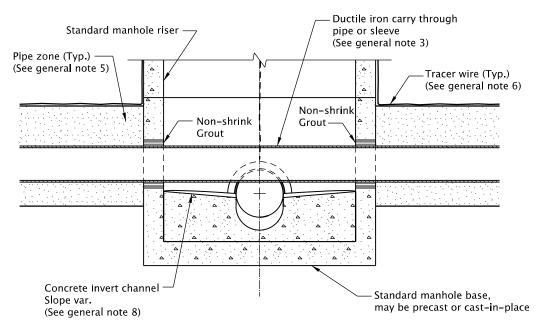






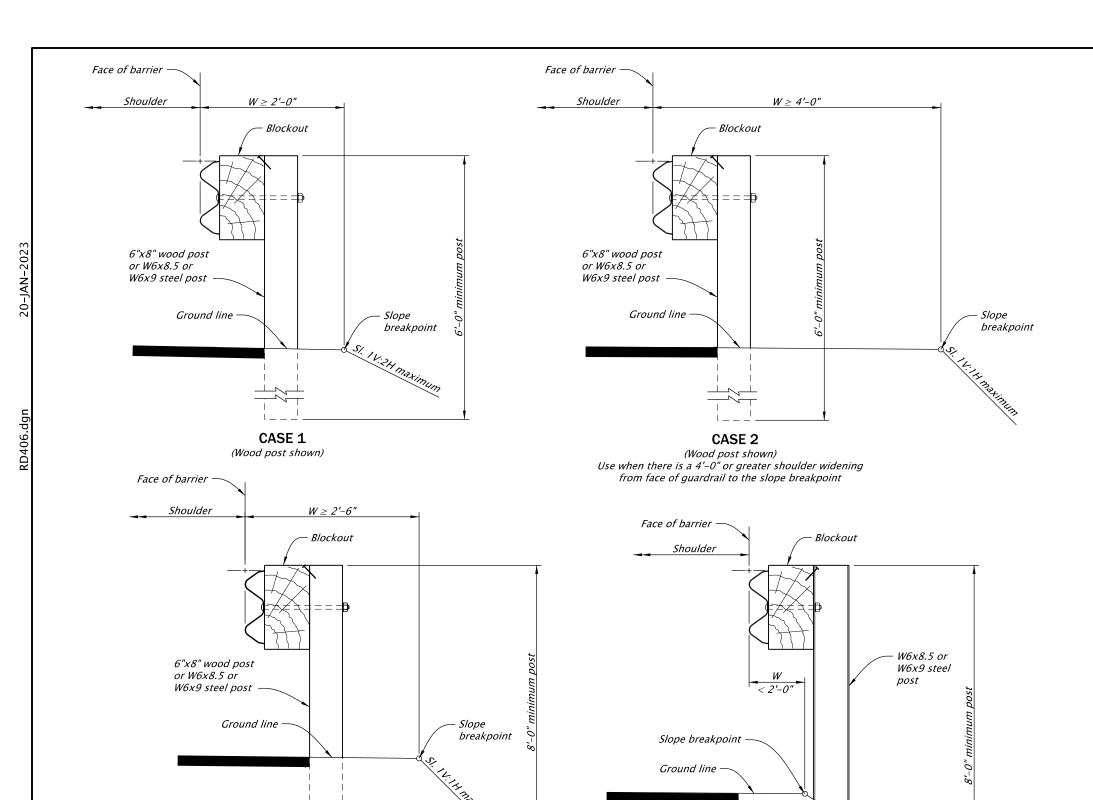
GENERAL NOTES FOR ALL DETAILS ON THIS SHEET:

- 1. See appropriate manhole standard drawings for details not shown.
- 2. Location, elevation, diameter, slope, and number of pipe(s) varies, see project plans. See project plans for pipe material.
- 3. Carry through pipe or sleeve shall be ductile iron, class as specified. No joints allowed on the carry through pipe or sleeve inside the manhole.
- 4. This manhole design shall be used only as directed by the engineer to mitigate unavoidable grade conflicts.
- 5. Pipe zone varies, see Std. Dwg. RD300.
- 6. All connecting pipes shall have a tracer wire, or approved alternate. See Std. Dwg. RD336 for tracer wire details.
- 7. See Std. Dwg. RD336 for manhole steps details.
- 8. Invert channels shall be constructed to provide smooth slopes and radii to outlet pipe.
- 9. See Std. Dwg. RD345 for pipe to manhole connections.



SECTION B-B

All materials shall be in accordance with the current Oregon Standard Specifications. The selection and use of this **OREGON STANDARD DRAWINGS** Standard Drawing, while **CARRY THROUGH MANHOLE** designed in accordance with **STORM** generally accepted engineering principles and practices, is the 2021 sole responsibility of the user DATE REVISION DESCRIPTION 01-2023 REVISED DETAILS AND NOTES and should not be used without first consulting a Registered Professional Engineer. CALC BOOK NO. _ _ _ _ N/A _ SDR DATE_ 20-JAN-2023 **RD354**



PLACEMENT OF GUARDRAIL ON SLOPES

NOTE: Cases shown do not apply to terminals,

transition sections or anchors.

CASE 3

(Wood post shown)
Use when there is a 2'-6" or greater shoulder widening

from face of guardrail to the slope breakpoint

GENERAL NOTES FOR ALL DETAILS ON THIS SHEET:

- 1. See appropriate guardrail standard drawing(s) for details not shown.
- 2. Wood blocks shown. Blocks of an approved alternate material may be used. See ODOT's OPL.
- 3. All posts for guardrail run shall be of the same type: wood or steel.

SLOPE / EMBANKMENT TABLE						
POST LENGTH (ft)	POST TYPE	SLOPE (V:H)	W (ft) (Face of barrier to slope of breakpoint)			
6	Wood/Steel	1:2 or flatter	2'-0" minimum			
6	Wood/Steel	1:1 or flatter	4'-0" minimum			
8	Wood/Steel	1:1 or flatter	2'-6" minimum			
8	Steel	1:1½ or flatter	Less than 2'-0"			

The selection and use of this
Standard Drawing, while
designed in accordance with
generally accepted engineering
principles and practices, is the
sole responsibility of the user
and should not be used without
first consulting a Registered
Professional Engineer.

All materials shall be in accordance with the current Oregon Standard Specifications.

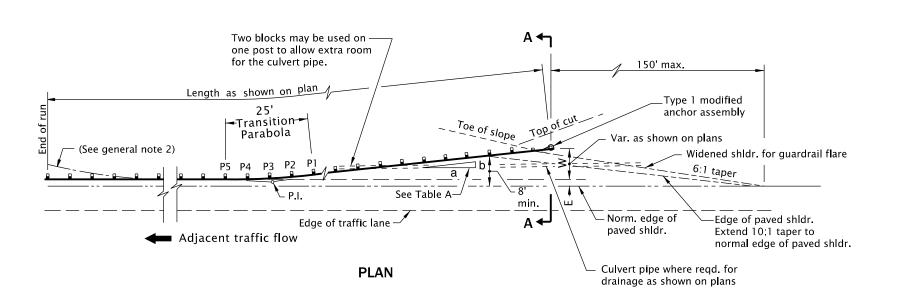
OREGON STANDARD DRAWINGS

PLACEMENT OF GUARDRAILS ON SLOPES

2021

DATE	REVISION DESCRIPTION						
07-2021	NEW DRAWING CREATED						
12-2021	REVISED DETAILS AND NOTES						
12-2022	REVISED NOTE						
CALC. BOOK NO	D N/A SDR DATE 20-JAN-2023 RD406						

designed in accordate generally accepted of principles and practive sole responsibility of and should not be use when there is less than a 2'-0" shoulder widening from face of guardrail to the slope breakpoint designed in accordate generally accepted of principles and practive and should not be used to sole responsibility of and should not be used first consulting a Responsibility of an acceptance of the should not be used for the shope breakpoint and should



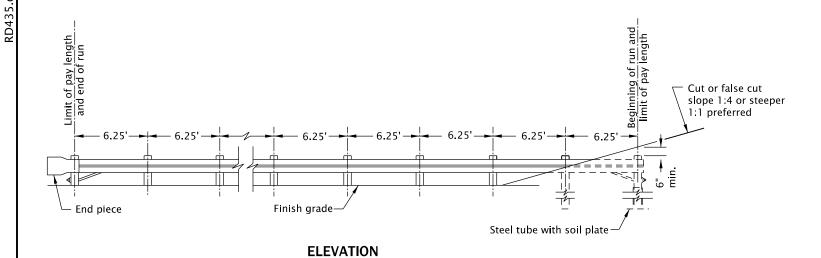
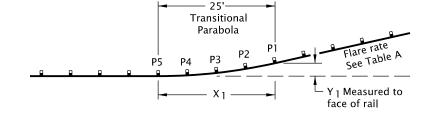


TABLE AFLARE RATE & 25' TRANSITION PARABOLA

FLARE		POST NUMBER				
RATE		P5	P4	P3	P2	P1
a b	X (ft)	0	6.25	12.49	18.72	24.92
15:1	Y (ft)	0	0.05	0.21	0.47	0.83

E= 2' where shown on plans.

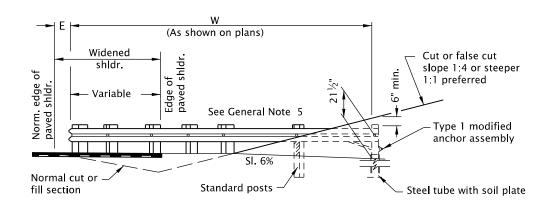


TERMINALS (Cut or false cut)

GENERAL NOTES FOR ALL DETAILS ON THIS SHEET:

- 1. See appropriate guardrail standard drawing(s) for details not shown.
- 2. On two way, two lane highways, both ends of guardrail runs shall be provided with a flared or non-flared terminal from ODOT's QPL if not buried, as detailed on plans.
- 3. Type 2A guardrail shown.
- 4. Where terminal end is buried, a Type 1 modified anchor will be attached to the end post.
- 5. Trailing ends (freeways, multilane and similar one-way facilities) not exposed to opposing traffic
 - (a) Guardrail terminals, use a Type 1 modified anchor, Type B end piece and do not flare.
 - (b) At bridge ends, omit Transition Guardrail & Type 3 guardrail and substitute the normal required guardrail.
- 6. (a) All bolts except adjustment bolts shall be drawn tight on rails and components on initial installation.
 - (b) Final tightness check on rail and component bolts and retightening as required to be done 30 days after initial installation.
- 7. If the cut or false cut slope is flatter than 1:1 ensure that there is a clear recovery area behind the guardrail, with no funneling effect to the back of the obstruction.
- 8. "W" distance is measured to face of guardrail at end post.

NOTE: THIS DRAWING IS RETAINED FOR MAINTENANCE PURPOSES. DO NOT USE FOR NEW CONSTRUCTION.



SECTION A-A (CUT OR FALSE CUT)

The selection and use of this Standard Drawing, while designed in accordance with generally accepted engineering principles and practices, is the sole responsibility of the user and should not be used without first consulting a Registered Professional Engineer.

OREGON STANDARD DRAWINGS
GUARDRAIL INSTALLATION
TERMINAL (CUT OR FALSE-CUT)
(29" RAIL HEIGHT)

All materials shall be in accordance with

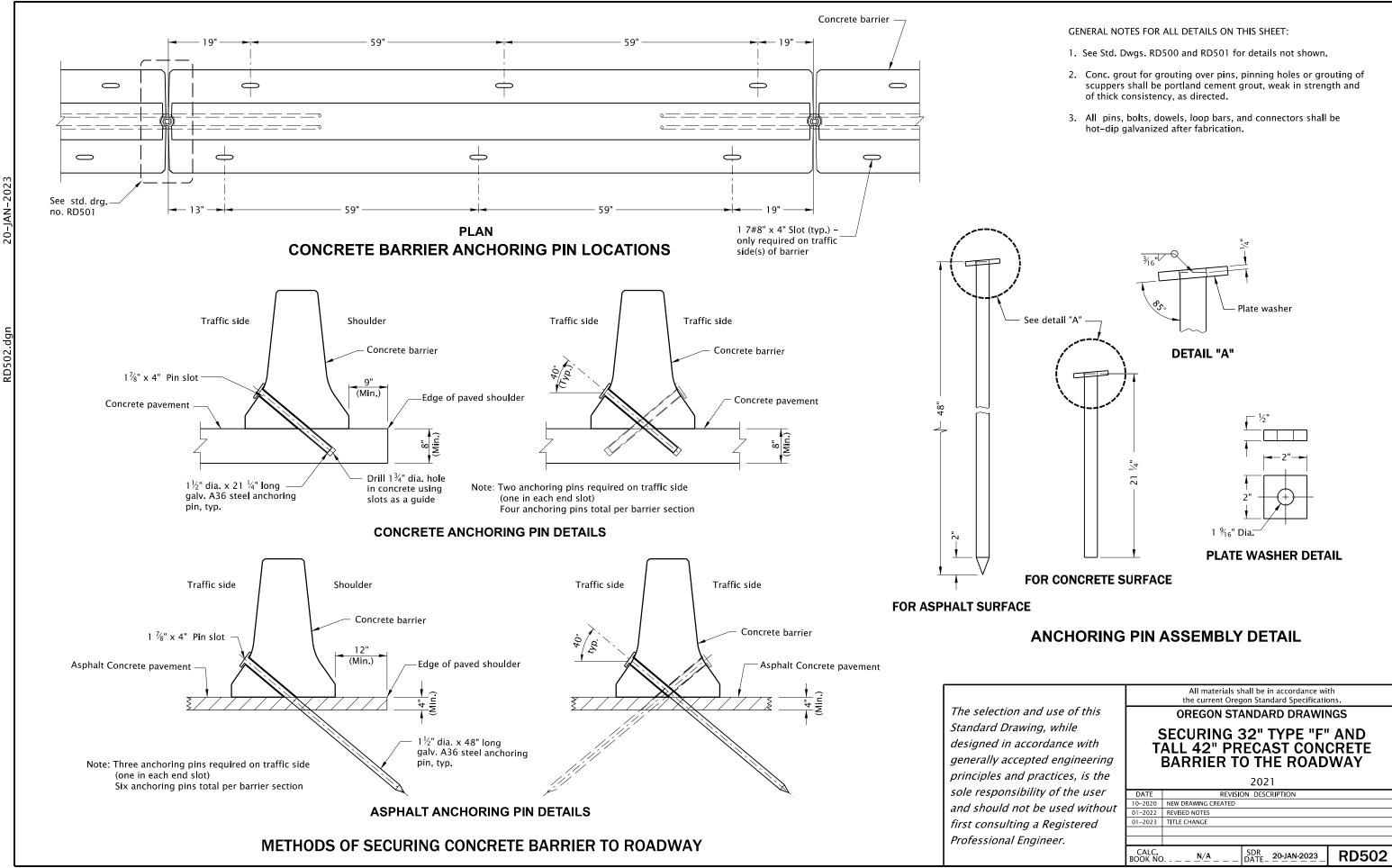
the current Oregon Standard Specifications.

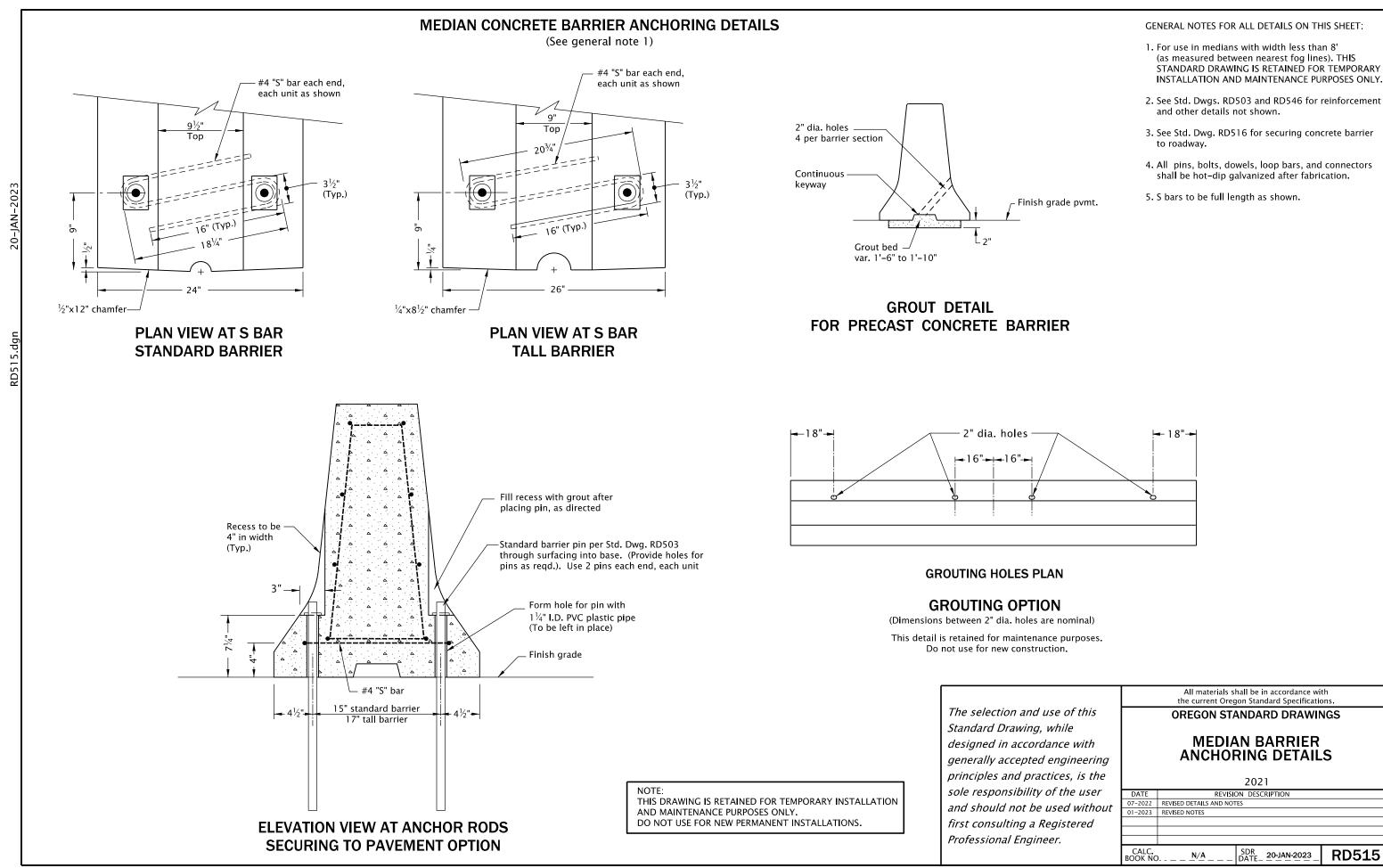
DATE REVISION DESCRIPTION

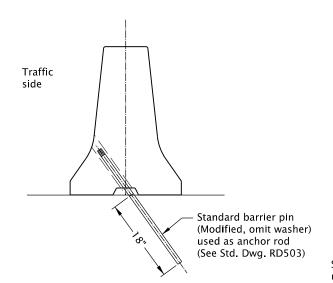
01-2023 REVISED NOTES

CALC.
BOOK NO. _ N/A _ DATE 20-JAN-2023 RD435

2021







Standard barrier pin used as anchor rod (See Std. Dwg. RD503)

METHODS OF SECURING PRECAST CONCRETE BARRIER TO ROADWAY

(See general note 1)

THIS DETAIL IS RETAINED FOR MAINTENANCE PURPOSES.

SHOULDER INSTALLATION

Secured using anchor rods (Angled) Not allowed in narrow medians, see general notes 1 and 6

PRECAST CONCRETE BARRIER

(See Std. Dwg. RD503)

MEDIAN INSTALLATION

Secured using anchor rods (Vertical) (See general note 1)

PRECAST CONCRETE BARRIER

(See Std. Dwgs. RD503 & RD515)

MEDIAN INSTALLATION

Secured using anchor rods (Vertical)

(See general note 1)

PRECAST TALL (42")

CONCRETE BARRIER

(See Std. Dwgs. RD545 and RD515)

SHOULDER OR MEDIAN INSTALLATION

Secured using grout (See general note 1)

PRECAST CONCRETE BARRIER

(See Std. Dwg. RD503)

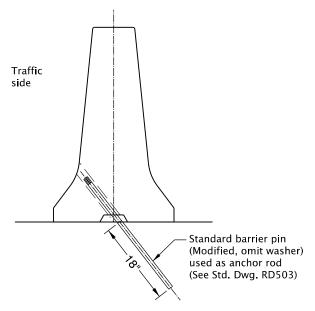
GENERAL NOTES FOR ALL DETAILS ON THIS SHEET:

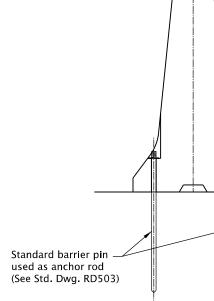
- 1. This standard drawing is intended to show acceptable methods of securing concrete barrier to roadway and is RETAINED FOR TEMPORARY INSTALLATION AND MAINTENANCE PURPOSES.
- 2. Secure concrete barrier to roadway when any of the following conditions exist:
 - a) Barrier deflection requirements per Std. Dwg. RD503 cannot be obtained, or
 - b) When required by plans, or
 - c) As directed by the Engineer.
- 3. Select one of the securing methods shown. For details not shown, see the standard drawing(s) referenced for the selected method.
- 4. Securing concrete barrier to roadway is in addition to connections between adjacent concrete barrier sections, bridge rails, retaining walls, and similar existing or constructed objects.
- 5. Temporary concrete barrier to be precast concrete median barrier with pin and loop assembly.
- Precast concrete barrier used in medians less than 8 feet in width (as measured between nearest fog lines) shall be secured to roadway to resist impacts from both sides.
- 7. Anchor rods are standard barrier pins, modified for shoulder installation, as shown.
- 8. Normal permanent installation of concrete barrier is on top of finish grade, to provide 3 inch vertical reveal. Modify placement when required by plans, or as directed.
- 9. All pins, bolts, dowels, loop bars, and connectors shall be hot-dip galvanized after fabrication.

SHOULDER OR MEDIAN INSTALLATION Secured by continuous shear key (See general note 1)

METHODS OF SECURING CAST-IN-PLACE **CONCRETE BARRIER TO ROADWAY** (See Std. Dwg. RD505)

METHODS OF SECURING PRECAST TALL (42") CONCRETE BARRIER TO ROADWAY (See general note 1)





THIS DETAIL IS RETAINED FOR MAINTENANCE PURPOSES.

SHOULDER OR MEDIAN INSTALLATION

Secured using grout (See general note 1)

PRECAST TALL (42") **CONCRETE BARRIER**

(See Std. Dwgs. RD545 and RD546)

NOTE:

THIS DRAWING IS RETAINED FOR TEMPORARY INSTALLATION AND MAINTENANCE PURPOSES ONLY. DO NOT USE FOR NEW PERMANENT INSTALLATIONS.

The selection and use of this Standard Drawing, while designed in accordance with generally accepted engineering principles and practices, is the sole responsibility of the user and should not be used without first consulting a Registered Professional Engineer.

All materials shall be in accordance with the current Oregon Standard Specifications. **OREGON STANDARD DRAWINGS**

SECURING CONCRETE BARRIER TO ROADWAY

2021

DATE	RE	VISION DESCRIPTION	
01-2023	REVISED NOTES		
CALC.	N/A	SDR 20-JAN-2023	RD516

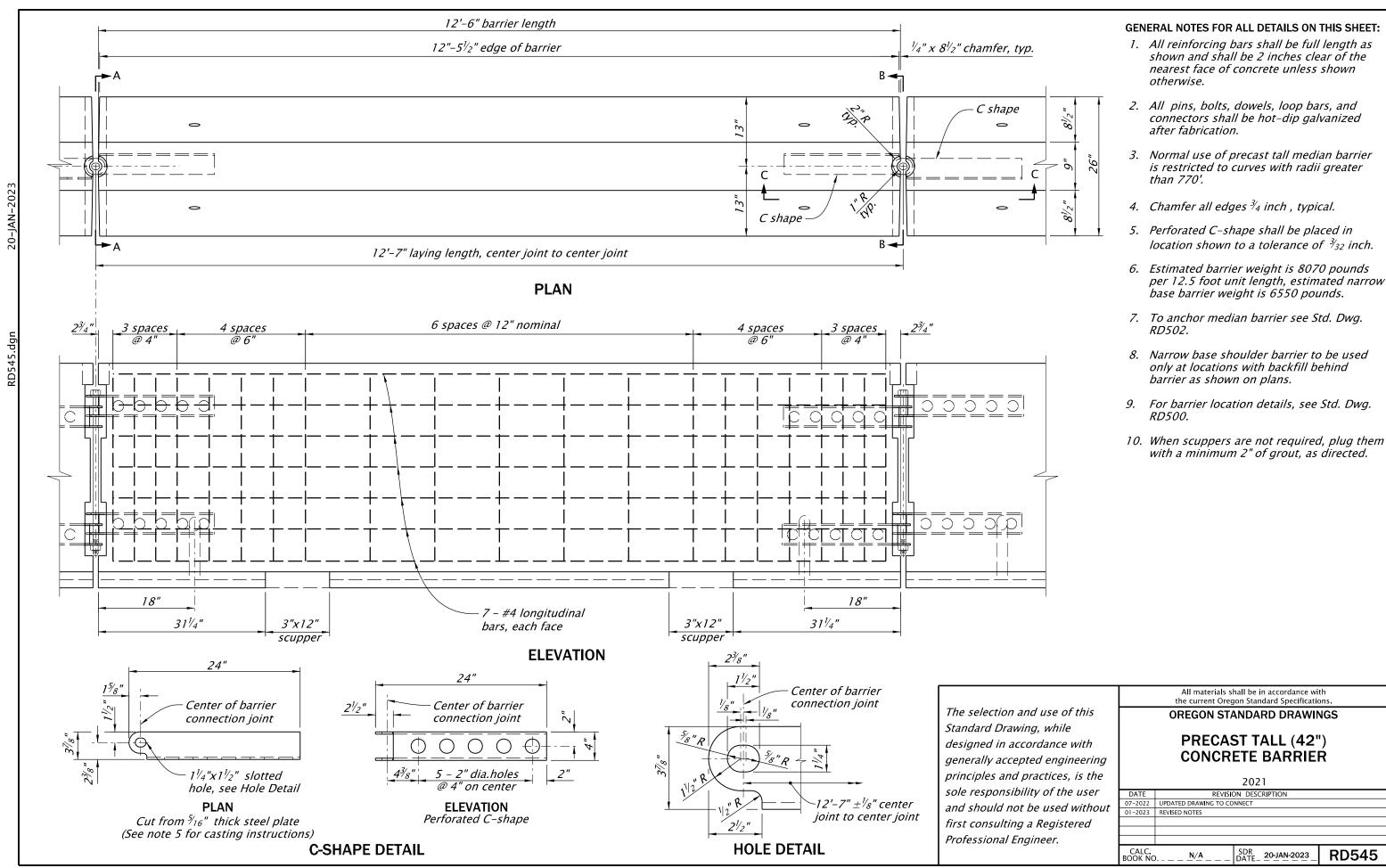
SHOULDER INSTALLATION

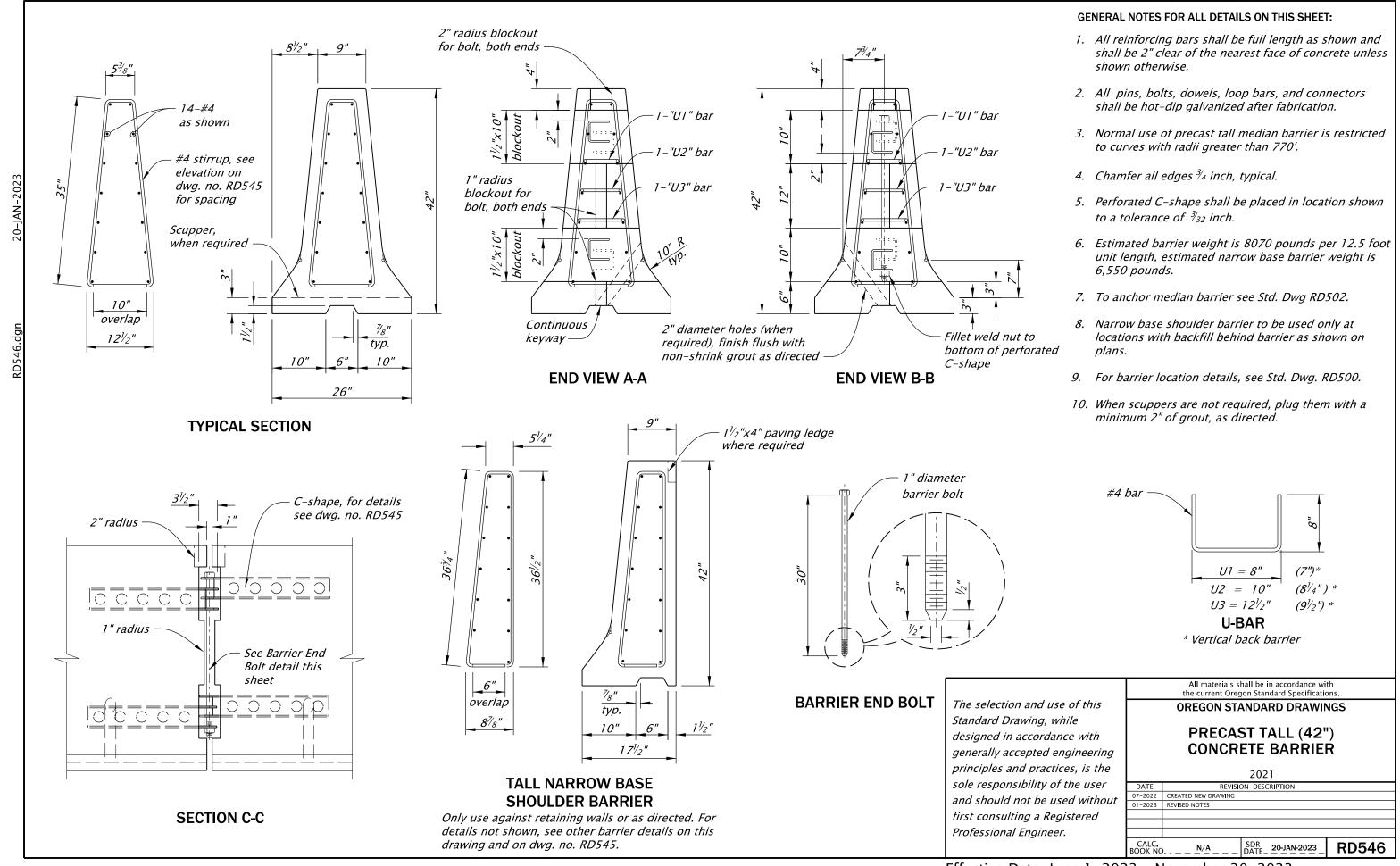
Secured using anchor rods (Angled) not allowed in narrow medians, see general notes 1 and 6

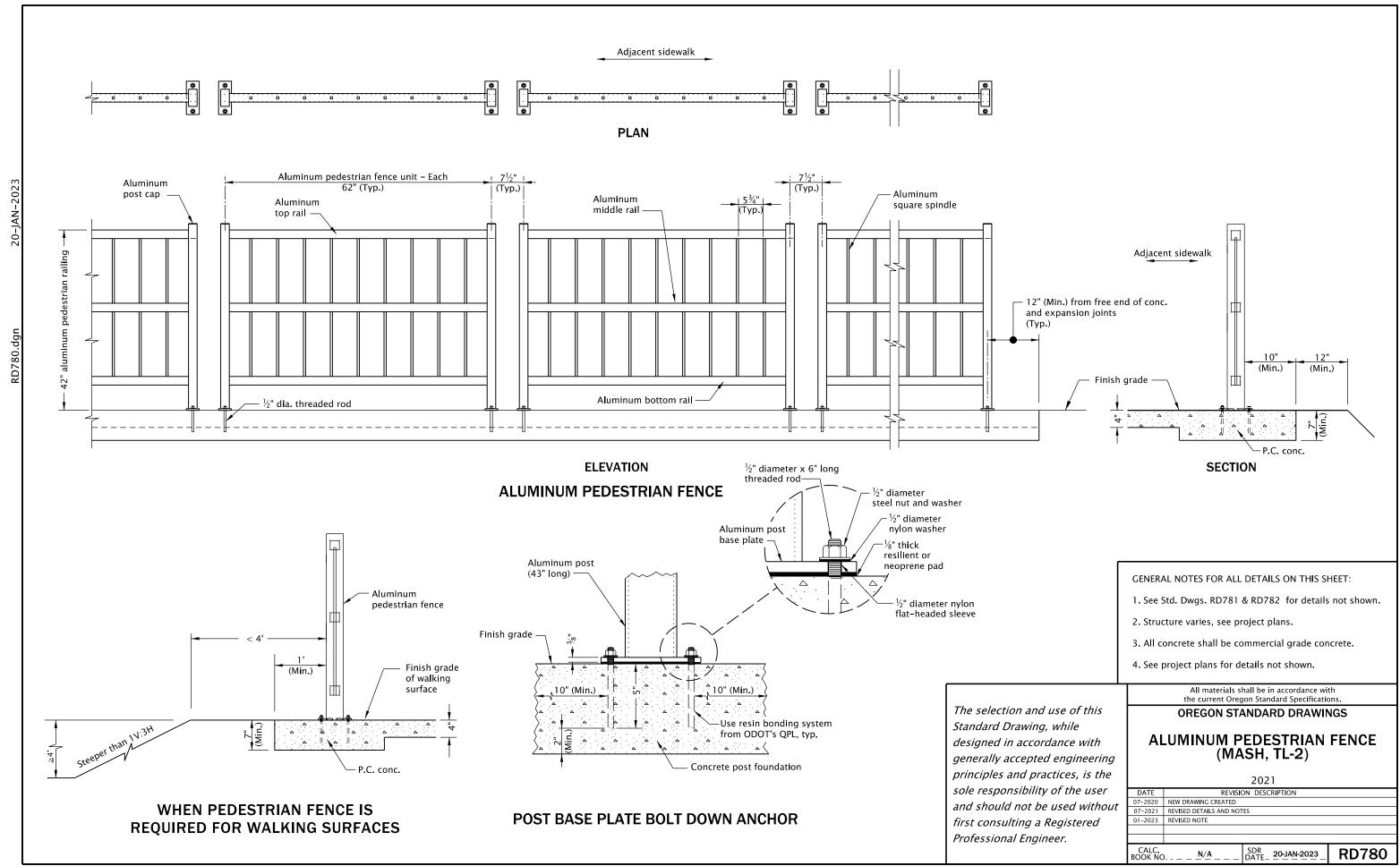
PRECAST TALL (42")

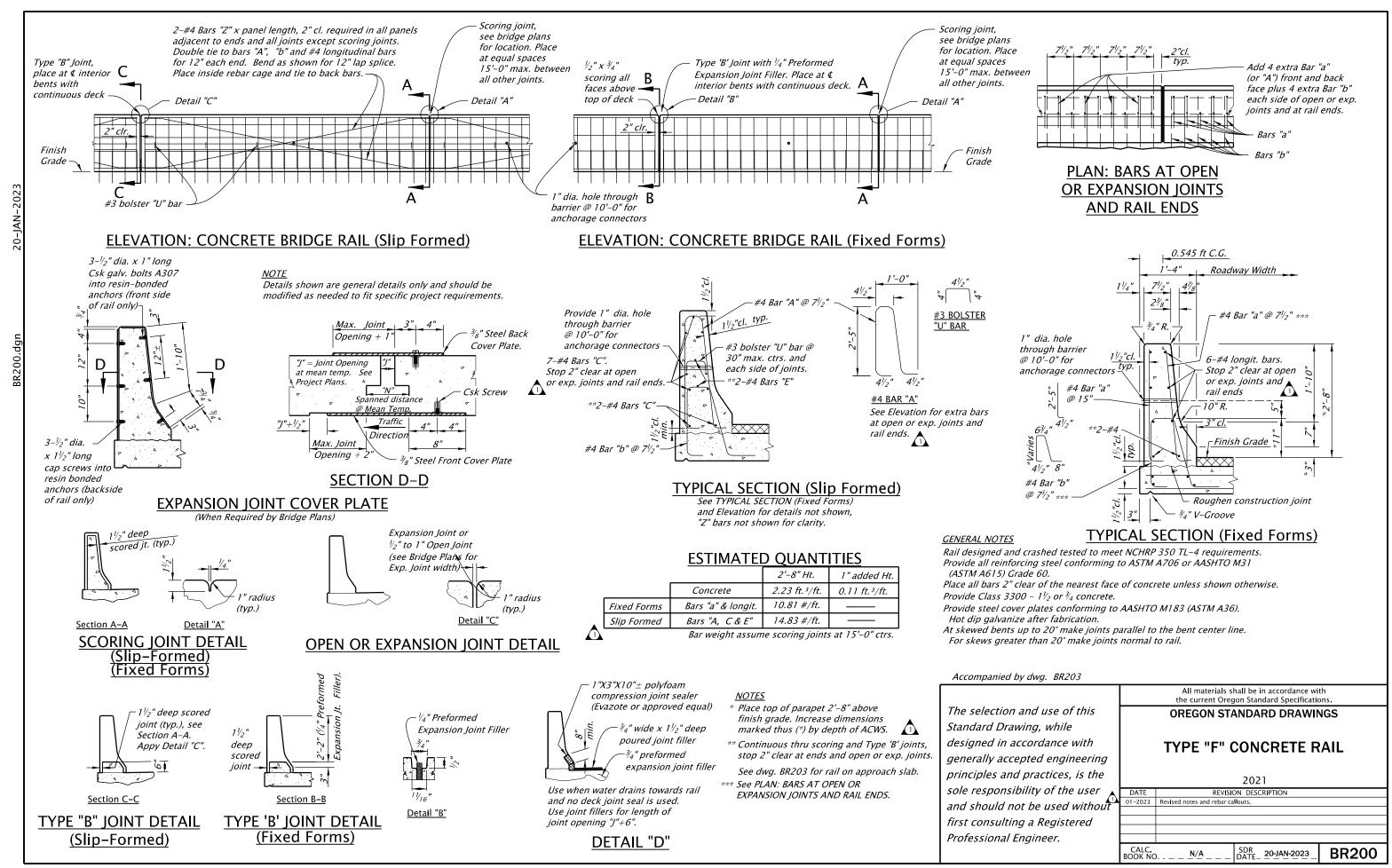
CONCRETE BARRIER (See Std. Dwgs. RD545 and RD546)

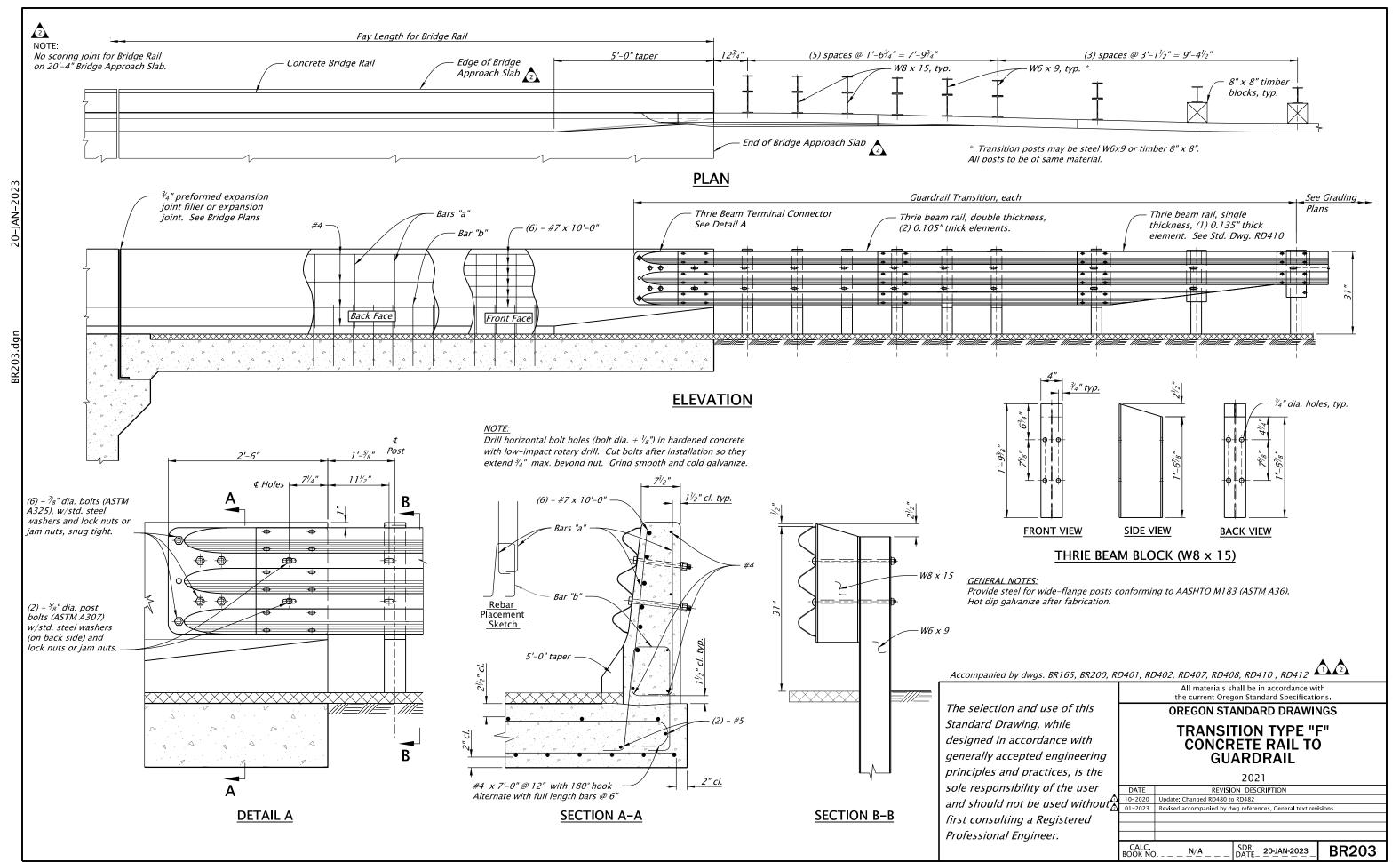
Effective Date: June 1, 2023 - November 30, 2023

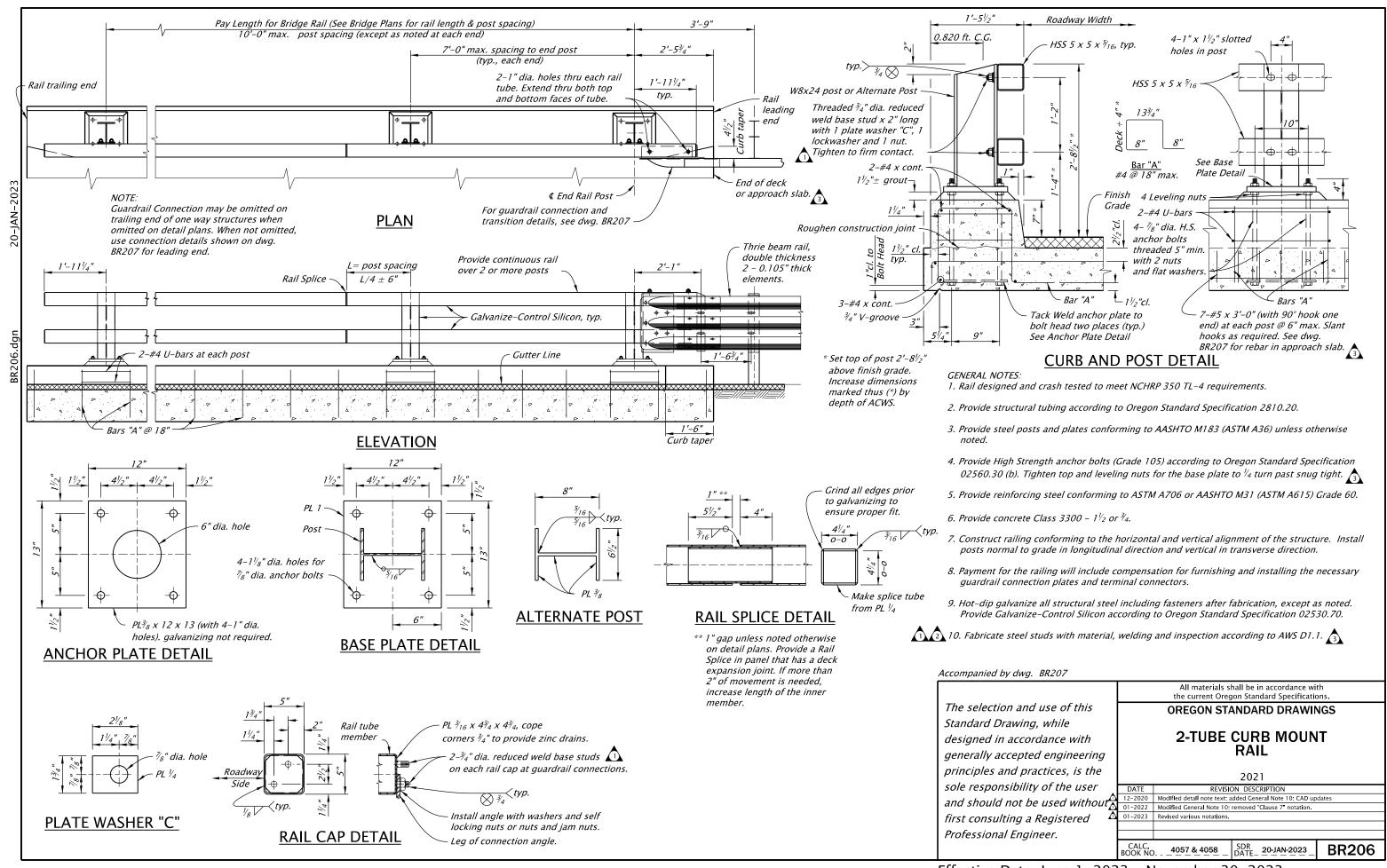


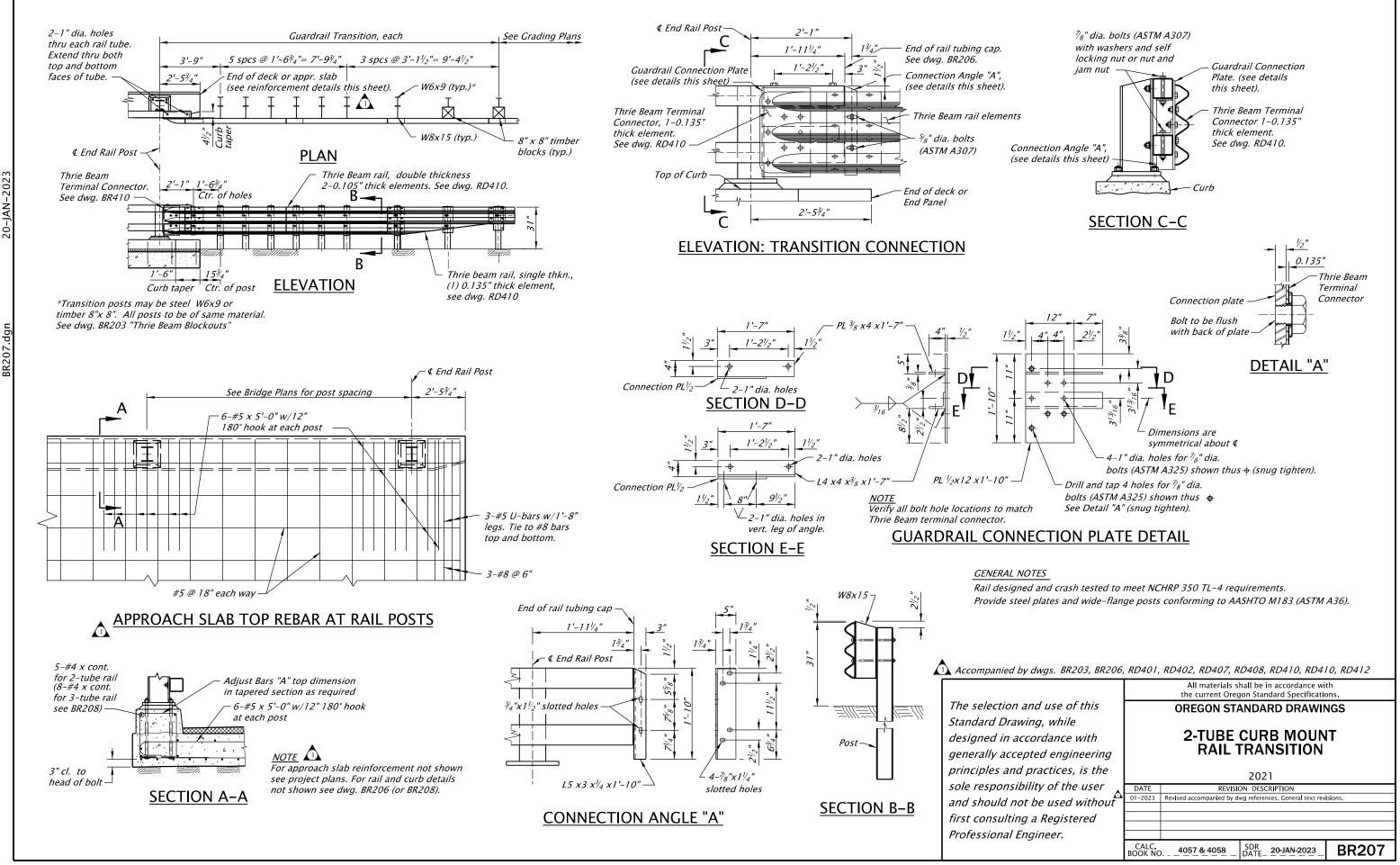


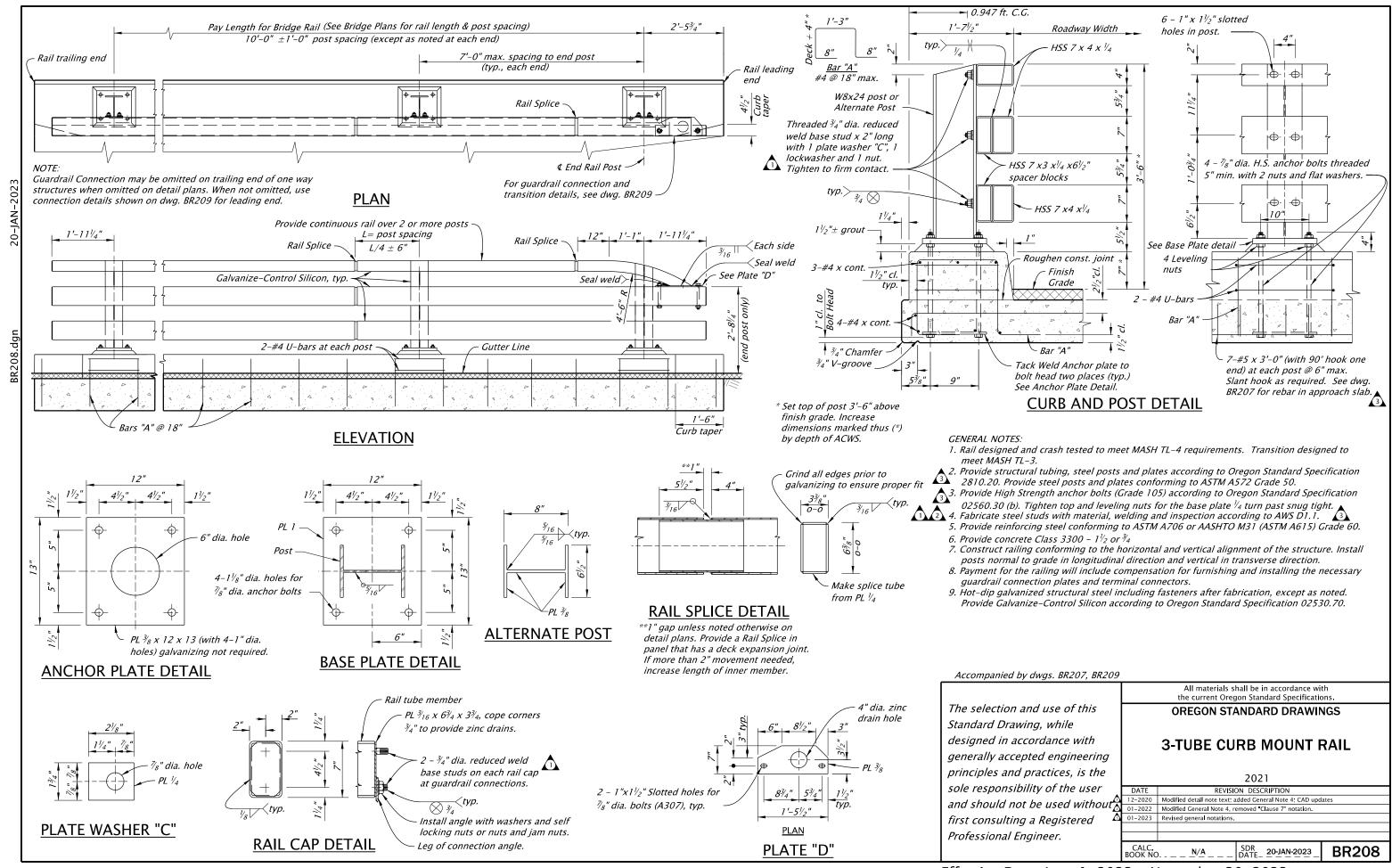


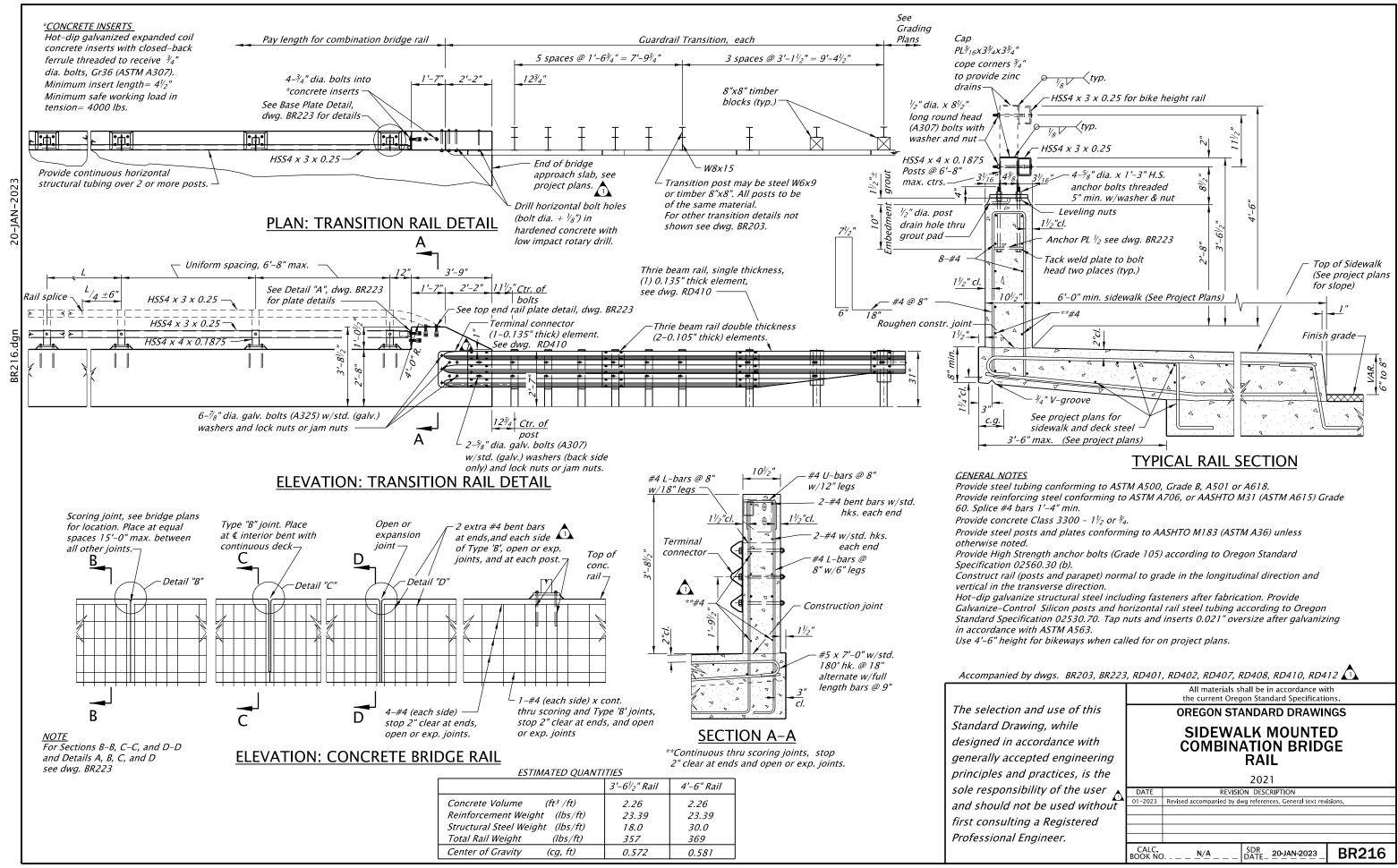


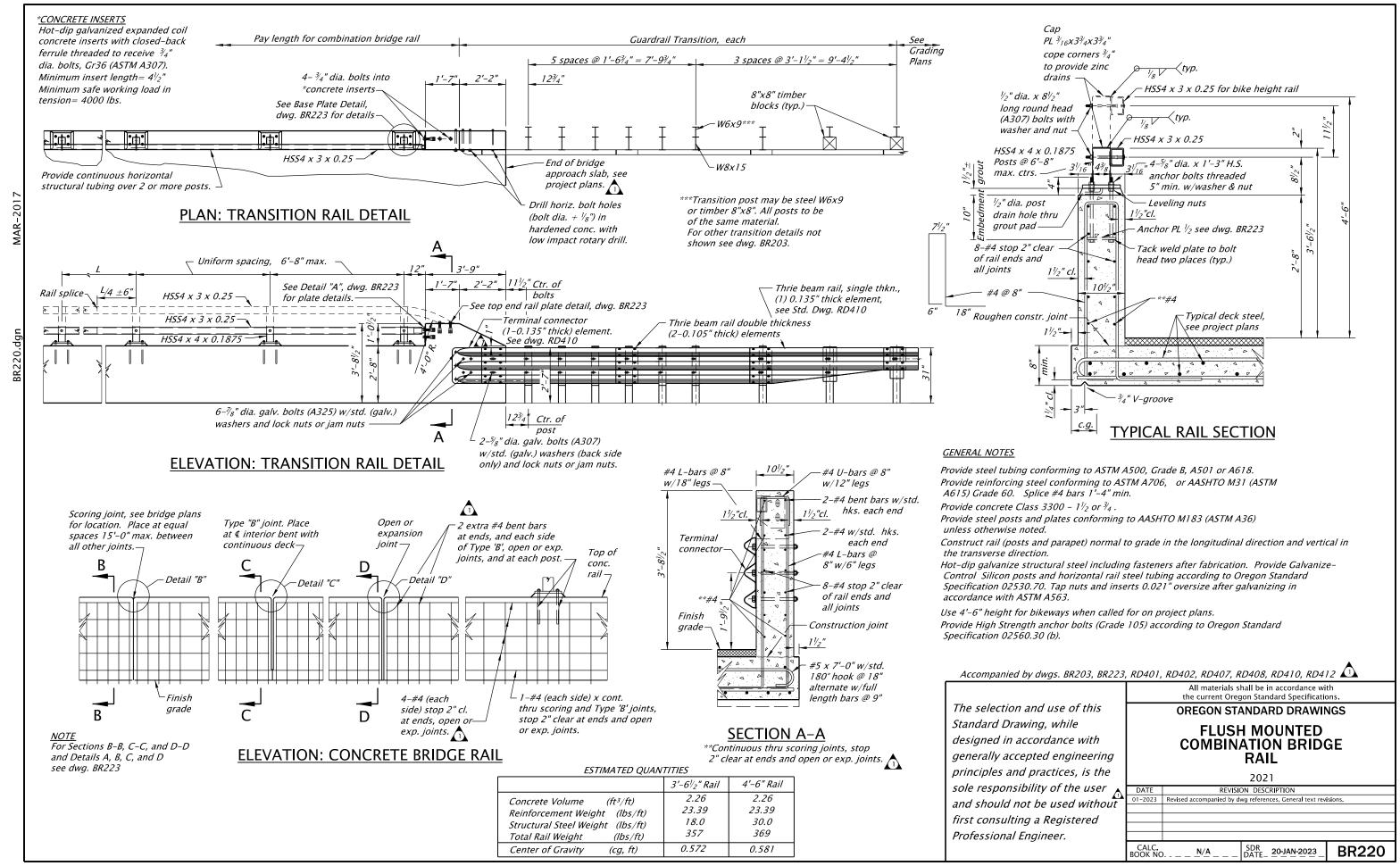


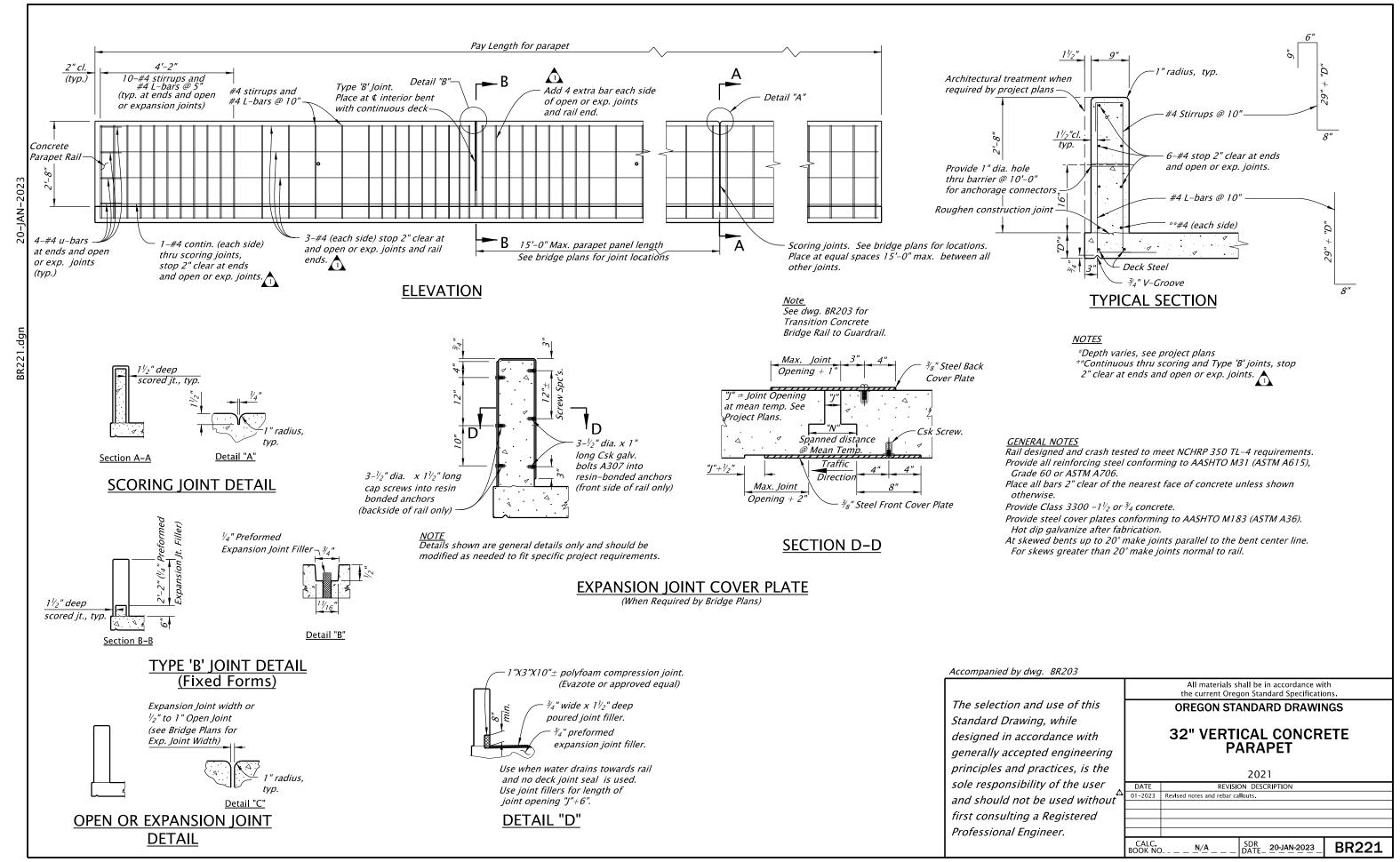


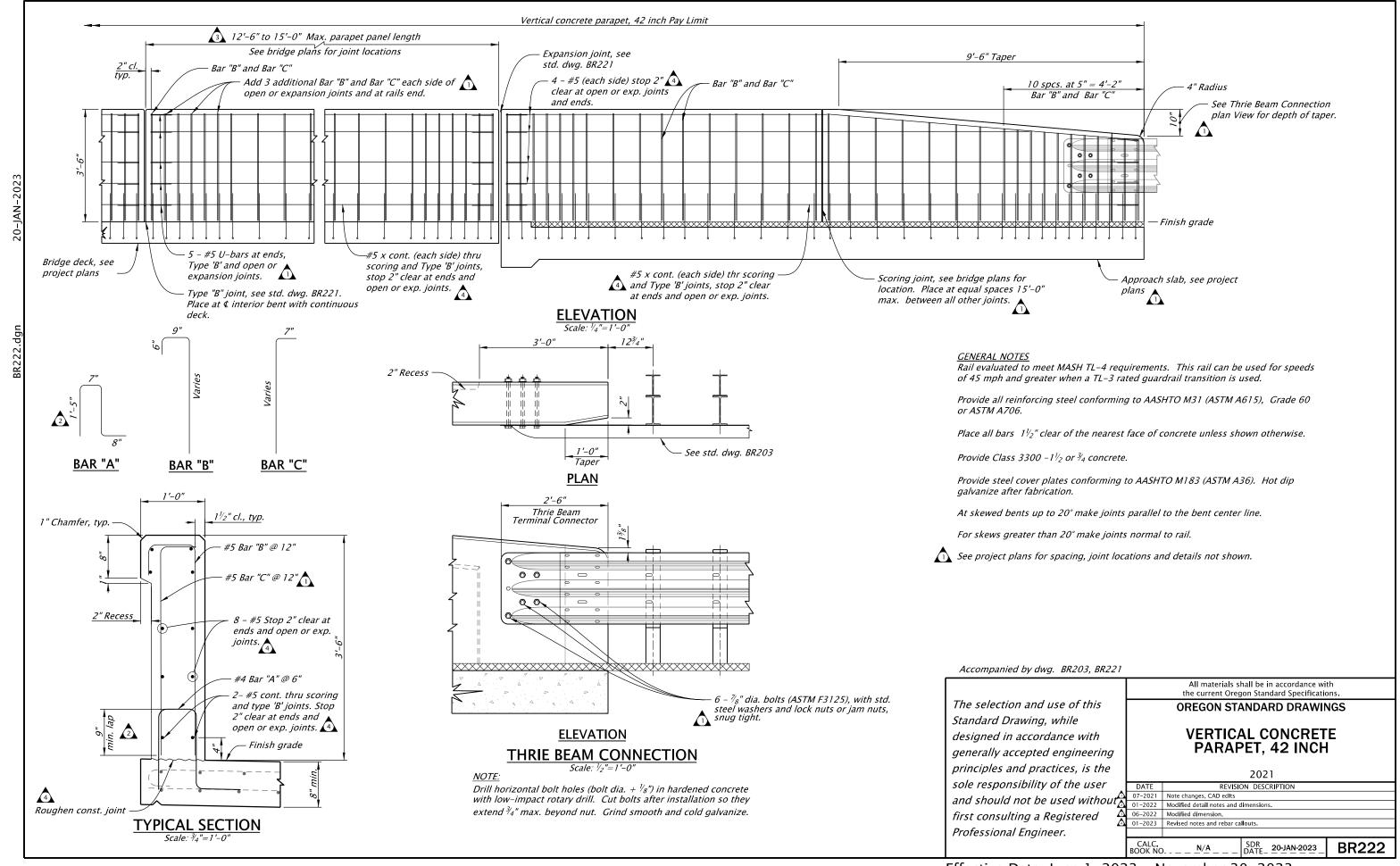


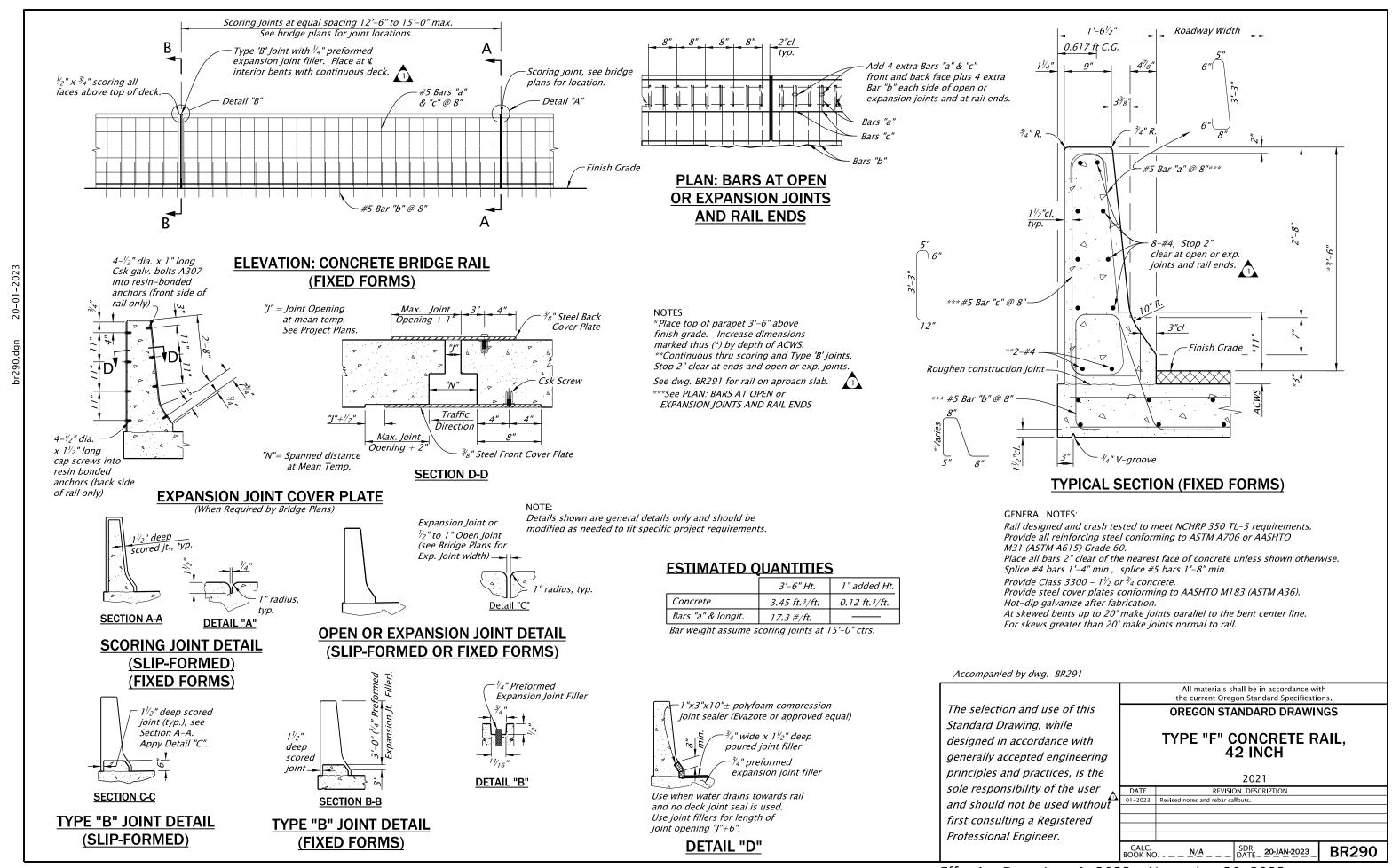


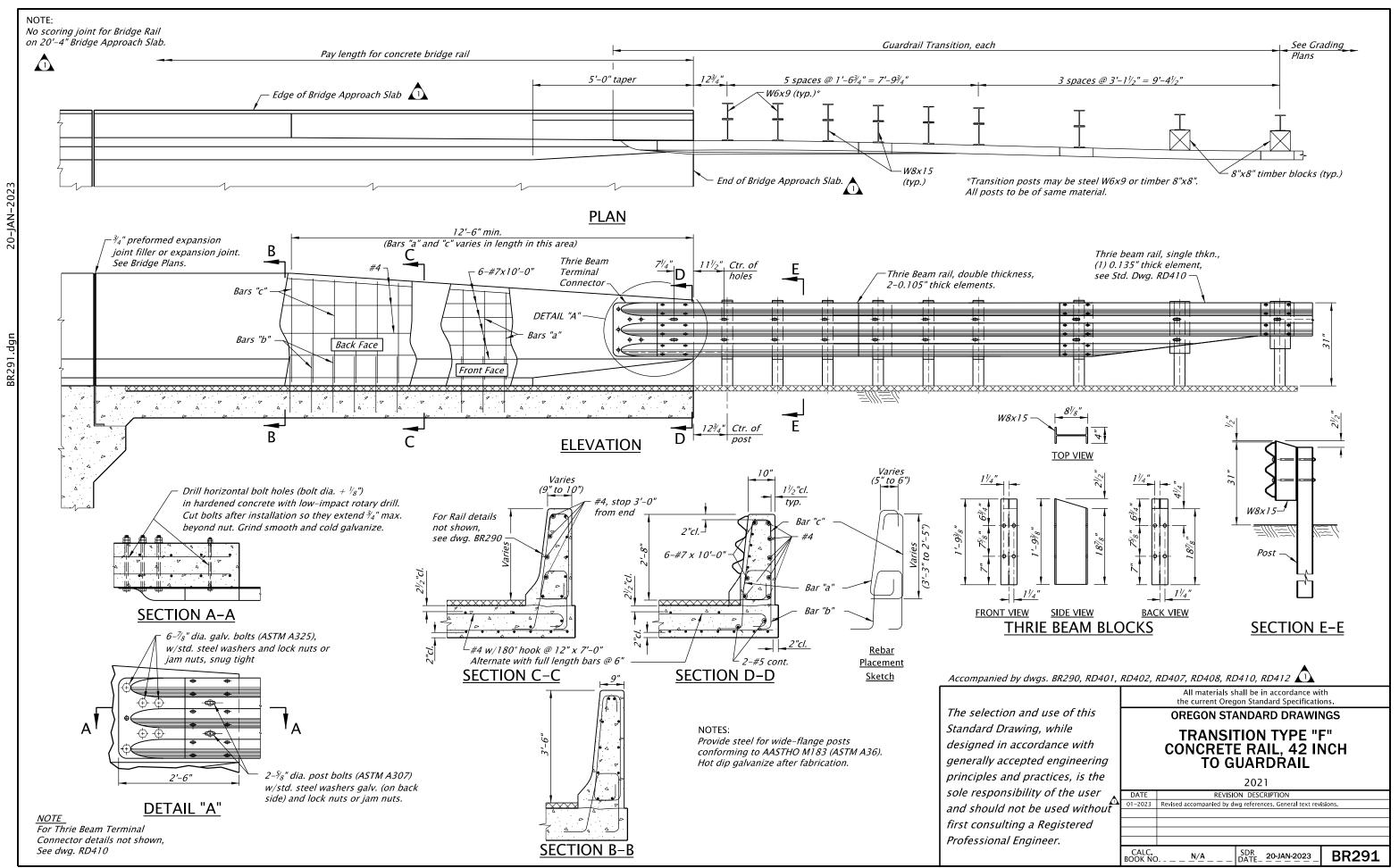


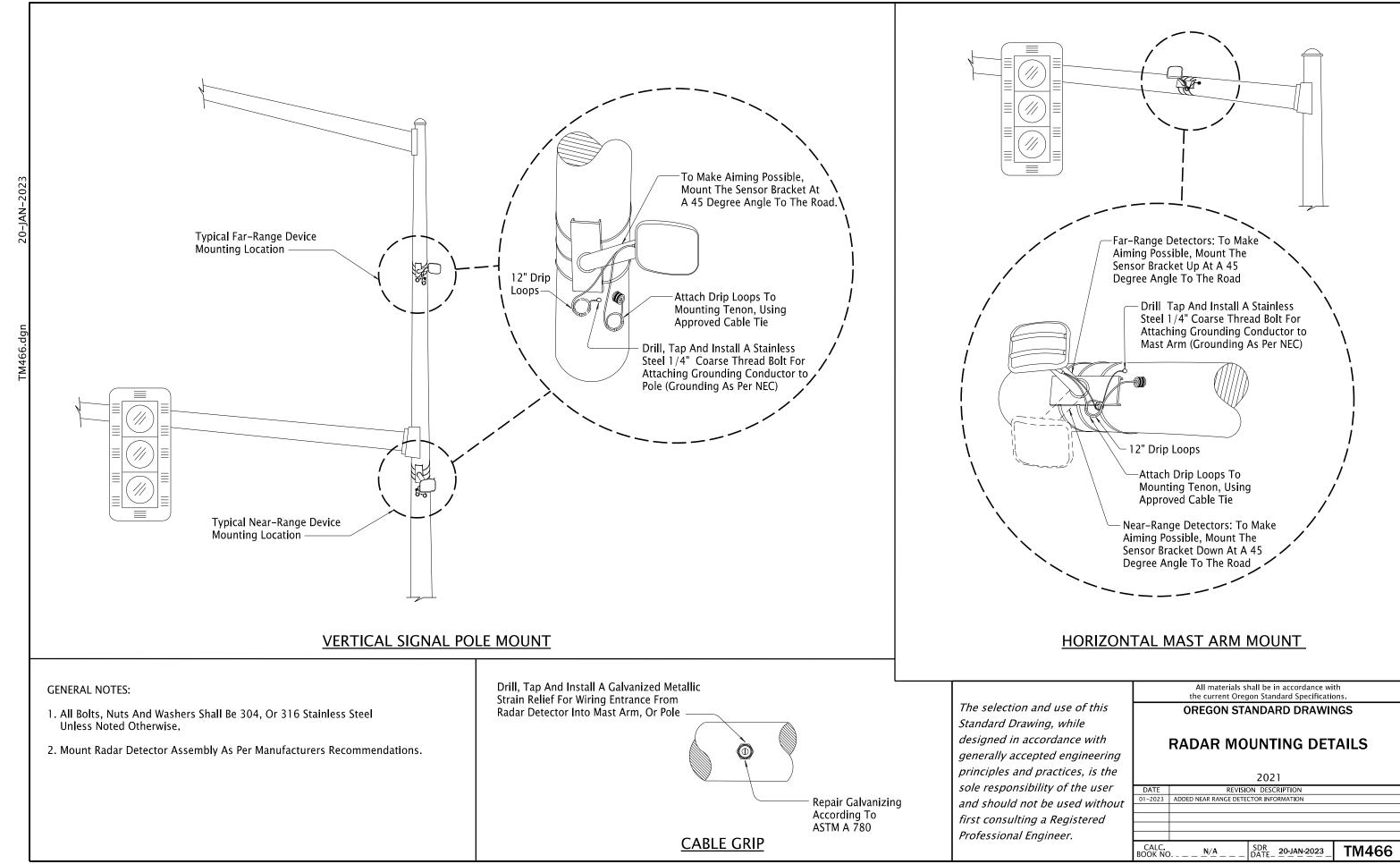


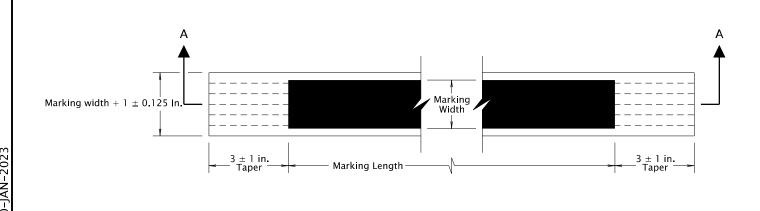


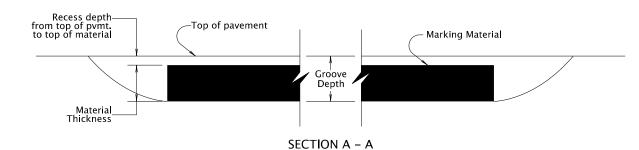












GROOVE INSTALLED GROOVE AND MATERIAL DIMENSIONS

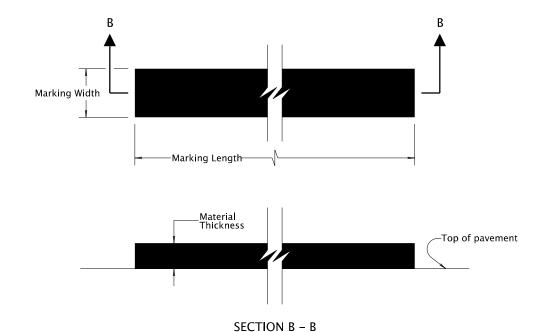
Pavement Marking Material Type	Groove Depth	Recess Depth	Material Thickness
Durable Method 'A' & Method 'D'	220 ± 20 mils	45 ± 5 mils	Var.
High Performance	60 ± 10 mils	Var.	25 mils

GROOVE INSTALLED MARKINGS

General Notes:

1) See Standard Drawing TM500 and/or project plans for marking length and width dimensions.

2) See Standard Specification 00850.46 for marking installation tolerances.



SURFACE INSTALLED MATERIAL THICKNESS

Pavement Marking Material Type	Thickness
Durable Method 'A' & Method 'B' & Method 'D'	120 mils
High Performance	25 mils

SURFACE INSTALLED MARKINGS

The selection and use of this
Standard Drawing, while
designed in accordance with
generally accepted engineering
principles and practices, is the
sole responsibility of the user
and should not be used without
first consulting a Registered
Professional Engineer.

All materials shall be in accordance with the current Oregon Standard Specifications.

OREGON STANDARD DRAWINGS DURABLE & HIGH PERFORMANCE PAVEMENT MARKINGS SURFACE & GROOVE INSTALLED NON-PROFILED

2021

DATE	REVISION DESCRIPTION					
07-2021	Changed groove width for 4 In. markings					
01-2023	Changed groove width back to previous width for 4 in. markings					
CALC.	N/A	SDR	20-JAN-2023	TM521		

Effective Date: June 1, 2023 - November 30, 2023

- 2. All traffic signal supports shall conform to the design criteria and details shown on these drawings except as approved by the Engineer.
- 3. The design basic wind speed (3 second gust) shall be 110 mph, gust factor G = 1.14, Ir = 1.0 (50 year recurrence interval), Fatigue Category II, no galloping, and truck speed = 55 mph.
- 4. Signal poles from this standard are not allowed over highways I-5, I-84, I-205, I-405, US 26 (Sunset Hwy.) between milepoints 64.3 73.0, I-105, and I-82. Signal poles on these highways require a Fatigue Category I.
- T. Pole and arm shafts may be either round, hexdecagonal, dodecagonal, or octagonal but shapes shall not be mixed on a project. Dimensional tolerances of ASTM A595 shall apply to all tapered steel tubing members. Additionally, the diameter of round tapered steel tubing members or the dimensions across the flats of octagonal tapered steel tubing members shall not vary more than 2 percent from specified dimension. Two ply and fluted poles or arms are not permitted.
- 6. Pole taper shall be equal to .0117 in/in.
- 7. Anchor rods shall conform to ASTM Specification F1554 Gr. 55, Supplementary Requirement "S2" that include grade and manufacturer's identification.
- 8. High strength bolts shall conform to ASTM F3125 Grade A325 Type 1.
- 9. Nuts for high strength bolts shall be heavy hex and conform to ASTM A563 Grade DH with supplementary requirements "S1" and "S2".
- 10. Hardened steel washers shall conform to ASTM F436 Type 1.
- 11. Direct Tension Indicators (DTI) shall be the compressible washer type, mechanically galvanized, conforming to ASTM F959.
- 12. Steel sheet for poles and arm shall conform ASTM A595, Grades A or B, ASTM A572 Gr. 50, or approved equal. All other steel sheet and plate shall conform to AASHTO specification M223 (ASTM A572), or approved equal. Supplement S18 of ASTM A6 regarding maximum tensile strength shall apply.
- 13. All structural steel including fasteners shall be hot-dip galvanized after fabrication unless otherwise noted.
- 14. Galvanize-Control Silicon, typical. Silicon content of the base metal shall be in the range of 0 to 0.04 percent or 0.15 to 0.25 percent.
- 15. Footing concrete shall be Commercial Grade Concrete (fc=3000 psi) per Specification Section 440. Grout in grout pad shall be non-shrink high early strength grout (non-ferrous) with a minimum strength of 5000 psi.
- 16. Reinforcing steel shall conform to AASHTO M31, Grade 60 (ASTM A615 or A706). A minimum lap splice length of 32 bar diameters shall be used unless shown otherwise.

- 17. Computed deflection of these poles at full design loading shall be limited to 5 percent of the pole length. Computed dead load deflection of the poles shall be limited to 1 percent of the pole length. Pole shall be raked to offset the computed dead load deflection. Computed deflection (ignoring pole bending and/or rotation) of signal arms shall not exceed that listed in the Signal Arm Deflection Table on TM650. Additionally, the amplitude (maximum up to maximum down as measured at the tip of the arm) of wind induced vertical oscillations shall not exceed 1.5 percent of the signal arm length. Luminaire arms and pole extensions to support luminaire arms shall meet requirements of standard drawing TM629.
- 18. Hubs for cabinets and/or other appurtenances shall be welded into the pole prior to galvanizing. Poles may be tapped for up to 1" galvanized bolts after pole has been galvanized.
- 19. Longitudinal seam welds within 6 inches of a circumferential weld shall be complete penetration welds. Weld inspection shall be in accordance with AWS D1.1 and the special provisions. Inspect seam welds using cyclically loaded criteria. Hubs shall be 3000# threaded forged carbon steel flat weld hubs by Anvil Products Inc., Phoenix Forging Co., Bonney Forge & Tool Works or approved equal.
- 20. Grounding terminal shall be $\frac{1}{2}$ " UNC x $1\frac{1}{2}$ " Type 308, 309 or 310 threaded stainless steel weld studs.
- 21. Assemble support, tighten anchor bolts, tighten HS thru bolts and tighten HS bolts in tapped holes according to 00962.46(j)(2).
- 22. Round and smooth all edges along electrical way.
- 23. The minimum arm flange thickness shall be equal to the value where prying action is not included in the bolt calculation.

Standard Maximum Base Reactions (Unfactored)								
Signal	Signal		Wind Lo	ad Case I	/	Cor	trolling Fa	atigue
Pole	Arm	Axial	Shear	Moment	Torque	Shear	Moment	Torque
Туре	Lengths	(Kips)	(Kips)	(Kip-ft)	(Kip-ft)	(Kips)	Kip-ft)	(Kip-ft)
SM1	15'	2.10	5.15	80.39	16.95	0.68	10.39	2.13
SM2	20', 25'	2.66	6.23	105.41	42.54	0.82	13.35	<i>5.37</i>
SM3	30', 35'	3.49	7.77	138.43	82.87	1.00	17.10	10.31
SM4	40', 45'	4.51	9.00	173.46	132.72	1.16	20.54	16.50
SM5	50', 55'	5.69	9.23	190.91	181.60	1.18	21.62	22.55
SM1L	15'	2.96	6.09	113.28	23.22	0.79	14.08	2.84
SM2L	20', 25'	3.69	7.23	139.41	48.81	0.94	17.17	6.08
SM3L	30', 35'	4.39	8.80	176.51	87.88	1.14	21.43	11.02
SM4L	40', 45'	5.94	10.14	215.11	136.97	1.31	25.27	17.21
SM5L	50', 55'	7.34	10.56	241.17	187.96	1.34	26.49	23.26

Standard Maximum Mast Arm Reactions							
Signal	Signal	Wind Load Case II Controlling Fa			ng Fatigue		
Pole	Arm	Axial Shear Moment		Shear	Moment		
Туре	Lengths	(Kips)	(Kips)	(Kip-ft)	(Kips)	(Kip-ft)	
SM1, SM1L	15'	0.06	1.98	18.44	0.23	2.18	
SM2, SM2L	20', 25'	0.10	3.14	46.20	0.37	5.48	
SM3, SM3L	30', 35'	0.15	4.51	89.42	0.53	10.51	
SM4, SM4L	40', 45'	0.23	5.91	146.67	0.67	16.82	
SM5, SM5L	50', 55'	0.34	6.78	211.94	0.70	22.99	

Luminaire Arm Reactions						
Arm	Wind Load Case II			Controlling Fatigue		
Lengths	Axial Shear Moment		Shear	Moment		
Lengths	(Kips)	(Kips)	(Kip-ft)	(Kips)	(Kip-ft)	
6'	0.03	0.31	1.49	0.03	0.15	
10'	0.06	0.38	2.85	0.04	0.29	
15'	0.08	0.47	4.96	0.05	0.51	
20'	0.05	0.55	7.24	0.06	0.74	

Accompanied by drawings TM650, TM652, TM653, TM654

The selection and use of this
Standard Drawing, while
designed in accordance with
generally accepted engineering
principles and practices, is the
sole responsibility of the user
and should not be used without
first consulting a Registered
Professional Engineer.

All materials shall be in accordance with the current Oregon Standard Specifications.

OREGON STANDARD DRAWINGS

TRAFFIC SIGNAL SUPPORTS NOTES AND REACTIONS

2021

		2021				
DATE	REVISION DESCRIPTION					
07-2020	ADDED ACCOMPANIED BY STANDARD DRAWING TM654					
07-2021	ADDED "(UNFACTORED)" TO THE TABLE HEADING					
01-2023	CHANGED HIGH STRENGTH BOLT TIGHTENING TO 00962.46(j)(2)					
CALC. BOOK NO) <u>5301</u>	SDR DATE_ 06-JAN-2023	TM651			

Effective Date: June 1, 2023 - November 30, 2023

- 1. Signal supports shall be designed in accordance with the AASHTO Standard Specifications for Structural Supports for Highway Signs, Luminaires and Traffic Signals LRFD 1st edition with 2017 and 2018 interim revisions.
- All traffic signal supports shall conform to the design criteria and details shown on these drawings except as approved by
- 3. The design basic wind speed (3 second gust) shall be 145 mph, gust factor G = 1.14, 50 year recurrence, Fatigue Category I, no galloping, and truck speed = 65 mph.
- The design service basic wind speed (3 second gust) shall be 91 mph.
- Signal poles from this standard are not allowed over highways 1-5, 1-84, 1-205, 1-405, US 26 (Sunset Hwy) between milepoints 64.3 - 73.0. I-105, and I-82.
- Pole and arm shafts must be round. Dimensional tolerances of ASTM A595 shall apply to all tapered steel tubing members. Additionally, the diameter of round tapered steel tubing members shall not vary more than 2 percent from specified dimension. Two ply and fluted poles or arms are not permitted.
- Pole taper shall be equal to .0117 in/in.
- Anchor rods shall conform to ASTM Specification F1554 Gr. 55, Supplemenetary Requirement "S2" that include grade and manufacturer's identification.
- High strength bolts shall conform to ASTM F3125 Grade A325 Type 1.
- 10. Nuts for high strength bolts shall be heavy hex and conform to ASTM A563 Grade DH with supplmentary requirements "S1"
- 11. Hardened steel washers shall conform to ASTM F436 Type 1.
- 12. Direct Tension Indicators (DTI) shall be the compressible-washer type, mechanically galvanized, conforming to ASTM F959.
- 13. Steel sheet for poles and arm shall conform ASTM A595, Grades A or B, ASTM A572 Gr. 50, or approved equal. All other steel sheet and plate shall conform to AASHTO specification M223 (ASTM A572), or approved equal. Supplement S18 of ASTM A6 regarding maximum tensile strength shall apply.
- 14. All structural steel including fasteners shall be hot-dip galvanized after fabrication unless otherwise noted.
- 15. Galvanize-Control Silicon, typical. Silicon content of the base metal shall be in the range of 0 to 0.06 percent or 0.13 percent to 0.25 percent.
- 16. Footing concrete shall be according to TM628.
- 17. Reinforcing steel shall conform to AASHTO M31, Grade 60 (ASTM A615 or A706). A minimum lap splice length of 32 bar diameters shall be used unless shown otherwise.

- 18. Computed deflection of these poles at full design loading shall be limited to 5 percent of the pole length. Computed dead load deflection of the poles shall be limited to 1 percent of the pole length. Rake pole, apply mast arm and appurtenance loads, and verify final pole position is plumb.
- 19. Luminaire arms and pole extensions to support luminaire arms shall meet requirements of drawing TM629.
- 20. Hubs for cabinets and/or other appurtenances shall be welded into the pole prior to galvanizing. Poles may be tapped for up to 1" galvanized bolts after pole has been galvanized.
- 21. Longitudinal seam welds within 6 inches of a cirumferential weld shall be complete penetration welds. Weld inspection shall be in accordance with AWS D1.1 and the special provisions. Inspect seam welds using cyclically loaded criteria. Hubs shall be 3000# threaded forged carbon steel flat weld hubs by Anvil Products Inc., Phoenix Forging Co., Bonney Forge & Tool Works or approved equal.
- 22. Grounding terminal shall be $\frac{1}{2}$ " UNC x $1\frac{1}{2}$ " Type 308, 309 or 310 threaded stainless steel weld studs.
- 23. Assemble support, tighten anchor bolts, tighten HS thru bolts, and tighten HS bolts in tapped holes according to 00962.46(j)(2).
- 24. Round and smooth all edges along electrical way.

	Re	At Base P. tored)	late	Reaction At Base Plate (Service)			Plate	
Signal Pole Type	Axial (lb)	Shear (lb)	Moment (ft-lb)	Torsion (ft-lb)	Axial (lb)	Shear (lb)	Moment (ft-lb)	Torsion (ft-lb)
SM6L	7,430	13,000	301,000	322,000	6,520	5,200	163,000	127,000
SM7L	8,860	13,100	349,000	385,500	8,080	5,190	212,720	153,000

Note:

The base plate reactions shown in the table are worst case Extreme I and Service I loads. Engineer of Record to specify shaft depth and confirm shaft design for local soil conditions based on a site specific geotechnical study and loads shown in table. If shaft size or reinforcement shown in the table on TM628 for the required design number are not adequate for local soil conditions, Engineer of Record must adjust the shaft design accordingly.

Accompanied by drawings TM628, TM654, TM655, TM657, TM658

The selection and use of this Standard Drawing, while designed in accordance with generally accepted engineering principles and practices, is the sole responsibility of the user and should not be used without first consulting a Registered Professional Engineer.

All materials shall be in accordance with the current Oregon Standard Specifications.

OREGON STANDARD DRAWINGS

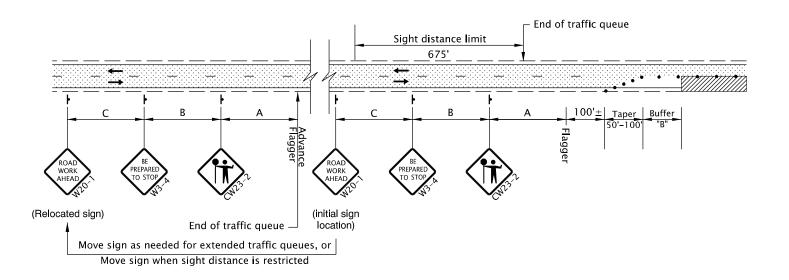
TRAFFIC SIGNAL 60' THROUGH 75' **MAST ARM SUPPORTS NOTES AND REACTIONS**

2021

DATE	REVISION DESCRIPTION			
07-2020	ADDED ACCOMPANIED BY STANDARD DRAWING TM654			
07-2022	ADDED REACTIONS TO TITLE AND ADDED REACTIONS TABLE			
01/2023	CHANGED HIGH STRENGTH BOLT TIGHTENING TO 00962.46(J)(2)			
CALC. BOOK NO) <u>7088</u>	SDR DATE_ 06-JAN-2023	TM656	

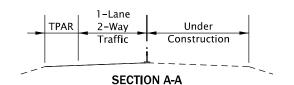


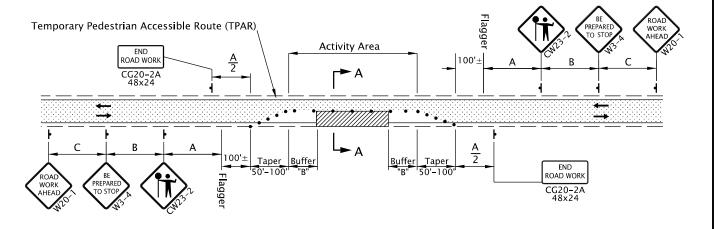
- Place Advance Flagger and additional signing when traffic queues extend beyond initial warning signing OR when sight distance is restricted.
- Place additional Tubular Markers for Flagger and Advance Flagger Stations according to FLAGGER STATION DELINEATION detail.
- Relocate initial "ROAD WORK AHEAD" (W20-1) sign in advance of additional "BE PREPARED TO STOP" (W3-4) and Flagger Ahead (CW23-2) signs, as shown.



NOTE:

- When using pilot cars with flaggers to control traffic during paving operations, the Tubular Marker spacing along centerline may be increased to 200' within the Activity Area, as shown or as directed.
- Include "WAIT FOR FLAGGER" (CR4-23) signs mounted on Type II Barricade located approx. 50' before each Flagger.
- Coordinate and control pedestrians movements through the TPAR using Flaggers, other TCM, or as directed. When the existing shoulder is greater than or equal to 4' wide, provide a minimum of 4' of width for the TPAR.





2-Lane, 2-Way Roadway ONE LANE CLOSURE

ADVANCE FLAGGER FOR EXTENDED TRAFFIC QUEUES

GENERAL NOTES FOR ALL DETAILS:

- This drawing is only intended to be used where an Automated Flagger Assistance Device (AFAD) cannot be utilized.
- The "FLAGGER" (CW23-2) symbol sign shall be used only in conjunction with the "BE PREPARED TO STOP" (W3-4) sign.
- Cover existing passing zone signing, as directed.
- Install temporary striping as required.
- To determine Taper Length ("L") and Buffer Length ("B"), use the "MINIMUM LENGTHS TABLE" shown on Dwg. No. TM800.
- To determine sign spacing A, B, and C, use "TRAFFIC CONTROL DEVICES (TCD) SPACING TABLE" on Dwg. No. TM800.
- Install a "BICYCLES ON ROADWAY" (CW11-1) sign in advance of the closure when a bike lane is closed, or when the shoulder is closed and bikes are expected.
- At night, flagger stations shall be illuminated according to the FLAGGER STATION LIGHTING DELINEATION detail on Dwg No. TM800.
- To be accompanied by Dwg. Nos. TM820 & TM821.

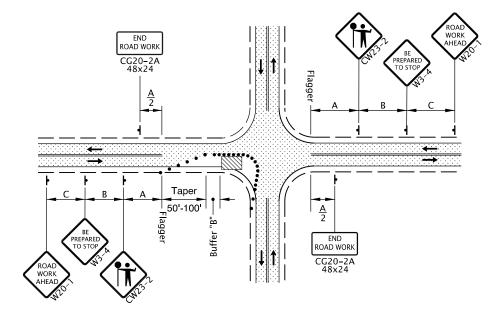
••••• 28" Tubular Markers on 10'max. spacing arround intersection radii.

- 28" Tubular Markers on 20' max. spacing for flagger tapers and stations
- 28" Tubular Markers See TCD Spacing Table on TM800 for max. spacing.

UNDER TRAFFIC

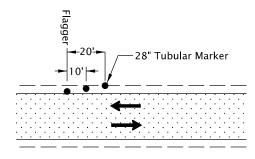
UNDER CONSTRUCTION

• Additional Traffic Control Measures (TCM) may be required for all legs of the intersection



2-Lane, 2-Way Roadway ONE LANE CLOSURE, INTERSECTION

• Use a minimum of 3 tubular markers in shoulder taper on 10' spacing for flagger station



FLAGGER STATION DELINEATION

The selection and use of this Standard Drawing, while designed in accordance with generally accepted engineering principles and practices, is the sole responsibility of the user and should not be used without first consulting a Registered Professional Engineer.

OREGON STANDARD DRAWINGS 2-LANE, 2-WAY ROADWAYS 2021

All materials shall be in accordance with

the current Oregon Standard Specifications.

	2021					
	ON DESCRIPTION	DATE REVISION DESCRIPTION				
TM855	SDR DATE_ 13-JAN-2023 _	<u>N/A</u>	CALC. BOOK NO			